CONSERVATION SERVICES INCORPORATED DESIGN AND OPERATIONS PLAN

Prepared for:

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TABLE OF CONTENTS

1.0	INTRO	DUCTION	1
	1.1	Site Selection Process	2
2.0	GENE	RAL INFORMATION	1
	2.1	Facility Need	2
	2.3	2.2.1 Site Access	6
3.0	SITE	DESCRIPTION	1
	3.1 3.2 3.3		1 1 5
4.0	GEOL	OGY AND HYDROGEOLOGY 4-	1
	4.1	Regional Geology 4-	1
		4.1.1 Regional Surficial Geology	3
	4.2	Site Geology	-5
		4.2.1 Field Investigations	·6 ·8
	4.3	Regional Hydrogeology 4-1	.0
		4.3.1 Regional Ground-Water Basin 4-1 4.3.2 Regional Aquifer Distribution 4-1	.0
		4.3.2.1 Laramie-Fox Hills Aquifer 4-1 4.3.2.2 Arapahoe Aquifer	.3

	4.4	Site Hydrogeology 4-13
		4.4.1 Water Well Inventory in the Vicinity of
		Site 4-13
		4.4.2 Field Investigation 4-17
		4.4.3 Ground Water Occurrence and Conditions 4-19
		4.4.5 Hydrogeologic Characterization 4-31
		4.4.6 Evidence of Perched Conditions 4-36
		4.4.7 Perched Ground-Water Flow Paths and
		Velocities 4-38
		4.4.7.1 Lateral Migration 4-38
		4.4.7.1 Dateral Migration 4-20
		4.4.7.2 Vertical Migration 4-39
5.0	FACI	LITY CONFIGURATION 5-1
	5.1	Non-Hazardous Waste Characterization 5-1
		5.1.1 Laboratory Quality Assurance/Quality
		Control Plan 5-4
		Control Plan 5-4
		5.1.1.1 Objective 5-4
		5.1.1.2 Sample Handling
		5.1.1.2 Sample nanuting 5-4
		5.1.1.3 Analytical Methodology 5-6
		5.1.1.4 Instrument Maintenance 5-6
		5.1.1.5 Corrective Action 5-7
		5.1.1.6 Recordkeeping 5-7
		T t o musicument and complitation
		5.1.2 Equipment and Capabilities 5-8
		5.1.3 Personnel and Qualifications 5-8
		5.1.4 Waste Analysis Plan 5-8
		5.1.4.1 Waste Characterization 5-9
		5.1.4.1 waste Characterization 5-9
		5.1.4.2 Waste Acceptance Procedure 5-10
		5.1.4.3 Waste Screening 5-14
	5.2	Non-Hazardous Liquids Solidification
	3.2	Non-inglations Diquids board
		and Disposal 5-16
		5.2.1 Process Description and System Capacity . 5-16
		5.2.2 Solidification Area 5-18
		5.2.2.1 Location and Surface
		Water Control 5-18
		5.2.2.2 Mixing Basin Design 5-18
		5.2.2.3 Mixing Basin Construction 5-20
		5 2 2 4 Auviliary Structures 5-22

		5.2.3	Solidi	fied V	Waste	Dis	posa	l Ar	ea	•	•	•	•		•	5-26
		5	.2.3.1	Cell	Quant Surfa	ace T	Vate	c Cc	ntr	col						5-26 5-28
			.2.3.4	Cons	struct	tion			•				• •	•	•	5-28 5-34
		5	.2.3.5	Phase	ed Cel	11 C	onsti	cuct	ior	1	•	• .				5-35
		5	.2.3.7		Frial- Cap										•	5-36 5-37
	5.3	Asbest								•	•	• (•	5-39
			Asbest				and	Con	str	uc	tio	on				5-40
			Waste :													5-41
			Waste :													5-43
	5.4	Surface	e Water	Cont	rol .					•	•	• (. ,		•	5-44
6.0	FACI	LITY OP	ERATION	• •					•	•	•	• (. 6 - 1
	6.1	Site Ma	anageme:	nt .					•	•	•	• , (, ,	. 6-1
			Operat													
		6.1.2														
		6.1.3	Facili'													
		6.1.4	Record	keepi	ng .		• •		•	•	•	• (•	, ,	•	6-3
		6	.1.4.1		Hazaro											, ,
		_		Reco	ordke	sprud		• •	•	•	•	• •	•	•	•	6-3
			.1.4.2		ytica											
			.1.4.3													
		6	.1.4.4	Misc	ellane	eous	Reco	orak	eer	nın	g .	• •	• •	•	•	. 6-4
		6.1.5	Site S	ecuri	ty .				•	•	•	• •			, ,	. 6 - 4
	6.2	Contro	l of Nu	isance	e Situ	uatio	ons		•	•	•				• •	6-6
		6.2.1	Dust Co	ontro:	l						•			, ,	, ,	6-6
		6.2.2	Odor C	ontro	l											6-7
		6.2.3	Fire P	revent	tion				•				, .	, ,	, ,	6-7
		6.2.4	Fire Pontro	lling	Water	r in	Oper	ı Ce	lls	;	•					6-8
	6.3	Health														
7 0	ביאנגיד די	ጉእፕለተመለተጠ አ	r MANTE	ODING												7 1

	7.1 7.2 7.3 7.4	Leachar Ground Operation	-Water	Monit Monit	oring	•			•	•			•		•	•	•	7-1 7-2 7-4 7-6
		7.4.1 7.4.2	Propo	sed Ad	of Ne ction ing Re	in t	the	Eve	ent	οſ	N	eg	at	iv	е			7-6 7-7
8.0	CONS	ructio	N QUAL	ITY AS	SURAN	CE I	AND	COI	ITRO	ΟL	•	•	•	•	•	•	•	8-1
	8.1 8.2 8.3 8.4	Materi Excava Clay L As-Bui	tion I iner a	nspect nd Fir	cions nal Ce	:11 (Cap	Ins	spec	cti	.on	s	•	•	•	•	•	8-1 8-2
9.0	CLOS	URE AND	POST-	CLOSUI	RE REÇ	UIR	EMEN	ITS	•	•	•	•	•	•	•	•	•	9-1
	9.1 9.2 9.3 9.4 9.5	Closur Reclam Post-C Post-C Projec	losure losure	Inspe	ection	ı . ıs a:	 nd N	Mair	ite	nar	· ice	•	•	•	•	•	•	9-6
					FIGU	IRES												
FIGUE	RE 2-	1 GENE	RAL SI	TE LO	CATION	Ι.		•		•	•	•	•		•	•	•	2-3
FIGUE	RE 2-	2 SITE	LOCAT	ION .		•		•		•	•	•			•			2-4
FIGUE	RE 2-	3 OPER	ATIONS	AREA	CONFI	GUR	ATIC	N.		•	•						•	2-5
FIGUE	RE 2-	4 ADJA	CENT Z	ONING	AND C	WNE:	RSH]	ſΡ.		•					•			2-8
FIGUI	RE 3-	1 SURF	ACE WA	TER W	ITHIN	A 2	MII	LE I	RAD	IUS	3				•			3-2
FIGUE	RE 3-	2 SITE	DRAIN	AGE P	ATTERN	Ι.		•		•					•	•	ψ,	3-3
		3 DRAI																
		4 WIND																
		1 GENE																
		2 LARA																
		3 <u>1</u> 2 12 12 12 12 12 12 12 12 12 12 12 12 12																

FIGURE 4-4	DENVER AQUIFER 4-16
FIGURE 5-1	DRUM STORAGE EXTENSION PAD 5-24
FIGURE 5-2	CLAY LINER COMPATIBILITY 5-30
FIGURE 7-1	TYPICAL GROUND WATER MONITOR WELL CONSTRUCTION
	TABLES
TABLE 2-1	OWNERSHIP WITHIN 1 MILE OF THE PROPOSED CSI FACILITY 2-9
TABLE 4-1	GEOTECHNICAL LABORATORY RESULTS 4-11
TABLE 4-2	WELLS WITHIN A 2 MILE RADIUS 4-18
TABLE 4-3	WATER LEVEL DATA 4-22
TABLE 4-4	FIELD PARAMETER RESULTS 4-28
TABLE 4-5	GROUND-WATER ANALYTICAL RESULTS 4-29
TABLE 4-6	PACKER TEST RESULTS 4-32
TABLE 4-7	RISING HEAD TEST RESULTS 4-33
TABLE 4-8	CONSTANT RATE PERMEABILITY TESTS 4-35
TABLE 5-1	TYPICAL WASTES PROCESSED AT CSI 5-3
TABLE 5-2	KILN DUST ANALYSIS 5-17
TABLE 5-3	APPROXIMATE CELL VOLUMES 5-27
	APPENDICES
APPENDIX A	TRAFFIC STUDY
APPENDIX B	DRAINAGE AND FLOOD PLAIN REPORT
APPENDIX C	CLIMATOLOGY
APPENDIX D	ORIGINAL FIELD DATA AND BORING LOGS
APPENDIX E	FIELD AND LABORATORY SOILS TEST RESULTS

APPENDIX F	MONITOR WELL AND PIEZOMETER COMPLETION FORMS
APPENDIX G	WATER QUALITY RESULTS
APPENDIX H	PACKER TEST RESULTS
APPENDIX I	RISING HEAD TEST RESULTS
APPENDIX J	ESTIMATED TRAVEL TIME OF WATER
APPENDIX K	ANALYTICAL LABORATORY PAPERWORK AND FORMS
APPENDIX L	CSI PERSONNEL JOB DESCRIPTIONS
APPENDIX M	CLAY LINER EFFICIENCY CALCULATIONS
APPENDIX N	COMPATIBILITY INFORMATION
APPENDIX O	WATER BALANCE CALCULATIONS
APPENDIX P	EROSION AND SOIL LOSS CALCULATIONS

APPENDIX Q CELL CONSTRUCTION SPECIFICATIONS

PLATES

PLATE 1/2	CSI EAST SITE PLAN
PLATE 3	SOIL BORING AND GEOLOGIC CROSS-SECTION LOCATIONS
PLATE 4	GEOLOGIC CROSS-SECTIONS A-A', B-B'
PLATE 5	GEOLOGIC CROSS-SECTIONS C-C', D-D'
PLATE 6	GEOLOGIC CROSS-SECTIONS E-E', F-F'
PLATE 7	CSI EAST SOLIDIFIED WASTE DISPOSAL CELL DESIGN
PLATE 8	CSI EAST ASBESTOS CELL DESIGN AND FILLING PLAN
PLATE 9	MIXING BASIN A - PLAN AND SECTIONS
PLATE 10	MIXING BASIN A - SECTIONS AND DETAILS
PLATE 11	CSI EAST PROPOSED FINISHED GRADES
PLATE 12	MIXING BASIN B - PLAN AND SECTIONS
PLATE 13	MIXING BASIN B - SECTIONS AND DETAILS

1.0 INTRODUCTION

This document presents the revised Design and Operations Plan for an asbestos and non-hazardous liquid solidification and disposal facility owned and operated by Conservation Services Incorporated (CSI). This facility is a continuation of a unique alternative to using current landfilling practices for disposing of liquid wastes. The site offers an environmentally sound method of disposal for asbestos, non-hazardous liquid wastes and various other materials not often accepted by sanitary landfills.

The information used to design this facility and prepare this report originated from a variety of sources including previously-approved sanitary landfill permit applications, the approved permit for operation of the existing CSI facility, and from site-specific field work. Additional information was taken from data gathered during operation of the CSI facility at 62nd and Huron. More specific portions of the Design and Operations Plan were revised according to comments and revisions of a previous plan that had Colorado Department of Health (CDH) approval but that was not issued a Certificate of Designation (CD) because of the proposed airport alignment. The engineering designs and field investigations conform with Colorado Solid Waste Siting, Design and Operations regulations, as amended October, 1985. Operations for the disposal of asbestos materials conform to Colorado Solid Waste regulations, effective May 1, 1988.

CSI had originally completed a Design and Operations Plan for a new facility in 1987 at another location. The submittal included a descriptive narrative related to land use issues and was approved by the Adams County Planning and Zoning Commission and the Adams County Commissioners. Following this approval, CSI submitted a detailed technical document outlining the geologic and hydrologic characteristics of the site, engineered features of the facility, protocol for accepting waste streams, a filling plan for the site, and a reclamation plan for the site. Design and Operations Plan received approval from the Colorado Department of Health and was placed on the Adams County Planning and Zoning Commission agenda. Prior to the Planning Commission hearing, the Adams County residents voted to allow Denver to The land includes the annex land for a proposed airport. original proposed CSI site therefore it became necessary for CSI to find another suitable site. CSI conducted a site search and chose the property located in the northwestern quarter and the western half of the northeastern quarter of Section 25, Township 2 South, Range 64 West.

1.1 Site Selection Process

CSI conducted a site search to find a suitable site for the proposed facility. Each site was researched according to the following criteria:

- * Site availability
- * Geological characteristics
- * Hydrogeological characteristics
- * Site location
- * Site size
- * Accessibility

Sites meeting the initial screening process were studied further using a preliminary drilling program to determine the subsurface characteristics. The proposed site was determined to be feasible for the facility following the preliminary drilling program. An extensive site study was conducted and results are included in this report.

CSI submitted a narrative description of the proposed site in August, 1988. The description contains a general outline relating to current and proposed land uses and some background information relating to the site.

As the next step in the Adams County Certificate of Designation process, CSI has compiled and submitted a detailed Design and Operations Plan. The plan is divided into sections that detail information in the following areas:

- * General Information
- * Site Description
- * Site Geology/Hydrogeology
- * Facility Configuration
- * Facility Operation
- * Environmental Monitoring
- * Construction Quality Assurance and Control
- * Closure and Post-Closure

Existing operational information and the results of the field investigations indicate that the site is suitable for the CSI facility and that it can be developed, operated and closed in a manner consistent with current environmental standards.

2.0 GENERAL INFORMATION

2.1 Facility Need

The CSI facility offers a unique and environmentally sound alternative to non-hazardous liquid waste disposal methods currently used throughout much of the Front Range. Liquid wastes are disposed of in a variety of ways including landfilling, evaporation from surface impoundments, landfarming, and solidification and landfilling in specially-designed containment cells, as CSI is doing. The first three methods increase the probability of leachate infiltration resulting in a decrease in ground-water quality, and are often not environmentally-prudent practices. Most area landfills have banned the acceptance of any type of liquids. The CSI procedure involves mixing the liquids with a solidification agent, resulting in a solidified material that is landfilled in cells lined with impermeable materials.

In addition to the non-hazardous liquid waste solidification and special-waste disposal services, CSI offers: asbestos disposal in a segregated cell; transportation and waste handling services; facility clean-ups; emergency response; and consultation services. This broad range of services allows CSI to analyze and solve special-waste problems through consultation, special-waste management, and disposal for the industrial community in Colorado.

The currently operating CSI facility is unique in the Front Range area. The continued operation of such a facility benefits Denver and the metropolitan area both economically and environmentally. Economically, the benefits include the revenue brought into Adams County and the employment opportunities created by CSI. In addition to these more immediate benefits, CSI provides industry with the option to decrease long-term financial liabilities by disposing of special wastes in a controlled and secured area.

The environmental advantages will benefit Adams County and the entire Front Range. Disposal of non-hazardous materials is becoming a more publicized environmental issue and the CSI facility uses state-of-the-art techniques to carefully and safely dispose of the materials.

2.2 General Facility Description

The CSI facility is located at Schumaker Road and 88th Avenue (Figures 2.1 and 2.2). The total site area is 240 acres; however, only the eastern 135 acres will be utilized for the CSI facility. The 135 acres include approximately 10 acres for the non-hazardous liquid solidification area, less than 115 acres for the solidified waste disposal cell area, and an asbestos cell that will require approximately 4 acres.

The facility is located approximately 7 miles north-northwest of the Town of Bennett and 9 miles north-northeast of the Town of Watkins. A one-acre parcel of land located in the northwest corner of the 240-acre site is owned by the Bennett School District. The closest disposal cell to this parcel is 1100 feet and the CSI operations facility is 2900 feet from the land. Colorado Interstate Gas Company (CIG) operates a gas pipeline that is located in the southeastern corner of the site.

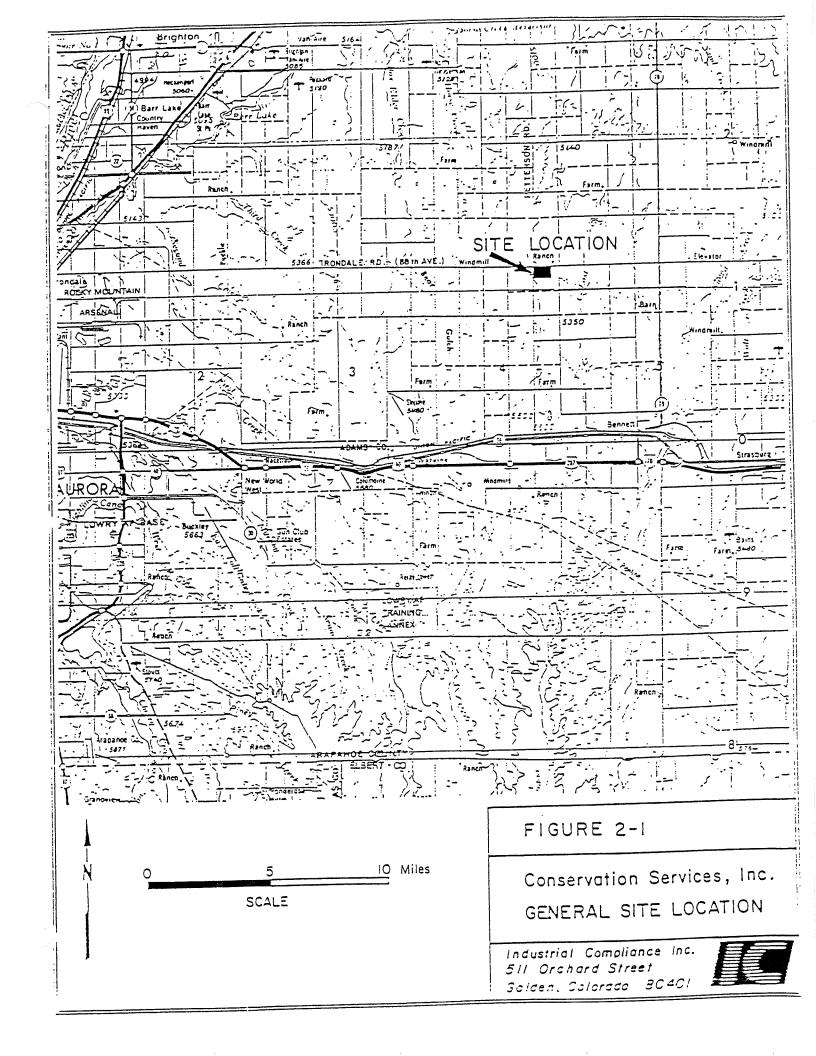
The legal description for the CSI site is:

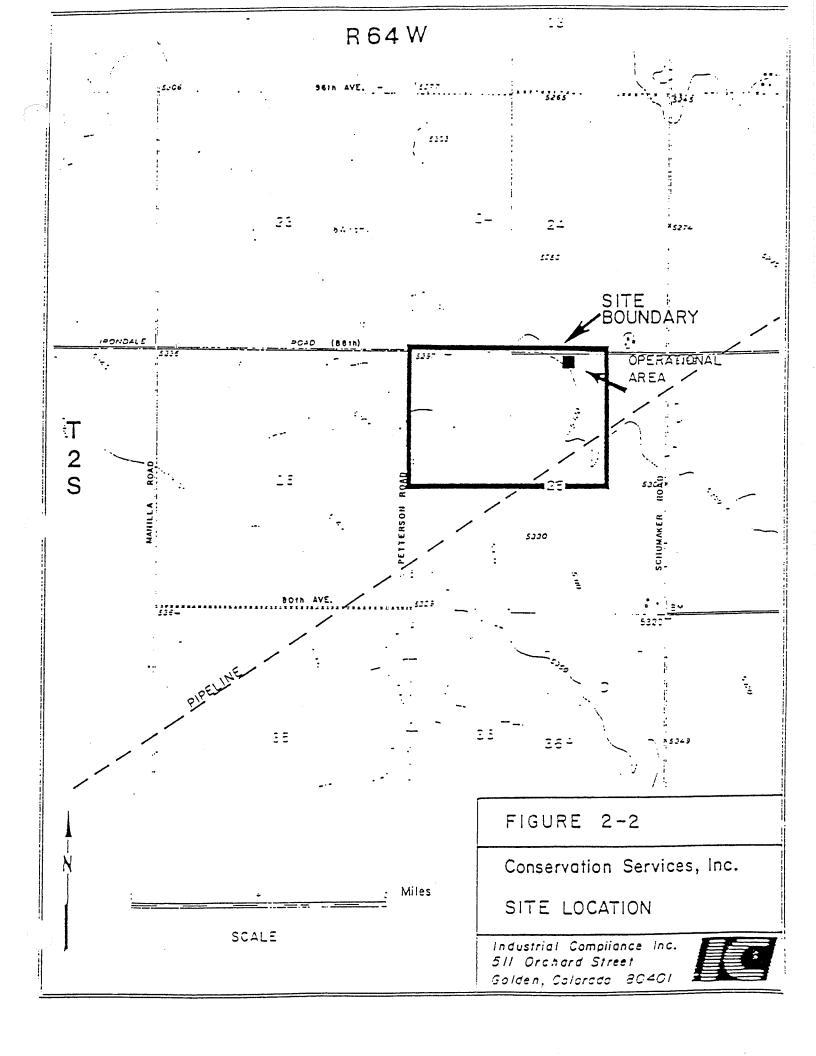
The east 1/2 of the northeast 1/4 and the northeast 1/4 of the southeast 1/4 of Section 34; Township 2 South, Range 66 West of the 6th Principal Meridian, County of Adams, State of Colorado.

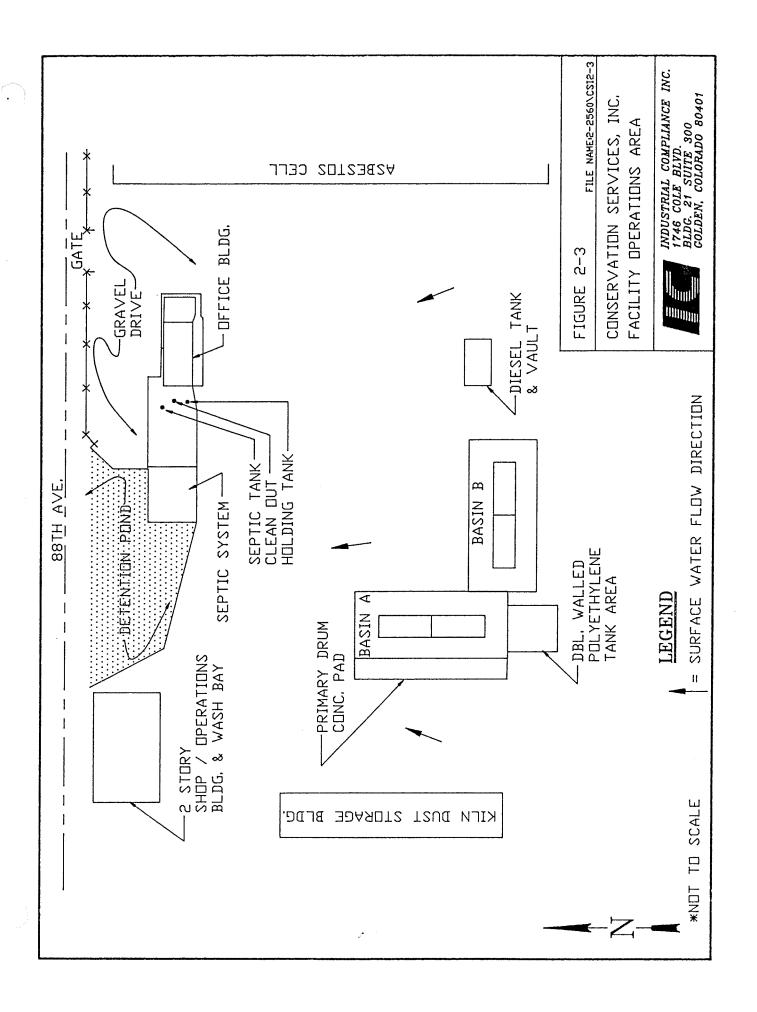
The only liquids accepted will be non-hazardous industrial waste liquids and sludges that can be solidified through the addition of a material such as cement kiln dust. The solidification process stabilizes the liquids and sludges into a cement-like material containing no free liquids and that is environmentally safe to handle and landfill.

The site consists of three areas: an operations area; a solidified waste disposal cell area; and an area used for the disposal of asbestos. Plate 1 is a diagram of the site layout.

The operations area is located on approximately 10 acres in the northcentral corner of the site. The operations area, shown in Figure 2.3, includes the following:







- * Offices
- * Disposal office
- * Sampling station
- * Mixing basins
- * Kiln dust storage
- * Truck wash/maintenance shop
- * Tank farm
- * Drum storage area
- * Equipment storage
- * Parking spaces

Approximately 115 acres of the site will be used for solidified material filling. Specially-designed cells will be constructed using natural and synthetic materials to create nearly impermeable barriers on the cell bottom and sides. Following mixing, the solidified material is tested to ensure no free liquids are present and placed in the cell. Curing is allowed to complete in the cell. There will not be more than two cells or portions of cells available for disposal at any one time, except if a client specifically requires a cell to segregate their material (subject to Planning Commission approval as per resolution 5. G.). The cells will be closed and revegetated after they reach filling capacity so the disturbed areas of the site will be minimized.

An asbestos cell was constructed east the operations portion of the site. The disposal procedures are according to the CDH quidelines dated May 2, 1988.

Based on projections of filling 100,000 cy per year, the facility life is projected to be approximately 21 years.

2.2.1 Site Access

Access to the CSI facility is from 88th Avenue (Irondale Rd.). Another potential access route to the facility is from I-70 to Colorado State Highway 79 and north to 88th Avenue. A traffic study completed by a traffic engineering consultant concludes that the traffic system is adequate to serve the site and is not expected to pose a problem (Appendix A).

2.3 Zoning and Land Use

The CSI facility land is presently zoned A-3/Agricultural. Zoning and land ownership in the vicinity of the site are included as Figure 2.4. Table 2.1 includes ownership information within an approximate 1 mile radius of the site. The letter next to

each name coincides with the letter on the property map. Zoning of each parcel of land is included on Figure 2.4. The site is outside the boundaries of the influence area for the Front Range Airport expansion. The land surrounding the site is currently used for grazing and crop farming.

Adams County is in the process of completing a draft Environs Plan for the areas within the anticipated zone of influence of the proposed Denver Airport. The CSI site is in an area currently designated as potential residentially-zoned land because it is outside the noise impact area. The properties immediately to the north of the site are slated to be non-residentially zoned and have the potential to be used for light industry. The facility can be compatible with either of these uses in the future. Adams County feels it is unlikely that the area will be developed for another 30 to 50 years. The CSI facility will be closed by this time and uses for the land will be established as development begins. The site is also outside the influence zone of the proposed Front Range Airport expansion.

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AREA MAP OF PROPERTY OWNERS WITHIN 1 MILE

A-3 AGRICULTURAL

AIZ AIRPORT INFLUENCE ZONE

(FRONT RANGE AIRPORT)

* SEE TABLE 2-1 FOR

OWNERSHIP INFO.

FIGURE 2-4

Conservation Services, Inc.

ADJACENT ZONING &
OWNERSHIP

Industrial Compliance Inc. 511 Oronard Street Golden, Colorado 30401



TABLE 2-1

OWNERSHIP WITHIN 1 MILE OF THE CSI FACILITY

Α.	BENNETT SCHOOL DISTRICT #30 BENNETT, COLORADO 80102		
c.	SOPHIE J. WAGNER 5 N. 13TH AVENUE BRIGHTON, COLORADO 80601	D.	BYRON AND THELMA TRUPP BENNETT, COLORADO 80102
E.	SIDNEY ALLISON RICHARD KIMBROUGH SCOTIA, NEBRASKA 68875	F.	HAROLD MORAN 601 CHAMBERS ROAD, #300 AURORA, COLORADO 80011
G.	BERNICE BULLARD 208 - 10TH AVENUE LONGMONT, COLORADO 80501		IVAN AND IRENE STARK 33401 E. 88TH AVE. RT. 1, BOX 77 COMMERCE CITY, CO 80022
I.	VIRGIL PILAND RT 1, BOX 20 BENNETT, COLORADO 80102	J.	MARILYN CRONK BOX 159 BENNETT, COLORADO 80102
K.	DONALD AND ELLA TRUPP BENNETT, COLORADO 80102	L.	J.W.C. DAVIS BENNETT, COLORADO 80102

3.0 SITE DESCRIPTION

3.1 Existing Site Topography

The original topography is presented on Plate 1. The site is located in gently-sloping farmland that drains from the high point of the site (approximately 1000 feet in from the western site boundary) radially to the east and west or northwest. The cells are located east of this high point.

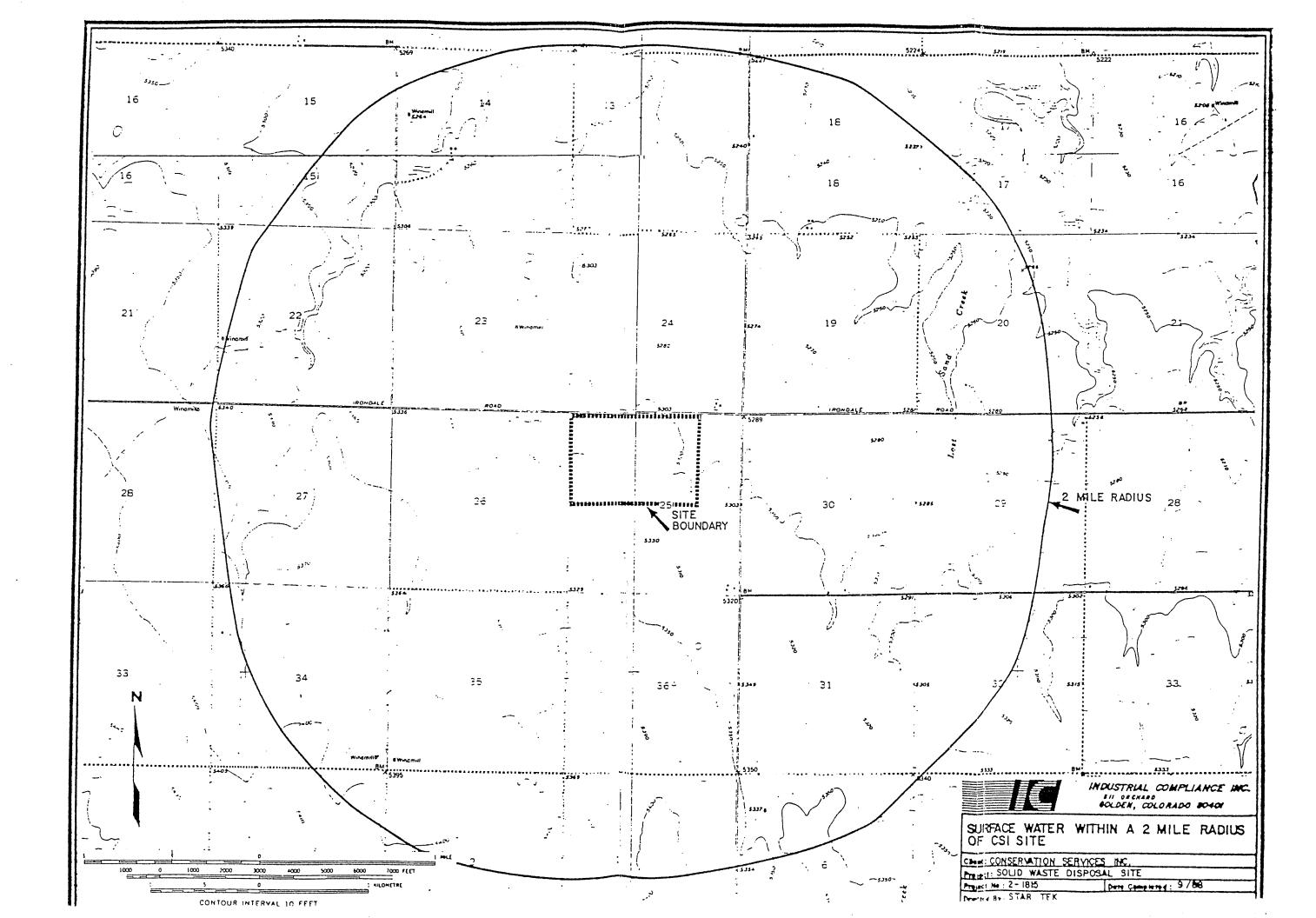
The property slopes an average of approximately 1.0 percent across the portions of the site that are for use. The steepest section of this portion of the property slopes approximately 3 percent. In the western unused portion of the site the slopes vary from less than 1 percent to 5 percent.

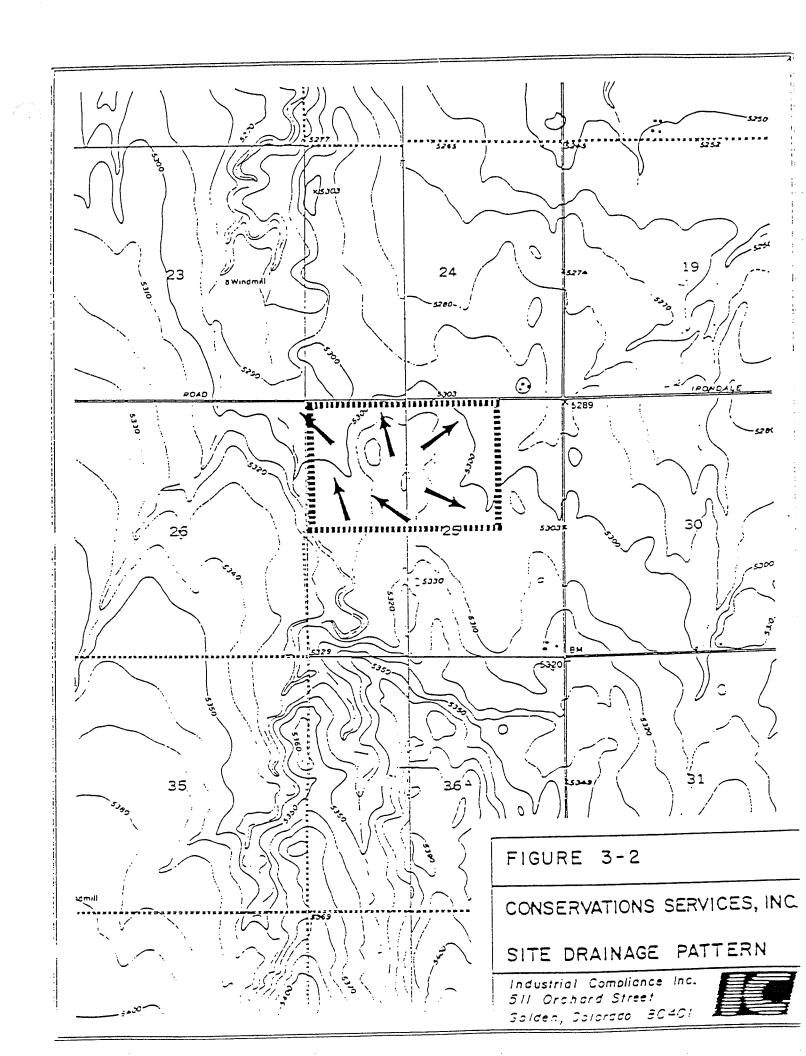
The general overland flow direction is from the west to the east in the eastern portion of the site and from the east to the west on the western third of the site. The site is used as farm land and was being plowed and seeded during site characterization activities.

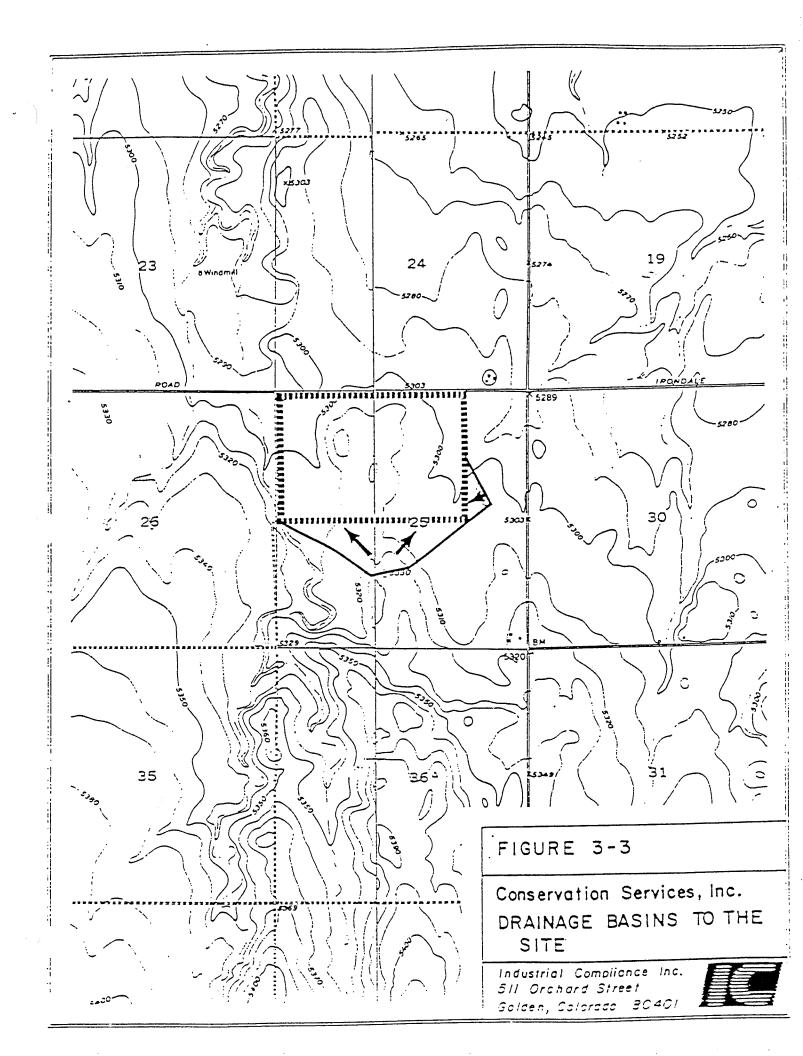
3.2 Surface-Water Drainage

Surface waters within a 2-mile radius are shown in Figure 3.1. The overland drainage from the site eventually flows in intermittent gullys to Lost Creek. The general drainage pattern for the site is shown in Figure 3.2. Lost Creek is an intermittent stream following a general south-to-north direction. Lost Creek is approximately 1.5 miles east of the site. Runoff from the eastern portion of the site will flow in intermittent streams that meet Lost Sand Creek approximately 1.5 miles northeast of the site. Runoff from the western portion of the site will flow in intermittent streams north until the stream meets Lost Creek approximately 8 miles north of the site.

There is no significant channelization on the site, with the exception of the very southwest corner of the property, nor is there any vegetation that would suggest any substantial volumes of water flowing through the site. The entire 240-acre site is currently farmed and water moves in the form of sheet flow. The drainage basins contributing to run-on in this area and to run-on to the site in general are shown in Figure 3.3.







The southwest corner of the site shows an intermittent swale. This swale eventually leads to a bridge under 88th Avenue west of the site boundary. This swale is shown to be part of a 100-year flood plain for a one-hour storm event. The 135 acres for their facility are not in the flood plain. Appendix B includes a drainage report for the area with a flood plain map. At the time of filling in the southeastern portion of the property, a temporary berm will be constructed to route runoff from the open cells. The berm on the southern property boundary will be a minimum of 3.5 feet tall to handle any potential 100- year storm events. As filling continues to other areas, the cell will be capped and closed and the berm will be removed.

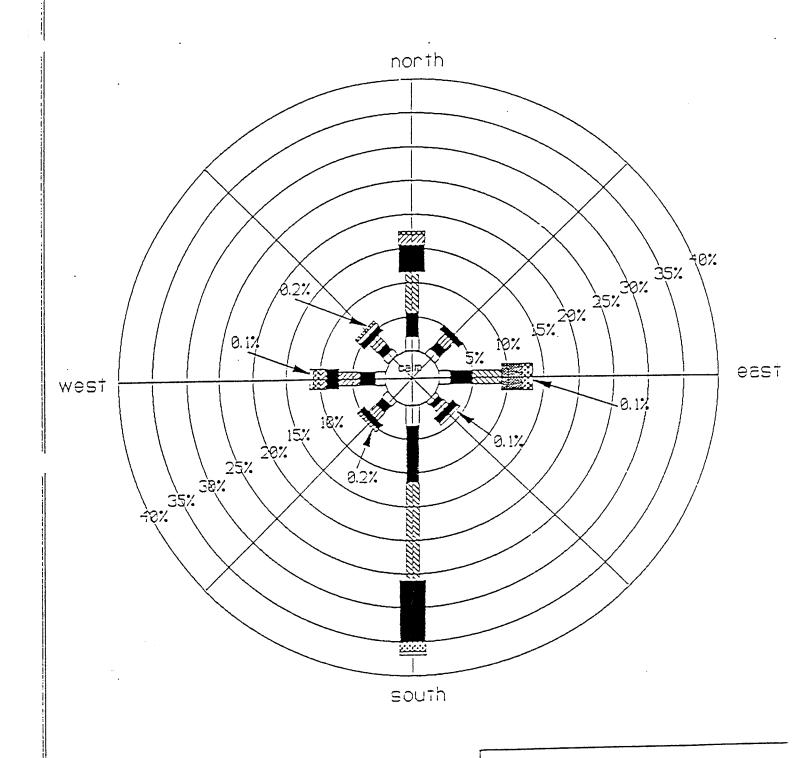
3.3 Climatology

Long term regional climatology data was obtained from records compiled by the National Oceanic and Atmospheric Administration (NOAA) and is typical of the Denver metropolitan region. The climatological summary for this area is presented in Appendix C of this report.

The average daily maximum temperature during the past 30 years is 64.3 degrees while the average daily minimum temperature is 36.2 degrees.

Appendix C includes climatology information regarding precipitation. Precipitation at the site is shown for 1985 and averaged from 1956 to 1985. The actual precipitation, as recorded by NOAA for 1985, is 16.31 inches with the greatest amount in a 24-hour period recorded as 1.57 inches in July. The maximum amount of rainfall in a 24-hour period was 3.55 inches in May 1973. The record annual mean precipitation from 1956 to 1985 is 14.62 inches. The greatest annual precipitation recorded from 1956 to 1985 is 23.31 inches in 1967.

The prevailing wind direction is from the south with the peak gusts from the west. Although the prevailing winds are from the south, the strongest winds are typically from the west and northwest. The average wind speed is approximately 9 miles per hour (mph) with the strongest gusts at 56 mph. A wind rose chart is included as Figure 3.4.



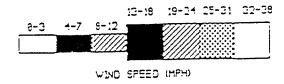


FIGURE 3-4

Conservation Services, Inc.

WINDROSE DIAGRAM

Industrial Compliance Inc. 511 Orchard Street Goiden, Colorado 20401



4.0 GEOLOGY AND HYDROGEOLOGY

4.1 Regional Geology

4.1.1 Regional Surficial Geology

The surficial materials found in the vicinity of the site include loess and eolian deposits and general pedisediments derived from the Rocky Mountain system. The eolion and loess deposits are loam, silty loam, or very fine sandy loam and contain a relatively large percentage of very fine sand. Tertiary and Pleistocene sediments originating at the eastern edge of the Rocky Mountain system are common in this region. These deposits are variable in texture, but they are dominantly sandy loam, sandy clay loam, or clay loam. Strata of sand and variable contents of gravel are typical of these beds (Sampson and Baber, 1974).

4.1.2 Regional Bedrock Geology

Bedrock in the area of the site includes rocks from virtually every geologic age group. Figure 4-1 is a generalized stratigraphic column showing formational relationships in the region. The formations discussed in this report are limited to a section of Upper Cretaceous age sedimentary rocks including, from bottom to top, the Pierre Shale, Fox Hills Sandstone, the Laramie and Arapahoe Formations and the Denver Formation. Formations below these do not warrant consideration because of their isolation from surficial processes.

The Pierre Shale is considered the basal unit of the major bedrock-aquifer system in the Denver Basin. The Pierre Shale is a marine deposit consisting primarily of shales with some interbedded siltstone and sandstone. Unit thickness ranges up to 8000 feet in the study area. The Pierre is characterized by extremely-low permeabilities and impervious clay layers distributed throughout the formation. Because of its extreme thickness and low permeability, formations below the Pierre are unaffected by the natural and artificial surficial processes in the vicinity of the site and will not be discussed further.

The Fox Hills Formation is a shallow marine deposit lying above the Pierre Formation. It is a silty sandstone with some interbedded shale units. The thickness of the Fox Hills Formation varies between 50 and 300 feet across the Denver Basin.

The Laramie Formation is a coastal plain deposit overlying the Fox Hills Formation. The Laramie is a series of alternating sandstones, claystones, and shales, with coal seams frequently

38A	ROCK TYPE	TIHU	THICK NESS							
RY			1-20	SURFICIAL WEATHERED MATERIAL: salty, soundy, clay, light brown to light gray, weathered becomes						
TERTIARY	Tou		400'	DAWSON FORMATION: interbedded conglomerate, sandstone, and shale, light gray to yellow brown beds of pale green shale						
		TKd	300.	DENYER FORMATION: interbedded shale, claystone, siltstone, and sandstone, dark brown to gray						
	D Ks 700'			ARAPAHOE FORMATION: interbedded sandstone, conglomerate, siltstone, and shale, gray to yellowish brown						
		Κl	200,	LARAMIE FORMATION: claystone, shale, and sandstone, light gray to brown						
EDUS		Κſħ	50'	FOX HILLS FORMATION: silty sondstone with interbeddedshales, gray to brown.						
CRETACEOUS		Кр	6000.+	PIERRE SHALE: shale, sandy shele, and some sandstone/siltstone lenses near the top, brown to dark grey-brown, Irlable						
		Κn	350'	NIOBRARA FORMATION: colocarcous shale and limestone, gray to grayish- yellow						
		Kαj	450°	CARLILE SHALE: silty claystone and stitstone, olive-gray, GREENHORN LIMESTONE: limestone, claystone, and siltstone, gray GRANERCS SHALE: siltstone and claystone, dark gray						
		Kđ	230'	DAKOTA GROUP: sandstone siltstone, carbonaceous shele, and conglomerate near the base light to dark oney						
JRASSIC		Jmr	380.	MORRISON and RALSTON CREEK FORMATION: snale, claystone, sancatone, and some thin limestone beds, varied colors,						
JURA		Jе	50.	ENTRADA FORMATION: sondstone, light brown to grey						
TR		TRPI	470°	LYKINS FORMATION: shale, siltstone, and limestone, red to red brown,						
PERM		PI	230.	LYONS SANDSTONE: sandstone, orange to plink to gray,						
	۵ و 0	PPſ	900,	FOUNTAIN FORMATION: sancatone, siltstone, and conglomerate, with thin shale beds, red-brown,						
<u>8</u>				Undifferentiated Precembrian igneous rooks						
				%						

FIGURE 4-1

Conservation Services, Inc.

GENERALIZED STRATIGRAPHIC COLUMN

Industrial Compliance Inc. 511 Oranged Street



occurring in the upper portion of the formation. The Laramie Formation varies in thickness from 400 to 600 feet across the Denver Basin.

Overlying the Laramie Formation are the fluvial deposits characteristic of the Arapahoe Formation. The Arapahoe consists of a series of interbedded conglomerates, sandstones, siltstones, and shales. Unit thickness varies between 400 and 700 feet across the Denver Basin.

The Denver Formation is found immediately beneath the surficial materials in the study area. The Denver Formation is a Tertiary Upper-Cretaceous age continental-type deposit characterized by interbedded shale, claystone, siltstone, and sandstone. Lignite lenses are found in abundance throughout the Denver Formation. Unit thickness ranges between 200 and 1000 feet across the entire Denver Basin.

4.1.3 Regional Geologic Structure

The landfill is located approximately 35 miles east of the structural axis of the Denver Basin. The basin is asymmetric, extending from around Pueblo, Colorado on the South, to Torrington, Wyoming on the North. On the western flank of the basin, the beds are near vertical at the contact with the Front Range, with a rapidly decreasing dip to the east. The town of Byers is at the approximate eastern boundary.

The beds near the landfill are on the eastern flank of the Basin, so they generally strike north-south and dip gently to the west at approximately 1 degree (Bryant and Others, 1981). The area is east and south of the most structurally complex region of the Denver Basin. The nearest major folds or faults are northwest of Broomfield, Colorado, approximately 35 miles northwest of the site. No indication of smaller-scale folding or faulting was found during the site-specific investigation.

4.1.4 Regional Economic Geology

Oil wells are located approximately 0.25 miles west and 0.25 miles northeast of the site boundaries. According to maps produced by the Adams County Planning Department (1983) and the U.S. Geological Survey (1977), additional wells are located about one mile south-southeast of the site. These wells are associated with the Radar and Bear Gulch oil fields. Wells are also located approximately one mile northeast of the site, apparently associated with the Chieftain and Hawkeye fields. Natural gas

fields exist approximately two miles east and four miles west of the site. Two high pressure gas lines, consisting of 18 and 24 inch diameter pipes, trend northeast-southwest through the southeast corner of the site.

The Denver Formation contains lignite to subbituminous coals that were mined through the early 1950's to satisfy local heating needs. The mining activities were concentrated in Boulder and Jefferson counties. According to the Adams County Planning Department (1983), the boundary of the Denver Formation coals in Adams county occurs approximately 5 miles southwest of the site. The Scranton mine is the nearest inactive coal mine in the area and is located near Watkins, about eight miles southwest of the site.

The site exists at least four miles outside any defined sand and gravel alluvial deposits. According to the Adams County Planning Department (1983), the nearest "probable aggregate resources" of sand and gravel occur approximately six miles east of the site along Kiowa creek and six miles west of the site along Box Elder Creek. Both of these deposits are identified by the CGS as valley fill material.

4.1.5 Geologic Hazards

The nearest major folds or faults are approximately 35 miles northwest of the site. In addition, no apparent geologic hazards that would affect the development of the facility are associated with the site. As stated previously, no underground coal mining activity has occurred within the immediate vicinity of the site. Historically, minor earthquakes in the area of the Rocky Mountain Arsenal (RMA) have occurred; however, this activity has not resulted in land surface disturbances. The earthquakes have been attributed to deep well injection of hazardous fluids on the RMA property, located approximately 20 miles west of the study area (Hollister and Weimer, 1968). The fluids were injected to depths of 12,000 feet. Earthquake activity subsided several years after the cessation of fluid injection in 1966 and it appears that this activity does not pose a threat to the development of the landfill.

The Adams County Soils Survey (Sampson and Baber, 1974) reports that the soils present at the site have moderate shrink-swell potential. Soil samples collected at the site were tested in a geotechnical lab to determine the actual shrink-swell potential of the soils. The results indicate that the soils have a low or low to moderate shrink-swell potential that will not affect the

operations at the site. Testing results are included in Appendix D. Although this factor will not be important during the operational phase, it will be considered for all post-closure land uses.

4.2 Site Geology

4.2.1 Field Investigations

A total of seventy-seven soil borings (SB-1 through SB-46, P17-P36, and MW-101-MW302) were drilled in Section 25; Township 2 South, Range 64 West of the 6th Principal Meridian in Adams County, Colorado. These soil borings were used to assess the geologic and hydrologic characteristics of the subsurface materials at the site.

The initial 19 soil borings (SB-1 through SB-19) were drilled as part of the exploratory field program. This program was initiated to provide the geologic and hydrologic information necessary for the selection of an appropriate site. Based on the preliminary information, a site encompassing approximately 240 acres was selected in the northwestern portion of Section 25.

Once the 240 acre site boundary was defined, a more thorough subsurface investigation was conducted. Twenty-seven additional borings (SB-20 through SB-46) were drilled during this investigation to further characterize the geology and ground-water hydrology within the site boundary. Borings SB-5, SB-12 through SB-16, SB-18, and SB-19, were drilled outside of the site boundary as part of the site selection process and are not included.

Following the initial forty-six borings, a conceptual site layout (Plate 1) was produced and CSI was granted conditional approval of the site. As a condition of approval, an additional 20 soil borings and piezometers (P-17 through P-36) were installed to characterize conditions beneath disposal cells 3, 4, 7, 10, 14, 16, 17, 22, 9, 12, and 20. Eleven additional ground-water monitor wells (MW-101 through MW-302) were also installed. The piezometers were installed at appropriate locations to suitably define the vertical separation between the lowest point in each cell and the uppermost saturated materials. The locations of the borings and monitor wells are shown on Plates 2, 3 and 12.

The soil borings were advanced with a truck-mounted auger rig using 4-inch diameter solid-stem or 6-inch diameter hollow-stem augers. The depths of the soil borings ranged from 15 to 101 feet. Relatively undisturbed soil and bedrock samples were obtained using a 2-inch Modified California Barrel Sampler or a

2-inch Split Spoon Sampler driven 18-inches into the soil by a 140 pound hammer dropping 30-inches. In addition, continuous, relatively undisturbed soil and bedrock cores were obtained from selected boreholes using a Central Mining Equipment (CME) continuous sampling system. The soil and bedrock samples, auger cuttings and cores were visually examined and logged in the field. The soils were classified in accordance with the Unified Soil Classification System (USCS). Lithologic and completion logs of the soil borings are presented in Appendix F. Actual field logs are included in Appendix D.

Selected soil/bedrock samples and bulk composite samples were submitted to a soils laboratory to evaluate the physical and engineering characteristics of the material. The results are summarized in section 4.2.4.

4.2.2 Site Soil Classification

The soils at the site have been mapped by the U.S. Soil Conservation Service (Sampson and Barber, 1974). However, the SCS states that the available soils information is highly generalized and may not accurately reflect local variations of the existing conditions. Moreover, this information is only relevant to a maximum depth of 60 inches.

The existing soil association is the Weld-Adena-Colby. This association formed on nearly-level to steep, well drained upland areas in loamy wind-deposited material. Sampson and Barber (1974) show the site to contain Ascalon, Weld, and Vona series soils, as well as loamy alluvial land. These soils are classified as mollisols and aridisols. Approximately 50 percent of the site (central) is covered by Ascalon-Platner association soils. The west and west central portions of the site are overlain by loamy alluvial sand and Vona loamy sand (3 to 9 percent slopes) respectively. The eastern portion of the site is covered by Ascalon sandy loam (1 to 3 percent slopes), Ascalon sandy loam (3 to 5 percent slopes), and Weld loam (1 to 3 percent slopes).

Ascalon series soils, including the Ascalon-Platner association, are well drained and are found in nearly level to moderately sloping upland areas. These soils typically form in loamy material containing varying amounts of sand and gravel. The surface is usually non-calcareous; however, visible lime splotches are found below depths of about 20 inches. These soils absorb water at a moderate to rapid rate. The available water capacity is high and surface runoff is low to medium. Typically,

30 to 50 percent of the material will pass through a #200 sieve. The field infiltration rate ranges from 4x10⁻⁴ to 1x10⁻³ cm/sec. The available water capacity varies from 0.13 to 0.15 inch/inch of soil. The water erosion hazard is moderate, the soils have low to moderate shrink-swell potential, and the soil blowing hazard is severe in unprotected areas.

Vona series soils are found on well drained, nearly level to moderately sloping upland areas. These soils form in wind deposited sandy material. In a representative profile, the surficial material is a non-calcareous loamy sand. The underlying material is a calcareous sandy loam with visible lime splotches. These soils absorb water rapidly, the available water capacity is moderate, and the surface runoff is medium. Typically, 20 to 40 percent of the material will pass a #200 sieve. The field infiltration rate varies from about 4x10⁻³ to 1x10⁻² cm/sec. The available water capacity ranges from 0.1 to 0.12 inch/inch of soil. These soils have a low to moderate shrink swell potential, and a severe soil blowing hazard.

Weld series soils are found in well drained nearly level upland areas. They typically form in wind-worked loamy material. The surface is a non-calcareous loam while the lower profile is a calcareous sandy loam with visible lime splotches. These soils absorb water at a moderate rate and the available water capacity is high. Surface runoff potential is medium. In Weld series soils, 60 to 90 percent of the material typically pass a #200 sieve. The field infiltration rate ranges from approximately 4×10^{-5} to 1×10^{-4} cm/sec and the available water capacity varies from 0.15 to 0.17 inch/inch of soil. These soils have a moderate to severe water erosion hazard, a high shrink-swell potential, and the soil blowing hazard is severe in dry-land farm areas receiving less than normal annual precipitation.

Loamy alluvial soils are found in long drainages with areas of deep silty deposits on slopes of less than 3 percent. The surface layer is usually a non-calcareous loam or clay loam 6-10 inches thick. The underlying material is a stratified silt or clay loam with varying amounts of sand and gravel. This soil absorbs water at a moderate rate, is well drained, and has a high water capacity. The water erosion hazard is severe in all cultivated areas.

4.2.3 Site Surficial Materials

The unconsolidated materials encountered during the drilling program can be classified into three categories: silty-sandy clay; silty sand; and gravel. Each of these materials are discussed below.

A light to dark brown silty, very-fine sandy clay was identified in all of the site soil borings. This deposit occurs in thicknesses that vary from 2 to 40 feet, with an average thickness of 17.2 feet. The thickest deposits generally occur throughout the central and northwestern portion of the site. This material was classified by visual field inspection (according to USCS) as CL/ML/SC. The field moisture contents ranged from dry to medium moist and the consistency ranged from medium stiff to very stiff.

A light-yellow-brown to light-gray silty, very-fine sand with varying amounts of clay was identified in 49 of the soil borings. Although these sand deposits occur sporadically, they appear to be more concentrated at the eastern portion of the site. These deposits are discontinuous and generally occur immediately above or below the silty-sand clay, interbedded within the silty-sandy clay or the claystone. The silty sand deposits vary in thickness between 1 and 19 feet, with an average thickness of 7.6 feet. The thickest deposits generally occur in the eastern portion of the site and in the drainage along the western boundary in the area of MW-201. This material was classified in the field as SC/SM. The field moisture content ranged from dry to wet with a consistency ranging from medium dense to dense.

Gravel deposits ranging in thickness of up to 1-foot were identified along the western portion of the site in borings SB-1 and SB-4 and MW-201. The gravels were encountered at a depth of 17-feet in SB-1, 8-feet in SB-4, and 16-feet in MW-201. These deposits are believed to be associated with a poorly developed north-south trending ephemeral stream channel. This ephemeral stream channel is generally comprised of reworked surfical materials. These gravels are generally interbedded with, or immediately overlying, a silty-sandy clay material. The gravel material was classified in the field as SM/GP. Water was found in the gravel intervals at both soil borings.

Plates 2, 3, and 12 show the soil boring and geologic crosssection locations. Geologic cross-sections were constructed of the site and are shown in plates 4, 5, and 6. The depths and thickness of the surficial materials are displayed in the above plates.

4.2.4 Site Bedrock Deposits

The Denver Formation was the only bedrock unit encountered during the site-specific drilling program. This formation was identified by cuttings obtained during drilling that are representative of Denver Formation lithologies typically found in the metropolitan Denver area. The lithologies encountered at the site consisted of claystones with discontinuous lenses of moderately cemented sandstone and unconsolidated sand.

Claystone is the dominant bedrock material at the site. claystone bedrock was encountered in all the soil borings drilled at the site, with the exception of SB-39 which was a shallow boring (less than 25 foot in depth) and monitor wells MW-102 and MW-201 (24-feet and 37-feet deep respectively). The depth to the claystone bedrock range from 1 to 43 feet, with an average depth of 18.6 feet. The claystone varied from light brown (weathered to slightly weathered zone) to olive, green-gray to gray (unweathered zone). The claystone was generally dry to slightly It is typically medium to very plastic and very stiff to hard when drilled. Iron oxide staining was observed in all of the soil borings. Gypsiferous infilling was also observed, but at a lesser extent than the iron staining. In addition, the claystone occasionally contained interbedded lenses of sand, sandstone, siltstone, shale and lignite. These lenses generally varied in thickness from less than an inch to approximately 4 feet.

Thin lenses of weak-to-moderately-cemented silty, fine-grained sandstone were logged in 25 of the soil borings. The sandstone ranged in thickness from approximately 6-inches to 13 feet and occur as discontinuous lenses. The sandstone generally contains some silt. The color ranges from light to dark brown. All the moderately cemented sandstone lenses encountered were unsaturated.

Forty borings contained isolated lenses of sand intercalated within the predominant claystone. These lenses generally contained light to yellow brown to light gray silty, fine sand. The lenses range in thickness from less than a few inches to approximately 15 feet. The thickest sand intervals occurred in

the northeastern and southeastern portions of the site. In many cases, the sand lenses within the claystone unit were saturated. These sand layers could not be traced laterally between borings and are considered to be discontinuous lenses.

The general characteristics of the bedrock material are shown in the geologic cross-sections (Plates 4, 5, and 6). These plates show the silty sand and sandstone lenses to be laterally and vertical discontinuous at the site.

4.2.5 Representative Material Properties

Representative materials from the site were collected and geotechnical testing was completed to determine physical and engineering characteristics of soils at the site. Samples composited from auger cuttings ranging in depths from 10 to 30 feet were collected from borings SB-32, SB-37, and SB-41. These materials generally consisted of the surficial silty, sandy clay and weathered to non-weathered claystone bedrock. The laboratory testing included soil classification (sieve analysis and Atterberg limits), Standard Proctor tests (ASTM D-698), remolded permeability tests and swell-consolidation tests. The results of the laboratory testing are presented in Table 4-1 and on individual test data sheets in Appendix D.

The composite materials exhibit very low permeabilities. These were recompacted to 95 percent of Standard Proctor Density and permeabilities averaged 3.8x10⁻⁸ cm/s. This suggests that the surficial and bedrock materials are suitable for the construction of the low permeability liners and caps associated with the disposal cells.

4.3 Regional Hydrogeology

4.3.1 Regional Ground-Water Basin

The site is located in the southern portion of the Lost Creek Ground-Water Basin. According to Nelson and others (1967), the Lost Creek Basin includes approximately 420 square miles in eastern Weld County, central Adams County and northern Arapahoe County. The important water-bearing geological members in the Lost Creek Ground-Water Basin include the Lost Creek Alluvium and sands of the Laramie and Fox Hills. Generally, the aquifers above the Fox Hills sandstone; including the Laramie, Arapahoe and Denver Formations, provide limited water supplies to stock

TABLE 4-1
GEOTECHNICAL LABORATORY RESULTS

Material Classification

	Sample	Atterbe	rg Limits		e Analys el Sand	
Soil <u>Boring</u>	Interval (feet)	Liquid <u>Limit</u>	Plasticity Index	(%)		Clay _(%)_
SB-32 SB-37 SB-41	15-25 10-30 10-25	44 46 45	6 19 25	0.0 0.0 0.0	10.0 4.6 2.6	90.0 95.4 97.4

Moisture Density Relationships

Soil <u>Boring</u>	Sample Interval <u>(feet)</u>	Maximum Dry Density(PCF)	Optimum Moisture Content (%)
SB-32	15-25	103.5	18.5
SB-37	10-30	102.5	18.5
SB-41	10-25	103.0	19.0

Material Permeability

Soil Borings	Sample Interval <u>(feet)</u>	Permeability (cm/s) (Remolded to 95% Standard Proctor Density)
SB-32	15-25	2.71x10 ⁻⁸
SB-37	10-30	5.12x10 ⁻⁸
SB-41	10-25	3.47x10 ⁻⁸

and domestic wells within the area. Since these aquifers are erratic, they are not considered large water supply sources within the Basin. The general characteristics of these aquifers are discussed in greater detail in Section 4.3.2.

The boundary of the Lost Creek Alluvium is the line of zero-saturated thickness of the Alluvium and the boundary of the Basin where the saturated thickness is greater than zero. The horizontal boundary of the sands of the Laramie and Fox Hills Formations is congruent with the boundary of the Basin. The vertical boundaries are the bottom of the Fox Hills sands and the top of the Laramie sands.

The principal source of ground water in the Lost Creek basin is the unconsolidated materials. The unconsolidated sediments include upland deposits, valley-fill deposits, and dune sand. The valley-fill deposits are the most important source of ground water for large capacity wells within the basin. These deposits which primarily consist of sand and gravel are generally thickest in the northern portion of the Basin.

4.3.2 Regional Aquifer Distribution

Three major ground-water aquifer units occur beneath the CSI landfill site; the Laramie-Fox Hills, the Arapahoe, and the Denver aquifers. These aquifers are discussed in detail below.

4.3.2.1 Laramie-Fox Hills Aquifer

The Laramie-Fox Hills Aquifer occurs in the lower sandstones of the Laramie and the upper part of the Fox Hills Formations. These sands appear to be hydraulically connected, so they are considered one aquifer. Near the site, the depth to the top of the aquifer is approximately 1200 feet with an average thickness of around 160 feet. In areas where the units are completely saturated, the Laramie-Fox Hills Aquifer is considered a confined unit. This is the situation beneath the site.

Insufficient water level data in the central part of the Denver basin, including the vicinity of the site, does not permit definition of the potentiometric surface in this area. Based on indirect data, Robson and Romero (1981) suggest that the direction of ground-water flow is towards the north-northeast. Figure 4-12 shows the potentiometric surface of the Laramie-Fox Hills Aquifer in 1978. Water derived from this unit is generally of good chemical quality and is characterized as a sodium bicarbonate.

4.3.2.2 Arapahoe Aquifer

The Arapahoe Aquifer occurs in the sandstone and conglomerate beds characteristic of this unit. The depth to the top of the Arapahoe Formation is approximately 450 feet with a total thickness of the sandstone and conglomerate beds ranging up to 150 feet in the vicinity of the site. The Arapahoe is also considered a confined aquifer beneath the site. Robson and Romero (1981) placed the potentiometric surface elevation of the Arapahoe Formation in the immediate vicinity of the site at 5175 feet with a predominant flow direction to the northwest. Figure 4-13 shows the potentiometric surface of the Arapahoe Aquifer in 1978. The chemical quality of water derived from the Arapahoe is generally classified as a sodium bicarbonate type.

4.3.2.3 Denver Aquifer

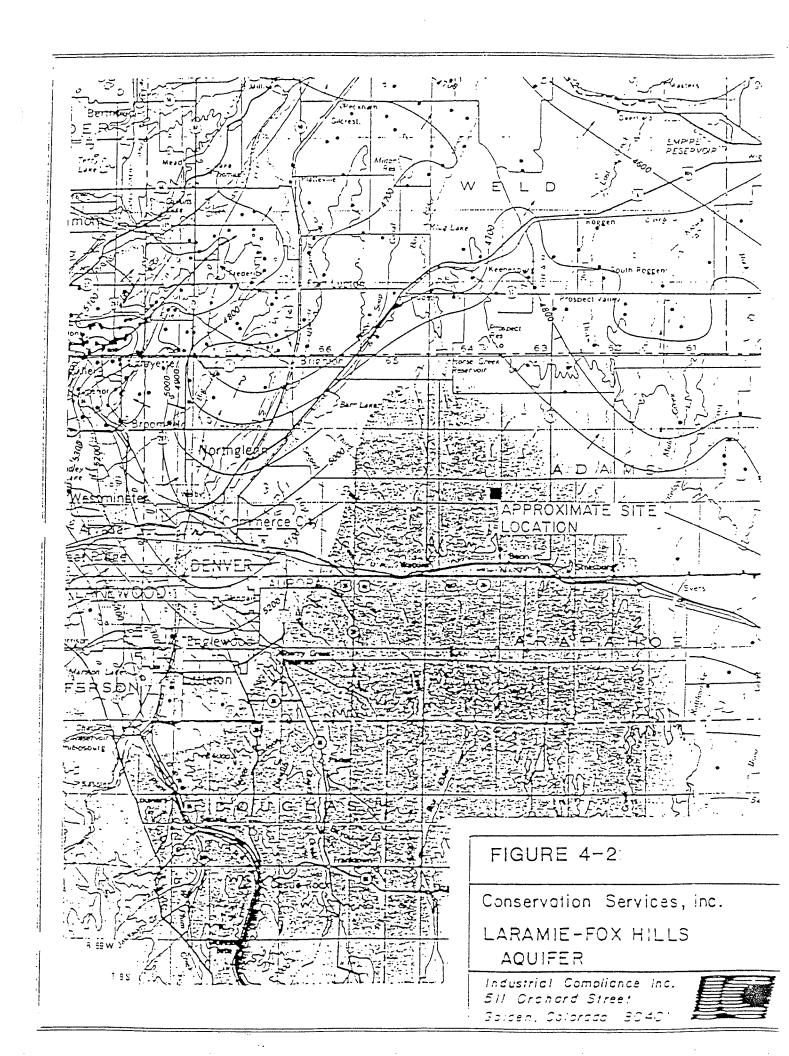
The Denver Aquifer is the uppermost aquifer beneath the landfill. Robson and Romero (1981) indicate less than 50 foot of saturated thickness. The base of the water-bearing strata is estimated to be 160 feet below land surface. The water bearing strata are described as irregular lenses of interbedded sandstone and siltstone that are hydraulically separated by thick sequences of claystone. Robson and Romero (1981) show the potentiometric surface elevation to be approximately 5250 feet, with groundwater flow towards the north (Figure 4-14). Wells in the Denver Aquifer generally yield from 0.05 to 1.0 gallons per minute per foot of drawdown.

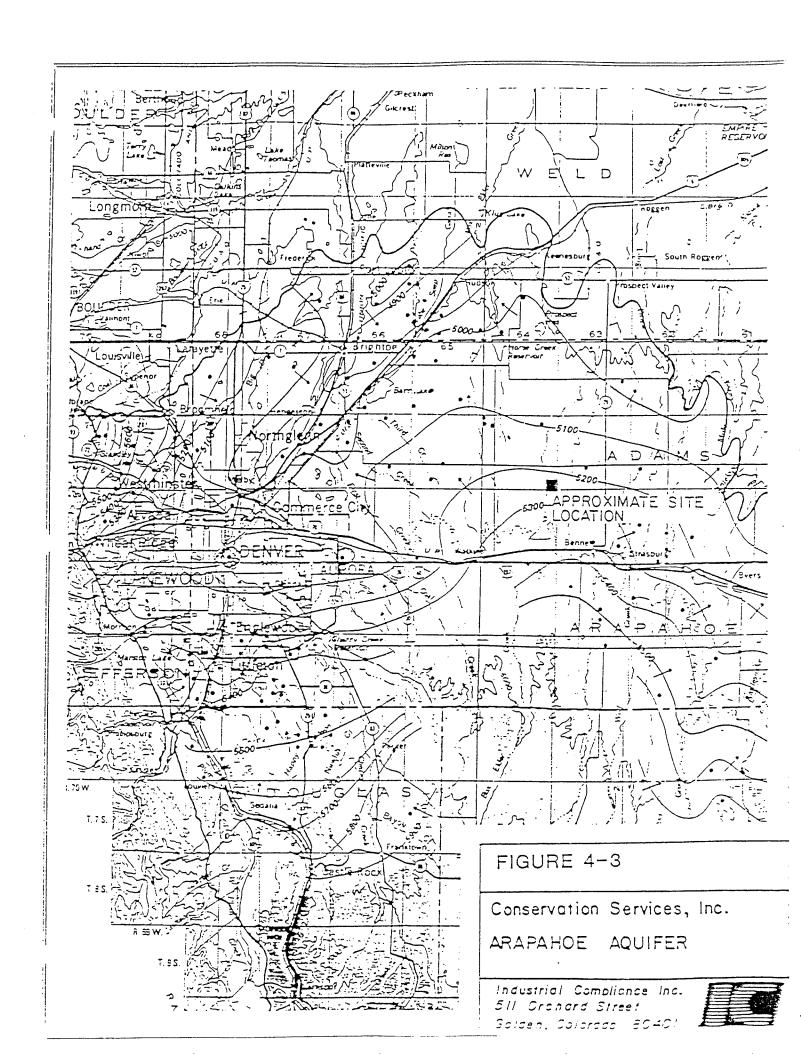
In general, the Denver provides water of acceptable chemical quality source, although it is not as good as water derived from the Arapahoe or Laramie-Fox Hills aquifers. The water is classified as either sodium sulfate or sodium bicarbonate type.

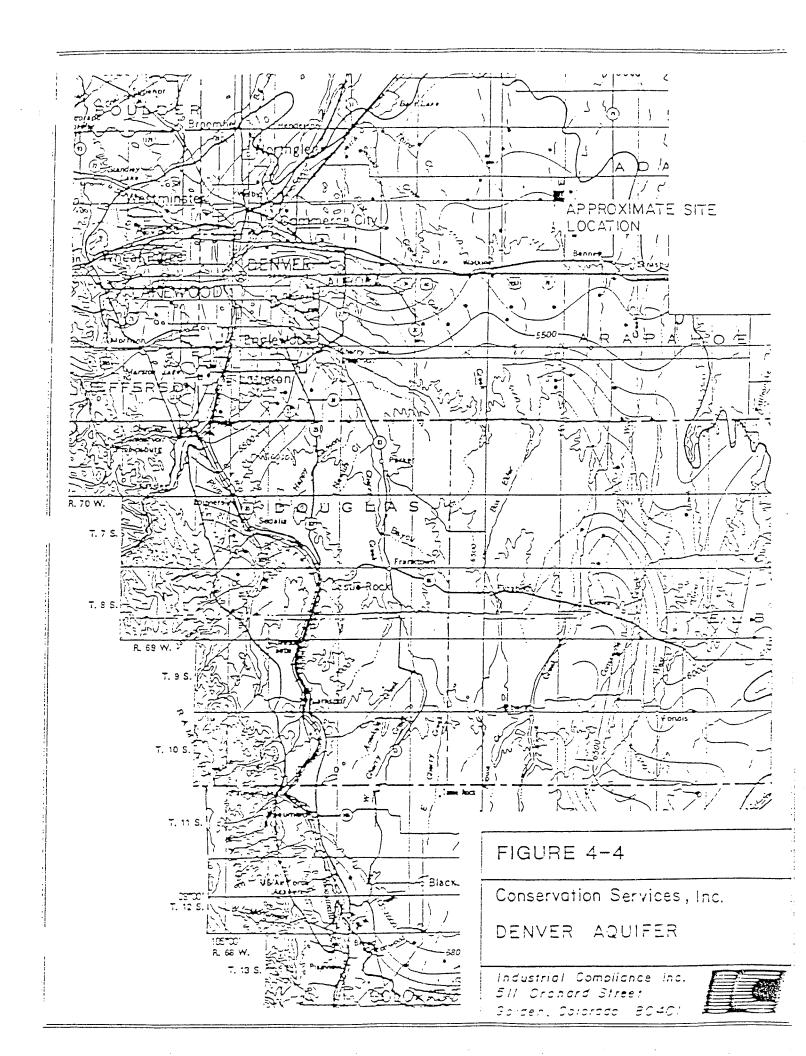
4.4 Site Hydrogeology

4.4.1 Water Well Inventory in the Vicinity of Site

Records on permitted ground water wells within a 2 mile radius of the site were reviewed at the Colorado Division of Water Resources, State Engineers office. The data examined included well locations to the nearest quarter section, water usage designation, total depth, water level, and approximate well yield. The information is presented in Table 4-2. Based on the limited information contained in the State files, it is not known if all of the listed wells are currently in existence and/or operation.







Most of the registered water wells were completed in the Arapahoe Aquifer. These are domestic and /or stock use wells that are listed as producing an estimated maximum yield of 10 to 20 gallons per minute. In addition, two shallow wells, ranging from 60 to 80 feet in depth were identified west and northwest of the site, respectively. These wells were completed as windmills.

4.4.2 Field Investigation

Sixty-nine of the seventy-seven soil borings were drilled within the CSI site boundary to assess the hydrogeologic characteristics of the subsurface material. The borings range from 16 to 101 feet in total depth. Thirteen soils borings were completed as monitoring wells, and thirty-six were completed as piezometers at the locations shown on Plates 2, 3 and 12. The monitoring wells vary from 16 to 90 feet in depth. The piezometer depths varied from 20 to 99 feet.

The monitoring wells were constructed with 2-inch, threaded, flush-jointed, polyvinyl chloride (PVC) pipe with 0.010-inch machine slotted, 5-foot or 10-foot screen sections. The annular space around the screen was backfilled with #10-20 Colorado Silica Sand. A two to three foot bentonite plug was placed on top of the sand. The remainder of the boring was grouted to the surface using a cement-bentonite mixture. Each well was secured at the surface with a locking steel protective casing. The monitoring well installation details are presented in Appendix #10-20. The piezometers consisted of 3/4-inch or 1-inch diameter PVC pipe with hand-slotted screen intervals. The piezometer installation details are also presented in Appendix #10-20 Colorado

The monitoring wells were bailed to develop the well prior to sampling. The initial two monitor wells installed were labeled MW-1 and MW-2. These were relabeled as MW-101 and MW-104 respectively following installation of the remaining monitor wells. Only sampling of MW-101 (MW-1) and MW-104 (MW-2) are discussed here. The other wells have been sampled on a quarterly basis since September of 1989, and the results have been sent regularly to CSI, CDH, and Adams County. Ground-water samples were collected from the intial two monitoring wells and submitted to a contract laboratory for analytical analysis of selected parameters. In addition, field measurements of pH, specific conductance and temperature were obtained from the monitoring wells and at selected piezometers and soil borings at the site. The results are summarized in Section 4.4.4.

TABLE 4-2
WELLS WITHIN A TWO-MILE RADIUS OF THE SITE

LOCATION	<u>DEPTH</u>	AQUIFER	WATER LEVEL	YIELD *	<u>USE</u>
SW1\4 SE1\4 SEC 18	315	AR	92	12	3
SE1\4 SE1\4 SEC 18	315	AR	90	12	3
SW1\4 NE1\4 SEC 20	140	AR	95	20	2
NE1\4 NE1\4 SEC 31	208	AR	50	15	2
NE1\4 SW1\4 SEC 14	NA	NA	NA	8	1
SE1\4 NE1\4 SEC 23	80	υ	65	10	3
SE1\4 SE1\4 SEC 24	150	AR	90	15	3
NE1\4 NE1\4 SEC 26	60	U	NA	10	3
SE1\4 NE1\4 SEC 36	280	AR	140	15	1

U = UNCONSOLIDATED MATERIAL

AR = ARAPAHOE

NA = NOT AVAILABLE

1 = DOMESTIC

2 = STOCK

3 = DOM/STOCK

* = THESE VALUES REPRESENT THE ESTIMATED MAXIMUM YIELD.

Aquifer testing was performed at the site to evaluate the hydraulic properties of the site-specific materials. This was accomplished by conducting packer tests and rising-head tests. In addition, laboratory permeability tests were performed on selected soil and bedrock samples. The results are discussed in Section 4.4.5.

4.4.3 Ground Water Occurrence and Conditions

Ground water was encountered in 47 of the 77 soil borings drilled within the site boundary. The drilling investigation indicates that the depth to ground water varies from less than 10 to more than 80 feet. The soil boring logs (Appendix F) and geologic cross-sections (Plates 4,5 and 6) show the probable source lithology of water and depth to water encountered during the drilling program.

The ground water encountered at the site generally occurs within discontinuous sand lenses. The saturated material in these lenses is generally comprised of fine, silty sand. The saturated material either occurs immediately above the impervious claystone or as interbedded lenses within the claystone. The drilling program indicated that the perched lenses occur sporadically across the site. Furthermore, the saturated materials associated with these lenses appear to be isolated and discontinuous in both the horizontal and vertical direction. Some of these saturated lenses exist under semi-confined conditions as evidenced by P-17 and P-35. In P-17 a thin (<2-feet) water bearing interval was encountered at an elevation of approximately 5226'. Water subsequently was measured in this boring at an elevation of 5233'. To determine if the water measured in this boring originated from below 5233', P-35 was drilled adjacent to P-17 to an elevation of 5231' (below the water level in P-17 but above the water bearing zone). P-35 was dry at the time of drilling and has remained dry, indicating that the water in P-17 is under approximately 7-feet of head.

Shallow ground water was encountered along the western, northern and southern portions of the site. In the west, ground water is shallowest beneath a poorly-developed ephemeral stream channel. The north-south trending channel has little to no vertical relief and is currently being utilized for dry-land farming in the vicinity of the site. The ephemeral stream channel is comprised of reworked surfical materials. Soil borings SB-1, SB-4, SB-22, and MW-202 encountered water that ranged from 9.5 to 18 feet in depth. The ground-water in the area of SB-1, SB-4, and MW-202 was associated with thin (less than 1-foot thick) gravel/sand

lenses that exist above the claystone bedrock. The water in the vicinity of SB-22 was associated within a 7-foot silty sand interval, of which the lower 5-foot portion was saturated. This silty sand material also existed directly above the claystone bedrock. The ground water in this area is probably associated with a shallow alluvial aquifer that lies above the impervious claystone bedrock (See cross-section D-D', plate 5 and cross-sections E-E' and F-F', plate 6). This shallow alluvial aquifer appears to be confined to the western third of the site.

Shallow discontinuous lenses of ground water were encountered in the north at SB-38, SB-39, SB-40 and SB-44. The depth to water varied from approximately 17 to 20 feet in this area. The ground-water in the area of SB-38, SB-39 and SB-40 was associated with relatively thin (less than 5-foot thick) silty sand lenses that overlie the claystone bedrock. The claystone bedrock in this area is topographically lower than areas to the south (See cross-section B-B', plate 4). The water in the vicinity of SB-44 is associated with a thin (3-foot thick) silty sand lens, which is interbedded within the claystone. Ground-water encountered in MW-103, MW-104 and P-34 ranged from 23 to 34-feet deep and also originates in lenses within the claystone bedrock.

In the south, a shallow perched lense was encountered at SB-6 and MW-106 where the ground water was approximately 15 feet below the ground surface. The water in the area of SB-6 and MW-106 is associated with a relatively thick (greater than 15-foot thick) silty sand lense that overlies claystone bedrock. The claystone bedrock in this area is topographically lower in elevations than areas to the north (See cross-section C-C', plate 5).

Ground water was encountered in the southeastern portion of the site in the area of SB-25, SB-30 and SB-46 (adjacent to each other), and SB-42 ranged from approximately 40 to 50 feet in depth. P-22, which is at a slightly lower elevation than these other borings, was dry at a depth of 54-feet. The water in the area of SB-25 is associated with relatively thin isolated silty sand lenses that are interbedded within the claystone bedrock. The water in the vicinity of SB-30 and SB-46, and SB-42 is characterized as having isolated, relatively thick (greater than 15-foot thick) saturated silty sand lenses. The thick silty sand intervals also occur as interbedded lenses within the claystone bedrock. Cross-sections A-A' and B-B', plate 4 and cross-section D-D', plate 5 show that the water in this area occurs as discontinuous lenses.

Perched lenses of saturated material were encountered sporadically across the central portion of the site. This portion of the site also contained 19 borings that were dry at the time of completion. The borings ranged from 45 to 80 feet in total depth. The ground water, where encountered, varied from approximately 35 to 65 feet in depth. Samples collected during the field drilling program indicate the saturated material occurred in hydraulically-discontinuous, perched lenses (See geologic cross-sections, plates 4, 5, and 6).

Twelve monitoring wells and thirty-six piezometers were installed to assess the existing ground-water conditions within the saturated materials. The stabilized static water level in each monitoring well/piezometer has been measured periodically during the field investigation. Static water levels were also recorded from the open boreholes. The observed ground-water elevations for the piezometers are summarized in Table 4-3.

Revised ' ch 1991 Table 4.3

CONSERVATION SERVICES INC. PIEZONETER WATER LEVEL DATA

88 Vater Elevation	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	5261.3	5242.5	N/A	N/A	N/A	5255.5	V/N	Y/N	Y/N	Y/N	Y/N	Y/H	Y/N	K/X	* * * * * * * * * * * * * * * * * * *	¥/X	4/3	< /a	* /2	. Y/H	K/A		K/N	X/X	*	K/X	K/X	Y/X	X/X	N/A	×/×	K/A	×/×	X/X	N/A	K/A	
8/25/88 Depth to Water	ê V û ê \$ \$ 4 4 0 0 8 2 0	59.1	9.99	K/ X	DRY	DRY	42.3	N/N	DRY	X /X	V/X	K/X	*	DRY	Y/N	W/W	K/N	¥/N	(/ / /	*/ *	X/X	Y/X	× ×	X/X	N/A	N/N	N/A	X/X	X X	×/×	X/X	X/X	W/W	N/A	N/A	N/A	N/A	
726/88 Vater Elevation	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	N/A	5242.1	5286.2	N/N	N/A	5252.8	N/A	X/X	N/N	N/A	N/N	N/A	N/A	N/N	N/N	5313.2	N/A	X X	X/X	×.	× ×	K/X	N/N	N/A	K/X	K/X	∀ /¥	N/A	N/A	X X	N/N	K/X	K/X	N/A	N/A	N/A	
7/14/88 to 7/26/88 DEPTH TO WA	• • • • • • • • • • • • • • • • • • •	N/A	0.79	12.0	DRY	DRY	45.0	DRY	N/A	N/N	N/A	N/A	K/A	DRY	N/A	N/A	13.5	Y/X	××	*	N/N	K/A	K/X	K/X	X / X	K/N	N/A	¥,¥	K / A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	
NORTHING	1 1 2 0 1 1	8984.6	8951.3	8969.5	8113.7	8950.0	8122.3	8929.9	8301.2	N/A	9825.5	8646.9	7926.6	9588.3	N/A	7.0997	7435.1	9249.3	8900.2	8883.5	8515.7	8553.2	8104.7	8221.1	8003.0	7653.7	8600.1	7619.7	7900.5	8270.7	8617.3	9344.4	9383.8	9619.8	9862.4	9239.5	8/42.3	
EASTING	; ; ; ; ; ; ;	6506.1	0.123	24/4.5	6514.8	7243.5	8002.4	5815.8	7095.9	N/A	7248.6	6260.8	7590.4	5959.0	N/A	8339.4	6521.5	8677.0	7868.6	8553.1	8553.3	7910.7	8549.4	7802.6	7783.1	7777.3	7262.4	7180.2	7288.6	7260.7	6710.7	6720.3	7282.2	6722.9	6774.1	8675.9	4.00%	
717/90 TOP OF PVC	1 f D t			-			* ;																			5304.06		5509.70	5308.73					5311.37				
REVISED 5/11/90 SURFACE TOP (ELEVATION PVC																									1	5501.27		5207.55						5309.95				
TOP OF PVC		5322.00	2300.07	2200.22	15.4750	N/A	5299.18	5316.99	5316.44	N/A	5309.01	5319.78	5502.18	5515.85	N/A	5300.27	5329.20	5292.91	5303.90	5298.84	5294.15	5299.33	5298.09	5299.99	5505.44	5505.74	320%.74	22.01.59	2509.21	5510.35	5525.17	5315.34	5308.06	5311.77	5508.24	5202.49	1670.74	
SURFACE Elevation		5320.39	5000	17.047	2266.10	5309.12	5297.78	5515.59	5513.14	5304.00	5508.26	5519.25	5507.78	55.5.55	2591.00	5296.92	5326.70	5289.68	5300.16	5296.02	5290.88	5296.05	5295.66	5297.36	2301.04	2001.44	2207.00	7000	2200.40	5508.00	55.1.54	5512.62	5505.66	5309.96	5505.81	5265 63	16.50	
COMPLETION DEPTH	ş	2.8	0.00	0.0	12.0	0.7	0.04	29.0	2.1	0.5	3,	0.0	D., V	0 °	0.47	0.	0.04	0.69	65.0	2.0	0.4°	24.0	55.7	5.0°	4.0	4.4	200	1	0. V.	27.0		0,00	0.40	26.0)) (, v.c	-	
BORING (DEPTH	9	200		0,0	20.0) ·	, y .	0 % C	: :	0.07	0.05	0.5	0.0	0.00	3.5	0.00	40.0	0.69	65.0	0.43	3;	0.4°	0.47	, o	0.44	100		ָרָ בְּיִבְּיִבְּיִבְּיִבְּיִבְּיִבְּיִבְּיִ	200	9.0	9.0), V	0. 0.	2,0) . , , ,	5,0		1
CELL	0	<u>o 6</u>	V/N	, T	5 5	<u>.</u> c	• }	N/A	<u>.</u>	v .	N/A	<u> </u>	^ :	¥ •	-;	= ;	¥/¥	- - ,	v ŋ -	o† l	n `) ه	7,	~ c	, 5	2 7	<u>,</u>	2 %	ţ	<u>.</u> †	- 7	7 6	2 6	77	3 +	- M	,	
BORING	4	P-2	. . .	7-d	· · ·	7	2 6	- 6-0	٥	y - 0	2 -	- 4	7 7 7	7-0	<u> </u>	2;	7.0	- S	۲- اه	<u> </u>	r-70	7-1	P-22	2,5	5-25	3 %	0-27	22.0	200	0-20	2 2 2	22.4	25.4	2,7	מל מ	P-36		

P - Piezometer
DRY - No water present at the time of measurement.
N/A - Not Applicable
Plezometer P-18 was replaced by P-36 in April 1990
Piezometers P-1 through P-16 were abandoned July 1990.

Revised ch 1991 Table 4.3

	1/29/90 D WATER ELEVATION	5266.0 5242.6 5242.6 5285.7 527.3 5257.3 5257.3 5257.3 5257.3 8267.7 87.8 87.8 87.8 87.8 87.8 87.8 87.8
	1/2 DEPTH TO WATER	54.4 66.5 12.5 70.8 70.8 56.9 8/6.9 8/6.9 15.8 56.3 56.3 56.3 56.3 56.3 56.3 56.3 57.0 67.7 60.4 60.4 60.4 60.4 60.4 60.4 60.4 60.4
	4/10/89) UATER ELEVATION	5262.3 5285.9 52742.3 5285.9 527.2 5256.3 5255.4 N/A N/A N/A N/A N/A N/A N/A N/A N/A N/A
L DATA	TER TC	58.1 47.7.7 47.7.7.3 50.9 50.9 50.9 63.4 63.4 63.4 63.4 63.4 63.4 63.4 63.4
CONSERVATION SERVICES INC. PIEZOMETER WATER LEVEL DATA	2/28/89 3 WATER ELEVATION	5262.9 5241.1 5284.0 5274.4 N/A 5286.2 5257.7 5268.5 5268.5 5268.5 5268.5 5268.6 6 6 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7
NC. PIEZOMETI	2/2 DEPTH TO WATER	N N N N N N N N N N N N N N N N N N N
M SERVICES I	1/24/89 NATER ELEVATION	5262.4 5242.9 8242.9 8242.9 8242.9 8242.9 8242.9 8242.9 8255.8 8221.9 8249.8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8
CONSERVATION	1/ DEPTH TO VATER	58.0 77.17 77.17 77.17 77.18 77.18 77.18 77.19 77.
	9/22/88 0 WATER ELEVATION	5261.3 8286.3 8286.3 878.0 879.0
	9/2 DEPTH TO WATER	59.1 11.9 11.9 11.9 12.1 12.9 14.8 14.8 17.9 17.9 17.9 17.9 17.9 17.9 17.9 17.9
	9/2/88 VATER ELEVATION	5242.4 5242.4 N/A N/A N/A N/A N/A N/A N/A N/A
	DEPTH TO VATER	59.2 66.7 12.0 12.0 12.0 12.0 12.0 17.0
	BORING	P-11 P-22 P-23 P-23 P-23 P-23 P-23 P-23 P-23

P - Piezometer DRY - No water present at the time of measurement. N/A - Not Applicable Piezometer P-18 was replaced by P-36 in April 1990. Piezometers P-1 through P-16 were abandoned July 1990.

	B/17/90 WATER ELEVATION	N/A N/A N/A N/A N/A N/A N/A S234.1 S24.1 S256.4 S256.4 S256.8 S256.0 N/A N/A N/A N/A N/A N/A N/A N/A N/A N/A
	S DEPTH TO WATER	N/A N/A N/A N/A N/A N/A N/A N/A N/A N/A
	7/6/90 VATER ELEVATION	N/A N/A N/A N/A N/A N/A N/A S233.9 S236.4 5236.4 5236.4 5256.6 5256.6 5256.0 N/A N/A N/A N/A N/A N/A N/A N/A N/A N/A
DATA	DEPTH TO NATER	N/A N/A N/A N/A N/A N/A N/A N/A N/A N/A
CONSERVATION SERVICES INC. PIEZOMETER WATER LEVEL DATA	5/11/90 WATER ELEVATION	N/A N/A N/A N/A N/A N/A N/A S246.5 N/A N/A N/A N/A N/A N/A N/A N/A N/A N/A
IC. PIEZOMET	S DEPTH TO WATER	22.24AAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAA
ON SERVICES II	4/27/90 VATER ELEVATION	5266.0 5241.9 5288.0 5274.9 6278.0 87.4 87.4 87.4 87.6 87.6 87.6 87.6 87.6 87.6 87.6 87.6
CONSERVATI	DEPTH TO Water	56.0 67.0 12.6 49.8 8.3.4 8.9 8.9 8.9 8.3.3 8.9 8.3.3 8.3 8
	3/30/90 0 WATER ELEVATION	5233.4 N/A A A A A A A A A A A A A A A A A A A
	3/ DEPTH TO WATER	**************************************
	2/8/90 to 2/13/90 DEPTH TO WATER WATER ELEVATION	5265.6 5242.3 5290.2 N/A 5257.7 5256.7 8257.7 8256.7 5250.2 5267.6 8267.6 8244.9 5244.9 5244.9 5244.9 5244.9 5244.9 5244.9 5244.9 5244.9 5244.9 5244.9 5246.6 825.8 8242.6 8242.6 8242.6 8242.6 8242.6 8243.1 8256.6 8260.5 8260.5 8260.5 8260.5 8260.5 8260.7 8260.9
	2/8/90 DEPTH TO WATER	56.4 10.3 10.5
	BORING	823 4 3 3 3 3 3 5 5 5 5 5 5 5 5 5 5 5 5 5

P - Piezometer DRY - No water present at the time of measurement. N/A - Not Applicable Piezometer P-18 was replaced by P-36 in April 1990 Piezometers P-1 through P-16 were abandoned July 1990.

CONSERVATION SERVICES INC. PIEZOMETER WATER LEVEL DATA

2/12/91 NATER ELEVATION	N/A	N/A	N/N	Α'X	¥ \ \	¥ \ 2	X / Z	N/A	X X	N/N	N/A	N/A	N/A	N/A	N/A	5234.4	N/A	5239.2	5236.6	5246.8	N/A	5257.1	5261.0	5262.4	5247.6	N/A	5259.1	5256.4	N/A	N/A	N/A	N/A	5273.0	N/A	N/A						
2, DEPTH TO WATER	N/A	¥/¥	X/X	۷ : 2 :	X	(4/2	Y/N	(X X	N/A	N/A	N/A	N/A	N/A	N/A	58.5	N/A	26.6	57.6	52.5	DRY	45.9	45.5	41.7	62.1	DRY	49.7	54.0	DRY	DRY	DRY	DRY	35.3	DRY	DRY						
12/11/90 WATER ELEVATION	N/A	N/A	Α :	Α/X :	¥ / Z	Z / Z	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	5234.4	N/A	5239.2	5236.6	5246.8	N/A	5257.1	5261.0	5262.4	5247.6	N/A	5259.1	5256.4	N/A	N/A	N/A	N/A	5273.0	N/A	N/A						
12 Depth to Water	N/A	Α/X :	N/A	A < 2	V / N	(A / N	A/N	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	58.5	N/A	59.6	57.6	52.5	DRY	45.9	45.5	41.7	62.1	DRY	46.7	54.0	DRY	DRY	DRY	DRY	35.3	DRY	DRY						
11/14/90 WATER ELEVATION	N/A	A :	K .	4 × ×	V / N	N/A	N/A	××	N/A	N/A	N/A	N/A	N/A	N/A	N/A	5234.3	N/A	5238.9	5236.5	5246.8	N/A	5256.9	5260.7	5262.2	N/A	N/A	5259.0	5256.2	N/A	N/A	N/A	N/A	5272.6	N/A	N/A						
11 DEPTH TO WATER	N/A	۷ ; 2 :	4/x	4 × ×	(V X	Υ N Α/Ν	N/N	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	58.6	N/A	59.9	2.72	52.5	DRY	43.1	42.7	41.9	DRY	DRY	2.65	54.2	DRY	DRY	DRY	DRY	35.6	DRY	DRY			•	Ō	1990.	
10/6/90 Water Elevation	N/A	A/N	¥ / X	X/X X/X	N/A	X/N	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	5234.3	N/A	5238.9	5236.6	5246.8	N/A	5256.9	5260.6	5262.2	N/A	N/A	5258.8	5256.2	N/A	N/A	N/A	N/A	5272.4	N/A	N/A		the time of meserment	ו וווכמסתו כוווכו ור	was replaced by P-36 in April 1990	were abandoned July 1990.	•
10 DEPTH TO VATER	N/A	4 ×	X	X \ X \ X	X / X	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	58.6	N/A	29.9	57.6	52.5	DRY	43.1	42.8	41.9	DRY	DRY	6.64	54.5	DRY	DRY	DRY	DRY	55.8	DRY	DRY		+	,	aced by P-36	P-16 were ab	
9/5/90 NATER ELEVATION	N/A	¥ \ X	4 / 2	X / X	N/A	N/N	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	5234.0	N/A	5239.0	5236.5	2546.7	N/A	5256.7	5260.5	5262.3	A/8	A/A	7238.4	0.0626	K/A	¥ :	N/A	N/A	5272.1	N/A	N/A	± 0	ter present a	cable	P-18 was repl	P-1 through	
9 Depth to Vater	N/A	¥	(o	(\ X	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	58.9	N/A	26.8	57.7	52.6	DRY	45.3	6.24	8.1.4	DRY	י ארן ארן		74.5	URY	DRY	טאי היי	DRY	20.1	DRY	DRY	D - Diazometer	· >			Piezometers P-1	
BORING	P-1	7.7	7-4	- 4	9-6	P-7	P-8	P-9	P-10	P-11	P-12	P-13	P-14	P-15	P-16	P-17	P-18	P-19	P-20	P-21	77-d	P-23	P-24	 	7-70 07-4	P-2/	P-20	7-79			P-52	P-55	7.0	P-55	P-36				et		

The ground-water elevations are extremely variable across the site, ranging from as high as 5313 feet at SB-6 to below 5217 feet at SB-45. The geologic cross-sections (Plates 4, 5 and 6) show that steep ground-water gradients exist from one soil boring/piezometer to another, suggesting that the saturated materials encountered generally occur as hydraulically-discontinuous perched lenses across the site. Ground-water elevations were plotted to identify ground-water trends at the site, however, upon geologic review, no continuous ground-water trends could be ascertained. Therefore, a potentiometric map was not constructed.

Piezometer P-2 may be completed within the regional Denver Aguifer. Based on the regional hydrogeologic information available (Section 4.3.2.3) this is a plausible situation. Piezometer P-2 was completed to a depth of 99 feet (an elevation of 5212 feet). Saturated sands were encountered while drilling at a depth of approximately 89 feet. The water elevation at the completion of the piezometer was approximately 5242 feet. Subsequent static water measurements show that the stabilized water level elevation is approximately 5244 feet. This indicates that P-2 exhibits a potentiometric elevation head of over 20 feet, suggesting the ground water to be under confined conditions. Piezometer P-5 was installed near P-2 to a depth of 79 feet (elevation of 5232 feet) to verify that the material above this depth was unsaturated. This piezometer was dry at the time of completion and during subsequent water level measurements. Based on this information, the ground water in P-2 cannot be attributed to upper leakage within the borehole. Piezometer P-14, was installed to the east at a depth of 74 feet (an elevation of 5217 feet) was dry at the time of completion and during subsequent water level measurements. This piezometer was completed 5-foot below the elevation depth where P-2 encountered ground-water (see cross-section E-E', plate 6). Piezometer P-14 contained no sandstones, so the difference probably results from the erratic sands that comprise the Denver Aquifer. Soil boring SB-33, which is located between P-2 and P-14, was drilled to a depth of 80 feet (elevation of 5217 feet). The borehole was completed to the same elevation as P-14. However, this boring was dry at the time of completion but contained measurable amounts of water a few days later. This suggests that the water was probably a result of upper leakage within the borehole from thin, slightly saturated materials, which are not associated with the water observed at P-2 (see cross-section E-E', plate 6). Monitor wells MW-301 and MW-302 are also likely completed within the Denver Aquifer and both appear to contain a significant quantity of water under confined conditions.

4.4.4 Ground-Water Chemistry

This section discusses only the initial two monitor wells MW-1 (MW-101) and MW-2 (MW-104) which are included in Appendix G. Quarterly sampling results of all wells have been submitted since September 1989.

Field parameters, such as pH, specific conductance, and temperature were measured at selected soil borings, piezometers and monitoring wells at the site. The field parameter results are presented in Table 4-4. Significant findings are summarized below:

- * Ground-water pH ranged from 6.4 to 8.3
- * Specific conductance, which is an indicator of ionized species, ranged from 600 to greater than 10,000 umhos/cm
- * Temperature ranged from 13° to 16° celsius

Ground-water samples were obtained from monitoring wells MW-1 (MW-101) and MW-2 (MW-104) and submitted for analyticial analysis. The analytical results are summarized in Table 4-5. The samples were analyzed for indicator parameters, pesticides and PCB's. Drinking water standards are also included in Table 4-5 for reference. Laboratory results are presented in Appendix F. Significant results are summarized below:

- * Total dissolved solids (TDS) exceeded the Secondary Drinking Water Standards (SDWS) of 500 mg/l in both wells. The standard was exceeded by more than 3 times at MW-1 and 9 times at MW-2.
- * Sulfate was detected in both wells in concentrations above the SDWS of 250 mg/l. The standard was exceeded by more than 9 times at MW-1 and 3 times and MW-2.
- * Nitrate concentrations exceed the Primary Drinking Water Standards (PDWS) of 10 mg/l in both wells. The nitrate concentrations exceeded the standard by more than 5 times at MW-1 and 3 times at MW-2.
- * Chloride concentrations exceed the SDWS of 250 mg/l in MW-1.
- * Pesticides and PCB's were not detected.

Revised June 1991

TABLE 4-4
FIELD PARAMETER RESULTS

LOCATION	Нд	SPECIFIC CONDUCTANCE (umhos/cm)	TOTAL DISSOLVED SOLIDS (mg/l)	TEMP. (degree C)
MW-1	6.6	2,000	1,300	
MW-2	6.4	4,400	2,800	13
P-2		600	400	
P-6		1,800	1,100	
P-10	7.0	>10,000	>6,000	16
P-11	6.7	3,300	2,100	14
P-13	7.2	3,100	2,300	14
P-15	7.1	1,600	1,000	
P-16	8.3	800	5,600	16
SB-3		3,000	1,900	
SB-11		3,800	2,400	
SB-28	7.4	5,000	3,200	15
SB-40	6.8	4,600	2,900	14

MW = MONITORING WELL

P = PIEZOMETER

SB = SOIL BORING

TABLE 4-5
GROUND-WATER ANALYTICAL RESULTS

	Standards	Secondary Drinking Water Standards	MW-1	MW-2
<u>Parameters</u>	(mg/l)	(mg/l)	<u>(mg/l)</u>	(mg/l)
Alkalinity			200.2	173.9
Specific Conductance			1730	3900
Total Dis- solved Solids		500	1515	4675
Calcium			83	468
Magnesium			35.7	194
Sodium			274	44.3
Potassium			43.1	22.5
Iron		0.3	<.05	<.05
Chloride		250	336	78
Nitrate	10		55.6	79
Sulfate		250	2463	716
Sodium Adsorption Rat	io		6.34	4.35
TOX			0.157	0.94
TOC			41	37
Pesticides			BDL	BDL
PCB's			BDL	BDL

BDL - Below Detection Limit

- * Total Organic Carbon (TOC) was detected at 41 mg/l in MW-1 and 37 mg/l in MW-2.
- * Alkalinity ranged from 174 to 200 mg/l, which is indicative of hard to very hard water.

Total dissolved solids were estimated on the basis of the specific conductance measurements. The specific conductance multiplied by a factor of 0.63 gives the dissolved-solid concentration.

The analytical results showed different water types to exist at each of the monitoring wells. MW-1 (MW-101) is classified as a sodium-calcium-magnesium type water, where MW-2 (MW-104) is classified as a calcium-magnesium-sodium type water.

The samples from both monitoring wells have several constituents that exceed the PDWS and SDWS. Therefore, the shallow perched water that occurs at the site is considered to be of poor quality and unsuited for domestic use.

Waters can be classified for suitability for irrigation by their conductivity and the sodium absorption ratio, a parameter that is calculated based upon the concentrations of sodium, magnesium and calcium. The ground water had a salinity classification of C3 (high-salinity water) at MW-1 and C4 (very-high-salinity water) at MW-2. The U.S. Department of Agriculture classification states that:

- 1.) "High-salinity water (C3) cannot be used on soils that have restricted drainage. With adequate drainage, special management for salinity control may be required and plants with good salt tolerance should be selected."
- "Very-high salinity water (C4) is not suitable for irrigation under ordinary conditions. If used, the soils must be permeable, drainage must be adequate, considerable excess irrigation water must be applied, and very low tolerant crops should be selected".

The waters had sodium classifications of S2 (medium-sodium water) for MW-1 and MW-2. Water that is classified as S2 is "hazardous for use on fine-textured soils that have cation-exchange capacity. This water may be used on coarse-textured or organic

soils with good permeability". Clearly, the shallow ground water found in the saturated low-permeability materials is unsuitable for irrigation purposes.

A final potential use is stock watering. McKee and Wolf (1963) report that stock can tolerate total dissolved solids concentrations up to 10,000 mg/l. They also state that the upper limit for livestock lies between 1,000 and 2,000 mg/l for sulfate and 1,000 mg/l for sodium. It appears that the water could be marginally suitable for livestock watering provided that it could be extracted in reasonable amounts.

4.4.5 Hydrogeologic Characterization

Aquifer testing was performed at the site to evaluate the hydraulic properties of the site-specific materials. This was accomplished by conducting packer tests, and rising head tests. In addition, selected relatively undisturbed soil cores were submitted for laboratory testing.

Packer tests were conducted at six soil borings (SB-29, SB-30, SB-31 SB-34, SB-35 and SB-37). The packer test field data are presented in Appendix H. The packer test results are summarized in Table 4-6 and are shown in the geologic cross-section plates.

Two packer tests were conducted in the interbedded, silty sand material at the site. The hydraulic conductivity for this material ranged from 1.1×10^{-5} cm/s at SB-30 to 1.3×10^{-4} cm/s at SB-29 with an average value of 7.1×10^{-5} cm/s. Four packer tests were conducted in the claystone bedrock at the site. These tests were conducted at SB-31, SB-34, SB-35 and SB-37. The hydraulic conductivity values ranged from 1.3×10^{-5} to 1.2×10^{-6} cm/s with an average of 4.9×10^{-6} cm/s.

Rising head tests were conducted in the two monitoring wells (MW-1 and MW-2) to estimate the saturated hydraulic conductivity of the interbedded, silty sand materials. The rising head test field data are presented in Appendix I. Table 4-7 contains the results of these aquifer tests. The hydraulic conductivity for the monitoring wells range from 1.5x10⁻⁴ cm/s at MW-1 to 1.8x10⁻⁴ cm/s at MW-2 with an average of 1.7x10⁻⁴ cm/s.

Constant rate permeability tests were conducted in the laboratory on selected soil and bedrock samples collected from borings SB-31, SB-32, SB-34, SB-35, SB-37, and SB-41. These tests were performed to supplement the packer and rising head test data in a effort to estimate the vertical hydraulic conductivity of the

TABLE 4-6
PACKER TEST RESULTS

			HYDRAULIC
CONDUCTIVITY SOIL BORING	TEST INTERVAL(feet)	LITHOLOGY	(cm/s)
SB-29	40-60	SILTY SAND (1)	1.3×10^{-4}
SB-30	30-55	SILTY SAND (1)	1.1x10 ⁻⁵
SB-31	59.5-75.5	CLAYSTONE (2)	1.7x10 ⁻⁶
SB-34	37-75	CLAYSTONE (2)	1.2x10 ⁻⁶
SB-35	40.5-55	CLAYSTONE (2)	1.3x10 ⁻⁵
SB-37	37-60	CLAYSTONE (2)	3.5x10 ⁻⁶

- (1) The silty sand lenses are interbedded within the claystone bedrock.
- (2) The claystone bedrock material may contain thin silty sand, sandstone or carbonaceous lenses.

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TABLE 4-7

RISING HEAD TEST RESULTS

SOIL BORING NUMBER	TEST <u>INTERVAL (ft)</u>	HYDRAULIC CONDUCTIVITY	(cm/s)
SB-43/MW-1	20-25	SILTY SAND	1.5x10 ⁻⁴
SB-44/MW-2	50-55	SILTY SAND	1.8x10 ⁻⁴

lithologic material at the site. The tests were performed in accordance with the American Society of Testing and Materials standards (ASTM D-2434). Table 4-8 contains the results of the constant rate permeability tests.

The samples obtained for laboratory permeability testing fall into two categories; composite and undisturbed. The composite samples were obtained from the auger cuttings, which generally ranged from 10 to 30 feet in depth. This material generally consisted of brown, silty clay with a trace of very fine sand. The permeabilities for this material range from 2.7×10^{-8} to 5.1×10^{-8} cm/s with an average of 3.8×10^{-8} cm/s. The relatively undisturbed samples were obtained from a specific lithologic interval using a California Barrel Sampler. The undisturbed samples submitted for laboratory testing were representative of the olive, greenish-gray claystone existing 40 to 50 feet below the ground surface at the site. This sample interval is significant, since it reflects the material underlying the disposal cells. The permeabilities for this material range from 3.0×10^{-8} to 3.1×10^{-7} cm/s with an average of 9.3×10^{-8} cm/s.

The packer and rising-head test results for the interbedded, silty sand material show similar permeability values, with an average horizontal permeability of 1.2x10⁻⁴ cm/s. This material occurs sporadically at the site as discontinuous lenses interbedded within the claystone bedrock. The horizontal permeabilities of the claystone, which were performed over test intervals ranging from 15 to 38 feet average 4.9x10⁻⁶ cm/s. The permeabilities of the claystone are approximately two orders of magnitude lower than that of the silty sand intervals. The vertical permeabilities for the surficial silty sandy clay material and for the claystone bedrock average 3.8x10⁻⁸ and 9.3x10⁻⁸ cm/s, respectively. These permeabilities are at least two orders of magnitude lower than the horizontal permeability values exhibited in the claystones and silty sands material.

The extremely low horizontal and vertical permeabilities of the natural material along with the extensive thickness of unsaturated materials, should effectively isolate the material in the disposal cells from the regional ground-water resources. Furthermore, the clay liners and sumps installed in each of the solidified waste disposal cells will provide additional environmental protection.

TABLE 4-8

CONSTANT RATE PERMEABILITY TESTS

SOIL BORING	SAMPLE INTERVAL <u>(feet)</u>	LITHOLOGY	SAMPLE TYPE	PERMEABILITY (cm/s)
SB-31	45	(1)	UNDISTURBED	5.1x10 ⁻⁸
SB-32	15-25	(2)	COMPOSITE	2.7x10 ⁻⁸
SB-34	40	(1)	UNDISTURBED	4.0x10 ⁻⁸
SB-35	50	(1)	UNDISTURBED	3.1x10 ⁻⁷
SB-37	10-30	(2)	COMPOSITE	5.1x10 ⁻⁸
SB-37	50	(1)	UNDISTURBED	3.0x10 ⁻⁸
SB-	10-25	(2)	COMPOSITE	3.5x10 ⁻⁸
SB-	40	(1)	UNDISTURBED	3.3x10 ⁻⁸

⁽¹⁾ Olive, greenish-gray-gray claystone

⁽²⁾ Brown, silty clay with trace very fine sand (weathered claystone)

4.4.6 Evidence of Perched Conditions

The hydrogeologic information from the field investigation provided conclusive evidence that the ground water encountered at the site exists under perched conditions. In addition, these perched saturated lenses occur sporadically and are hydraulically isolated in the vertical and horizontal directions. The evidence is as follows:

- * The ground-water analytical results show different water types to exist at each monitoring well. In addition, the specific conductance which is an indicator of ionized species, ranged from 600 to greater than 10,000 umhos/cm in the piezometers/boreholes tested.
- * SB-3, SB-11, SB-31, SB-33 and SB-34 were terminated in the claystone and were dry at the time of completion. During the course of the field investigation, these borings contained measurable amounts of water. This water is believed to be the result of downward leakage from discontinuous saturated lenses above the bottom of the borehole. The geologic cross-sections (Plates 4, 5, and 6) show that the water observed in these borings exist in thin discontinuous perched lenses across the site.
- * P-18, P-21, P-25, P-26, P-28, and MW-105 were dry at completion but contained measurable amounts of water during the course of the field investigation. This water is believed to be a result of thin, low permeability intervals within the zone of investigation that took time to produce water.
- * Perched lenses of water encountered at the site generally occurred on top of or within the impervious claystone. The elevations of the saturated materials varied greatly over relatively short distances across the site, suggesting that these lenses are hydraulically separated. A few examples of this situation include: 1) SB-38 and SB-39 were drilled to a depth elevation of 5278 and 5279, respectively, and are located within 100 feet of one another; however, the static water levels between the two sites varied over 7-feet. 2) SB-38 and SB-44 have similar water level elevations of 5291 and 5289 feet, respectively.

However, SB-28, which was completed between these borings exhibited a water elevation of 5267 feet, a difference of more than 20 feet. 3) P-22 and P-23 are approximately 750-feet apart. P-23 contains water at an elevation of 5257' but P-22 is dry at an elevation of 5242-feet, a difference of 15-feet. Furthermore, no lenses of saturated material were encountered above this level. This suggests that the saturated lenses encountered at these borings are hydraulically separated. This situation exists across the site wherever saturated lenses of water were encountered. Further evidence is presented in the geologic crosssection (Plates 4, 5, and 6) which show that the saturated lenses are hydraulically separated below the site. A few selected examples include soil borings SB-35, SB-45 and SB-34 on cross-section A-A', SB-42, SB-17, SB-36 and SB-11 on cross-sections B-B', and SB-4, SB-3, SB-23, SB-32 and SB-42 on cross-section D-D'.

- * Many of the borings were dry, while other borings completed within a few hundred feet encountered lenses of saturated material. This is evident upon review of the water level data and the geologic cross-sections. A few selected examples include soil borings SB-36, SB-11 and SB-40 on Cross-section B-B', SB-32 and SB-42 on cross-section D-D' and SB-22, SB-26 and SB-20 on cross-section E-E'.
- * The CME continuous sampling system was used to drill SB-36 to a depth of 75 feet. The elevation depth of this boring was 5229 feet, one of the deeper borings at the site. Continuous 5-foot soil core samples were collected and inspected to the total depth of the borehole. No lenses of saturated material existed (see cross-section B-B', Plate 4).
- * SB-2 was a dry hole completed to a depth of 60 feet. This boring was rebored with a larger diameter auger approximately two months later to a total depth of 65 feet. This boring was dry at the time of completion but contained measurable amounts of water a few days later. However, the materials sampled during drilling (between 60 and 65 feet in depth) were unsaturated. Moreover, the source of the water was identified in the field as being within an isolated perched zone at a depth of approximately 30 feet (see cross-section F-F', Plate 6).

* Saturated conditions exist in the moderately low permeability (1x10⁻⁶ to 1x10⁻⁴ cm/s) silty sand that occurs as discontinuous lenses either above or within thick sequences of low permeability (1x10⁻⁸ to 1x10⁻⁶ cm/s) claystone, with unsaturated conditions both above and below.

In summary, the results of the field program indicate that the claystone forms a nearly impermeable boundary beneath the entire site. Sixty-seven of the seventy-six borings drilled within the property boundary were terminated in the claystone bedrock. The only boring not completed in the claystone was a shallow boring (less than 25 feet in depth). Laboratory analysis and field inspections show that the claystone is unsaturated. The shallow ground water encountered during the drilling program is therefore hydraulically separated from the water table. Moreover, the shallow ground-water occurs in discontinuous saturated lenses under perched conditions across the majority of the site or is isolated within the ephemeral stream alluvium in the western portion of the site.

4.4.7 Perched Ground-Water Flow Paths and Velocities

The shallow ground water at the site exists as perched zones isolated by claystone aquitards above the Regional Denver Aquifer. The water will have a tendency to travel both laterally down-gradient within the perched zone and vertically toward the Denver Aquifer. Each component is discussed below.

4.4.7.1 Lateral Migration

The shallow perched water will preferentially migrate laterally down-gradient along the top of the low-permeability aquitard towards either a discharge point or towards the edge of the aquitard. The ground water will cease to move laterally when the higher permeability materials pinch out against the low permeability clays. All perched saturated materials pinch out within the boundaries of the filling area; therefore, lateral migration outside of the filling area from these materials will not occur.

The horizontal velocity of the ground water in the perched zones was estimated at 8.8 feet per year. This estimate is based upon:

* An average ground-water gradient of 0.017.

- * A mean composite saturated horizontal hydraulic conductivity of 1X10⁻⁴cm/sec.
- * An estimated specific yield of 0.2 as derived from information found in Lohman (1979).

4.4.7.2 Vertical Migration

The ground water will flow vertically through the thick sequence of unsaturated claystone towards the regional Denver Aquifer, where it will begin to flow north to northwest. All of the claystone materials logged were slightly moist to dry and dense to very dense.

Travel time calculations for water to flow vertically from the base of the disposal cell's clay liner to the Regional Denver Aquifer was estimated at 541 years. The calculations for the estimate are presented in Appendix J. Upon encountering regional saturation, the ground water would still have to travel a minimum of 2 miles until it encounterd the nearest well completed in the Denver Aquifer. All bedrock wells shown in Table 4-2 are completed in the Arapahoe Aquifer which underlies the Denver Aquifer and is separated from it by an extensive thickness of low-permeability claystone.

Based on the above information and calculations, the potential impacts associated with vertical ground-water flow through the unsaturated claystone bedrock material will be negligible.

5.0 FACILITY CONFIGURATION

The CSI facility is on a 240-acre parcel of which only approximately 135 acres is presently approved for use in the Certificate of Designation. The operational area of the CSI facility is located in the northcentral portion of the site on approximately 10 acres. This area is shown in Figure 2.3. Access to the site is from 88th Avenue. The waste disposal cells are situated in the southern and eastern portion of the site on less than 115 acres and the asbestos cells will be on 4 acres in the northeastern corner of the site.

It should be noted that the site includes a portion of a pipeline owned and operated by the Colorado Interstate Gas Company. CSI will remain a minimum of 50' from either pipe and will not use the alignment as roadway in order to protect the easement. In addition, CSI will not place more than 4 feet of cover over the easement. The cover will be placed only to match final grades across the site.

5.1 Non-Hazardous Waste Characterization

CSI is currently capable of handling a wide range of nonhazardous waste materials typically not accepted at the majority of solid waste landfills along the Front Range. Non-solid wastes are to be solidified by CSI prior to being placed into the cell for final disposal. Solid materials are placed directly into the disposal cells but are tested first using the EPA Paint Filter Method to ensure there are no free liquids present. Every waste stream accepted by CSI must first be scrutinized via CSI's waste acceptance procedure which entails a detailed analysis performed either by an independent lab or by CSI. Independent laboratory analysis may be verified through additional analysis (i.e. ignitability, corrosivity, and reactivity) performed by CSI. results of all analyses, in addition to other information (ie. knowledge of process, MSDS sheets, etc.), are reviewed by the CSI chemistry staff to ensure that only non-hazardous wastes are being accepted and disposed of in the cells. Upon review of all analyses, CSI may approve or deny acceptance, or request the generator to perform additional analysis. If the analysis indicates that the waste is compatible with CSI's waste disposal process, the waste is accepted and checked upon arrival at the site to ensure that the profiled material matches the material arriving at the site. Solidification of non-solid wastes will not occur at the site until the waste acceptance procedure, as described above and in Section 5.1.4.2 of this document, is in place.

The most common wastes disposed of at CSI are soils contaminated with petroleum products which consist of approximately 37 to 45 percent of the total waste stream. Water that has been contaminated by non-hazardous sources makes up approximately 30 to 36 percent of the total waste stream. The remaining streams consist of a variety of wastes. Table 5.1 details the most common wastes accepted at CSI, and the approximate percentage each contributes to the total amount of materials accepted. CSI is capable of disposing of other types of non-hazardous waste not included in this table, but currently these are the most common.

CSI will consider accepting any waste for disposal that is not a hazardous waste as defined in the Resource Conservation and Recovery Act (RCRA). These wastes will, however, only be accepted after successfully completing CSI's approval and screening process. CSI will not accept any Small Quantity Generator Hazardous Wastes, as defined in RCRA.

CSI may accept sulfide and cyanide bearing wastes as per a letter received from CDH on May 7, 1986. CDH has set an allowable limit of 200 mg/kg. This limit is 300 mg/kg lower than the EPA recommended guideline of 500 mg/kg and CSI will continue to accept only the lower limit unless granted approval by the appropriate regulatory agencies to increase it to the EPA guideline. For any volume of wastes accepted from CERCLA, SARA, RCRA Subtitle "D" clean-up sites, and for volumes of other waste streams anticipated to exceed 10,000 yards from one site to be disposed of in 60 days or less, the appropriate regulatory agencies shall be notified of the type of waste, screening, handling and acceptance procedures being utilized, the anticipated date the project might proceed and end, and the anticipated haul routes.

Types of waste not acceptable at CSI include:

- * Any and all hazardous waste as outlined in 40 CFR, Part 261.
- * Any waste with greater than 50 parts per million of Polychlorinated Biphenyls (PCB).
- * Any waste that cannot be legally treated by CSI.
- * Any waste CSI chooses not to accept.

CSI's laboratory and staff provide professional comprehensive chemical analysis, field sampling services, and data

TABLE 5.1
TYPICAL WASTES PROCESSED AT CONSERVATION SERVICES INC.

	PERCENTAG	
WASTE DESCRIPTION	OF VOLUM	E VOLUME
* Dry or solidified soil contaminated by petroleum products (gasoline, diesel,		
etc.)	37.0	45.0
* Liquids and solids from wastewater	15.4	19.0
treatment plants	15.4	19.0
* "Dirty" water (includes ground water, runoff water, neutralized acids/bases,		
sand trap sludges, water-based paints)	9.8	12.0
* "Oily" wastes (tank bottoms, oily slu pond sludges, asphalt clean-up, emulsifi	dges, ed	
asphalt)	5.7	7.0
* Water-oil emulsions (water-soluble		
cutting oils, coolants)	4.5	5.0
* FCC catalyst (primarily inert silica)	4.2	5.0
* Debris (concrete, plastic, scrap equi	pment,	
scrap steel, scrap empty containers*)	2.3	3.0
* Miscellaneous (mining ores, paint-dus	t	
residue, organic solid wastes)	3.2	4.0
Total	82.3	**100.0

^{*} If the containers were previously used to hold non-hazardous liquids, the liquids are removed and solidified prior to disposing of the containers. If the containers hold non-hazardous solid materials, the drums may be disposed of in the cells without removing the materials. CSI accepts containers that were used to hold hazardous wastes. These containers must be empty as defined by requirements set forth in 40 CFR, Part 261.7 prior to acceptance by CSI.

^{**} The remaining 17.7 percent of the material disposed at CSI is from one client. Empirical data was used to compile these figures and it is expected that the various percentages in the remaining table will be relative if the one particular client were not included and each percentage were raised to add a total of 100 percent. These relative figures are included in the column on the right.

interpretation services for the evaluation of a waste material. CSI is primarily involved with analyzing a waste's characteristics and constituents to determine whether it can be accepted for disposal. This is done by evaluating the process generating the waste, analyzing the waste for hazardous characteristics or constituents, and comparing both the analytical results and the process.

Laboratory analysis performed by CSI utilizing EPA approved methodologies will be done at their present laboratory site located at 2090 E. 104th Avenue in Denver. All analytical procedures are performed by qualified CSI personnel and the results reviewed and approved by CSI's chief chemist. The site for the CSI laboratory was selected for the purpose of serving the greatest number of clients in the Denver region. Samples being tested for landfilling at the CSI facility can be transported to the laboratory and tested more efficiently at this location.

5.1.1 Laboratory Quality Assurance/Quality Control Plan

5.1.1.1 Objective

It is CSI's policy that an active quality assurance/quality control (QA/QC) program is maintained to provide analytical data of known and supportable quality to ensure and maintain their high professional standards.

Within CSI's laboratory structure, QA/QC activities are practiced and enforced by analysts, the Laboratory Manager, and the Chief Chemist. QC activities are the checks performed on a routine basis to assure and document the quality of data generated. QA is the responsibility of the CSI management. They perform periodic audits and performance checks to ensure that the QC activities are being practiced.

5.1.1.2 Sample Handling

All of the samples received by CSI's laboratory must be accompanied by the appropriate paperwork before they will be analyzed. It is the responsibility of the waste generator to provide CSI with the proper paperwork to go with the sample. It is the responsibility of the sample custodian to ensure that the proper paperwork accompanies the sample prior to accepting custody of the sample. The following paperwork must accompany each sample:

* Chain of Custody Form (COC): The COC provides CSI with the necessary information pertaining to the methodology of sampling activity, analysis to be performed, and serves as a certification to ensure that the sample is representative of the waste generated. The COC must be signed by the generator in the presence of the sample collector who must also sign the form. A typical copy of the COC form is found in Appendix K.

An Industrial Waste Characterization Data form must be received prior to the waste being accepted for disposal. This form is discussed further in Section 5.1.4.1, Waste Characterization, and a copy of the form is included in Appendix K.

Samples are typically collected by CSI field or laboratory personnel instructed in the proper procedures and COC documentation requirements necessary when obtaining a representative sample. Each sample is submitted with the generator's name and the name of the waste material on the sample container. The laboratory personnel issues a unique 5-digit Laboratory Control Number (LCN) to the waste sample received. Once a LCN is assigned to a sample, it is placed on the sample container and entered into the Sample Log Book. The LCN is used to track the waste from the time it is submitted to CSI to the time it is either deemed unacceptable or the waste is received solidified (as necessary) and disposed of. The sample is kept in locked storage at a constant temperature until analysis is completed.

Samples collected by CSI personnel will be placed in containers compatible with the contents. Typical waste samples are collected in clean polyethylene containers with polyethylene caps, although many organic or petroleum-based samples are collected in glass containers. Samples requiring further analysis will be sent to independent labs following the proper chain-of-custody procedures and using the correct containers. Preservatives are to be used only when required by the analysis.

The progress of the sample analysis is documented during the analysis by the employee performing the test. The analysis is documented on a Laboratory Worksheet and tracked throughout the process via the LCN. A sample of the Laboratory Worksheet is included in Appendix K. The entire sample analysis noted on the Laboratory Worksheet is routinely completed in the same day. The analyst signs and dates the Laboratory Worksheet upon completion.

5.1.1.3 Analytical Methodology

The CSI laboratory utilizes EPA-approved test methods as outlined in the U.S. Government publication SW-846 "Test Methods for Evaluating Solid Waste, Physical/Chemical Methods" (SW-846). Copies of SW-846 are available to the CSI chemists in the laboratory.

In addition to SW-846, CSI also uses other accepted standard methodologies to screen for suspected contaminants. Prior to employing these screening methods, the method must be proven and documented in scientific publications, must be within the laboratory's capabilities as defined by the manufacturer, and meet with the approval of the Chief Chemist. CSI routinely uses the HACH colormetric methods for metals in water, and the toxic gas analyzer as qualitative screening methods. However, these methods may not accurately quantify the parameters. If the results of the screening methods indicate the presence of potential hazardous wastes, the sample is sent to an outside laboratory equipped to qualify and quantify the suspected wastes utilizing EPA-approved analytical methodologies. The outside laboratories utilized by CSI must first be audited by CSI's Chief Chemist to:

- * ensure they have an active QA/QC plan in place
- ensure the laboratories are strictly adhering to EPAapproved analytical methods
- * ensure they meet Good Laboratory Practice (GLP) standards as dictated by the nature of their business

All of the outside laboratories are subject to audits as deemed appropriate by the Chief Chemist.

5.1.1.4 Instrument Maintenance

Calibration procedures establish the relationship between a calibration standard of known concentration and the measurement of that standard concentration by an instrument or analytical procedure. CSI's calibrations are performed when an analytical method is initially set up, when an instrument is subject to major maintenance, and routinely throughout a batch of multiple analyses (usually 10%).

Preventive maintenance is performed on all of CSI's laboratory equipment to minimize any instrument failures and to ensure that the equipment is working correctly and to capacity. Normally, the preventive maintenance is performed every 90 days or as suggested by the manufacturer. In most cases, backup equipment is kept in stock so the laboratory can function without significant downtime because of equipment failure. If the equipment is expected to be out of service for any significant time, an outside lab will be contracted to complete the analyses required. As part of the recordkeeping and documentation, CSI will maintain a log book to record equipment calibration and maintenance procedures.

5.1.1.5 Corrective Action

One of the functions of an active QA/QC program is to identify problems in a timely manner and prescribe corrective actions.

Problems and corrective actions are discussed at the regularly scheduled quality control meetings held by the Chief Chemist. Some of the more common corrective actions taken at the CSI laboratory are:

- * updates in analytical procedures
- * equipment repair
- * response to out of control results
- * recurring sampling problems

5.1.1.6 Recordkeeping

CSI maintains the highest degree of recordkeeping possible for tracking samples and waste received. All the samples submitted are accompanied by a sample chain of custody (COC) form and a waste characterization data sheet (WCD). All of the information on these forms are entered into a computer with the related laboratory results. All the wastes received at CSI are accompanied by a four-part manifest. The generator receives two copies of the manifest, the transporter receives one copy, and CSI's laboratory receives one copy. The laboratory copies of the manifests, along with the analysis performed on the waste received, are entered into a computer. The computer software that contains the sample, laboratory and manifest information is backed up frequently. Backup discs with the computer information are stored at CSI and in an alternate, secure place away from CSI's facilities.

5.1.2 Equipment and Capabilities

The CSI laboratory is equipped with a variety of analytical instrumentation to perform a wide range of tests. Appendix K contains a list of the equipment currently utilized by CSI personnel involved in the waste characterization process. The CSI employees responsible for performing the analyses are trained in the proper use of the instrumentation by the Chief Chemist. CSI has the capabilities to perform all the necessary tests to ensure that only non-hazardous wastes are being accepted for disposal.

These tests include, but are not limited to the following:

- * Ignitability
- * Corrosivity
- * Reactivity
- * Radioactivity

CSI has the capabilities to perform various other tests to determine the physical properties of the wastes and the ratio of solidification agent to waste necessary to effectively harden the mixed substance.

5.1.3 Personnel and Qualifications

CSI only utilizes properly trained personnel to perform the analysis necessary to determine if a waste is acceptable at the CSI facility. The job descriptions of the key people currently responsible for the analyses are included in Appendix L.

5.1.4 Waste Analysis Plan

The chemical sampling and analyses used to screen the incoming samples and fingerprint the incoming loads of waste coupled with the paperwork submitted by the generator will effectively screen all the waste streams and ensure that only non-hazardous wastes are accepted for disposal. The foregoing Sections (5.1 through 5.1.3) are the waste analysis plan and is designed to meet this requirement.

5.1.4.1 Waste Characterization

Prior to analysis and acceptance of the waste, the generator must submit and sign all the necessary paperwork so CSI can The WCD is the generator's characterize the waste stream. certification that the information provided to CSI is complete The WCD is based on the generator's knowledge of and accurate. the waste-producing process and on chemical analyses performed on the waste. The generator will be required to document the chemical composition of each waste stream on the WCD, included in Each major constituent will be identified along with Appendix K. its concentration. If the concentration will vary from shipment to shipment, the potential range will be indicated on the WCD. All minor constituents, which fall under the listing of controlled contaminants and/or are toxic, must be identified and their concentration indicated. In general, trade names will not be acceptable. All the constituents will have to be described by International Union of Pure and Applied Chemistry (IUPAC) nomenclature or listed in an approved chemical handbook, such as the CRC Handbook of Chemistry and Physics. If the waste analysis constitutes revealing a trade secret, then the generator will be required to submit safety data sheets and a description of the chemical characteristics of each material. A questionnaire is included as part of the WCD as a more thorough check for hazardous wastes determinations. The following questions are included:

- * Is the waste stream from a non-specific source listed in 40 CFR Part 261.31 (F-codes)?
- * Is the waste stream from a specific source listed in 40 CFR part 261.32 (K-codes)?
- * Is the waste from a discarded, commercial chemical product, off-specification species, container residue and/or spill residue listed in 40 CFR, Part 261.33 (P-codes and U-codes)?
- * Does the waste have any hazardous characteristics listed in 40 CFR, Part 261.2, Subpart C (D-codes)?
- * Is the waste specifically excluded from hazardous waste regulations in 40 CFR, Part 261.4? If so, in what portion of 40 CFR, Part 261.4 is the exclusion mentioned?

The answer to any of the above questions must indicate that the waste disposed of at CSI is not a hazardous waste. Wastes that are excluded from hazardous waste regulations will be accepted only when the generator provides information showing the waste does not have hazardous characteristics or when it is proven that CSI's solidification process will prevent the excluded waste from having hazardous characteristics after the process is completed.

The waste stream approval process and bench scale testing provide data that allows CSI to determine compatibilities of various waste streams with another waste. The majority of the waste streams accepted at CSI have been found to be compatible.

CSI will help provide their clients with information to assist them in effectively completing the forms but does not make waste determination for them. The generator uses the information provided by CSI as well as any additional information available to make these waste determinations and complete the WCD.

5.1.4.2 Waste Acceptance Procedure

CSI has incorporated stringent guidelines to ensure that all wastes accepted at the facility are non-hazardous. Each waste stream that is submitted to CSI for consideration must pass through the waste acceptance procedure as follows:

1. WCD Review

Once the generator has submitted all of the appropriate paperwork and a representative sample, the WCD is reviewed by the Laboratory Manager or a CSI chemist. The CSI chemist will scrutinize the WCD to ensure that all of the pertinent information required by CSI is present, and that the generator has signed the WCD. If all of the information has been provided, the CSI chemist will review the chemical composition and characteristics of the waste stream to ensure that the material is a non-hazardous waste and that it is within CSI's capabilities to dispose of the waste. Based on the results of the review, a detailed analysis plan will be developed. The analysis plan will normally be completed by CSI, but may require the services of an outside lab on occasion.

If, based on the information provided by the generator, the waste is determined to be unacceptable for disposal at the CSI facility, a notice of rejection will either be verbal or will be delivered to the generator. CSI will document waste streams that have been rejected.

2. Analyses

Once the waste stream has been found acceptable in the review process, an analytical program will be instituted to ensure that the information provided on the WCD is accurate. If the profile sheet indicates the presence of a substance which may not be acceptable at the CSI facility and CSI is not equipped to quantify the substance, an outside lab will be used to determine the exact concentration of the material.

CSI will review the following analyses on all samples submitted for waste acceptance and disposal:

- * Corrosivity pH A pH meter is used to measure the pH of liquids and one percent solutions of solids to determine whether the sample is basic or acidic.
- * Ignitability flash point The Pensky-Martens closed cup flash point tester and stirrer is used to determine the flash point of liquids. Solids are also tested with the Pensky-Martens closed cup flash tester to evaluate the ability of the solid to sustain combustion, but solids do not have "flash points" per se.
- * Reactivity sulfides and cyanides The Matheson/Kitagawa toxic gas analyzer is used to measure the concentrations of various gases, such as ammonia, hydrogen sulfide, and cyanide. Further tests are performed using ASTM standard methods to more completely quantify the concentration of these materials in the waste, or the sample is sent to an outside laboratory for testing.
- * Total Metals Ba, As, Cr, Pb, Cd, Hg, Se, Ag The HACH spectrophotometer and HACH digesdahl are utilized for the detection of trace metals. Only total metals are run. When a problem metal is detected in sufficient quantity, the sample is sent to an outside laboratory for the Toxicity Characteristic Leaching Procedure (TCLP) metals test.
- * Bielstiens halogenation The "Bielstiens" analysis is a spot test for chlorides. If the chlorides are from chlorine, the chlorine is measured on the HACH spectrophotometer. If chlorides could be from the presence of halogenated organics, the sample is sent to an outside laboratory for testing.

The tests for total metals, halogenation and organics are qualitative tests. If the analysis indicates a potential for the waste to be hazardous, as defined in Section 5.1 above, the sample will be sent to an outside laboratory for additional testing. On organic samples where halogenated organic constituents are suspected, the samples will be tested for TOX and/or Volatile Organic Hydrocarbons (Method 8260 or its successor) to determine that it is not a hazardous waste.

Each sample will have a detailed description of its physical characteristics completed including:

- * Solubility
- * Percent moisture
- * Suspended solids
- * Odor
- * Color
- * Viscosity
- * Density

If any of the results from these tests demonstrate a significant deviation from the information provided on the WCD, the waste will require further analysis as recommended by CSI.

Prior to unloading the material, the following procedures will be followed:

- * The transport manifest is checked. CSI does not accept any shipments of waste without a manifest properly completed and signed by the generator of the waste or an authorized representative. The incoming material must meet the descriptions on the manifest. The manifests are retained in CSI's records with the analytical results from the waste screening process and the waste profile data sheets.
- * CSI checks to ensure the waste has a Waste Characterization Data Sheet and is assigned a Waste Code Number (WCN).
- * Waste streams are sampled as outlined in Section 5.1.4.3 by, or supervised by, on-site laboratory personnel.
- * The waste is screened for compliance with appropriate CSI guidelines and to ensure the waste arriving at the site for disposal is the same as that which was analyzed at the laboratory and listed on the manifest and WCN. (Section 5.1.4.2 and 5.1.4.3)

* The truck is directed to the proper unloading areas.

Unloading is directed according to the type of shipment container and the treatment process recommended by the CSI chemist.

Drummed materials will be unloaded at the mixing basin or drum storage area and marked with a WCN and a shipping date. The drums will also be labeled as non-hazardous waste. The drums are then segregated in the storage area according to the client and source of the material. Drums without adequate lids will not be accepted prior to shipment to CSI. All drums on-site are required to be covered or have the bungs closed. If bungs are missing, the drum will be covered with a plastic lid. All barrels or drums containing liquid waste that is to be solidified will be processed within 15 working days of receiving the waste. Typically, drums are recycled to a steel scrap yard after they have been emptied. Drums that are placed in the disposal cell will be crushed prior to disposal.

Materials transported in tanker trucks or vacuum trucks will be unloaded to the tank farm for future disposal or they are emptied directly into the mixing basin for solidification and disposal.

Mixing of the materials will be completed in concrete mixing basins designed expressly for this purpose (Section 5.2.2.2). The solidifying agent, usually cement kiln dust, is stored onsite in a metal building open on one side. The CSI tests run on samples of incoming materials to determine the ratio of solidification agent to liquid required to properly solidify the mixture when placed in the disposal cell. This ratio will be used when bulk-mixing of the material is completed in the basin. Additionally, CSI will perform a Paint Filter Test on each mixed load prior to removal from the basin to determine whether free liquids are present. Each load is required to pass this test.

The solidification agent will be added to the liquid in the mixing basin and mixed by a track-mounted backhoe until only a cement-like mixture remains. The backhoe will place the mixed material in a dump truck that will transport the material to the cell. The placement and compaction of the material in the cells is discussed in Sections 5.2.3.5 and 5.2.3.6.

Each sample submitted to the CSI laboratory is tested to determine its optimum mixing ratio with the solidification agent. This test is simply called a solidification test and is documented in the Laboratory Worksheet. The solidification test is also used to estimate the amount of reagent needed to solidify

the waste and any volume increase expected once the waste is solidified. This ratio is then used in the bulk mixing operations to ensure a liquid-to-solidification agent mixture that will result in an optimum pozzolanic process in the disposal cell.

If the waste stream submitted passes the WCD reviewing process and the test results confirm the information provided by the generator, the waste stream is accepted and assigned a WCN. The WCN differs from the LCN as the LCN is a 5-digit number assigned to each incoming sample, while the WCN is a number assigned to accepted waste streams. (Example: LCN - 01501, WCN - CO-02-072187-01501.)

5.1.4.3 Waste Screening

Each approved waste stream arriving at the CSI facility is screened by a properly trained chemist or lab technician to ensure that the material profiled and accepted matches the material arriving at the facility for disposal.

When a waste arrives, the transporter must submit the appropriate paperwork (manifest, WCN) which is then reviewed by CSI to ensure that all the necessary information is present and that the generator has signed the manifest.

Sampling of the waste stream is performed by, or supervised by, CSI personnel who have been trained in the appropriate sampling protocols. One representative sample will be taken from bulk solids, bulk liquids, and drummed wastes.

Each sample will be submitted to the laboratory for a screening test, prior to unloading. The tests that will be run on each sample are as follows:

- * Corrosivity
- * Ignitability
- * Reactivity
- * Radioactivity

In addition to the above tests, the physical characteristics of the sample will be compared to those of the sample submitted for approval.

On projects where several shipments from the same source are being delivered in the same day, at least 10 percent of the loads will be screened.

If any of these tests deviate significantly from the original results, the generator will be notified and the load will be rejected. If the load is rejected, the waste stream approval will be cancelled until the deviation can be explained by the generator or additional analytical work is completed.

CSI is performing a High Organic Content Waste (HOCW) study as a condition of the CD (see Section 7.3) to determine the effectiveness of the solidification process on HOCW. The HOCW study entails chemical analysis of the HOCW. Results of this study will be evaluated to verify that the screening tests are adequately characterizing the waste.

If, through analysis of a high organic content waste stream performed by CSI, it is determined by the appropriate regulatory agencies that the screening tests are inadequate for detecting waste that is hazardous due to its organic content, the waste analysis plan will be ammended and approved by the appropriate regulatory agencies to ensure that no hazardous waste is accepted.

CSI clients often dispose of the same waste stream at the facility continuously. For wastes that are received on a continual basis, the generator will be required to sign a notice verifying that the waste stream has not changed. The notice will include the following questions:

- * Has the waste stream changed? If so, explain the changes.
- * Is there any new analytical information regarding the waste stream that has been completed during the past year?

5.2 Non-Hazardous Liquids Solidification and Disposal

Wastes that meet the requirements for disposal at the CSI facility and have proven amenable to CSI's treatment processes are accepted for treatment and solidification. The CSI chemist will assign the waste a WCN and recommend procedures for handling, treatment, and disposal of that particular waste stream. No waste will be accepted without a WCN.

5.2.1 Process Description and System Capacity

A WCN will be assigned only after sampling and analysis of each waste by CSI chemists. More detailed information regarding the waste analysis plan is contained in Section 5.1 of this report. Following assignation of a WCN and acceptance, transport of the waste to the facility will be scheduled. Either CSI or another qualified transporter will deliver the waste to CSI for solidification and disposal.

Solidification has been used at various other facilities to stabilize liquid materials. The mixing of cement kiln dust with the liquid waste creates a pozzolanic process that is tested in the laboratory prior to mixing the bulk material. The pozzolanic process consumes free liquids therefore prohibiting the creation of leachates when the material is placed in the cells. The bench test, previously completed during the waste characterization phase, determines an adequate liquid to kiln dust ratio to use when mixing. In addition, CSI will perform a Paint Filter Test (EPA Method 9095) upon completion of mixing to determine whether free liquids are present in the material prior to removing it from the basin. If the material fails the test, CSI will add additional solidification agent until the material passes. The waste can then be placed in the disposal cells.

It should be noted that CSI intends to use cement kiln dust as its solidification agent. Table 5.2 is an actual analysis of kiln dust used at the facility and is typical of other kiln dusts that will be used. With future changes in technology and enhancements of techniques used at CSI, it is possible that new agents may be used if they prove feasible. CSI will notify the appropriate agencies on the proposed change and any variations from the use of kiln dust will require approval by the appropriate agencies prior to its use.

The facility will be able to process a maximum of approximately 23,000 gallons of liquid in each half of the mixing basin at one time for a total of 46,000 gallons. The process requires

Revised June 1991

TABLE 5.2

CONSERVATION SERVICES, INC.

CONSTRUCTION TECHNOLOGY LABORATORIES, INC. SKOKIE, ILLINOIS

Report of Chemical Analysis

Sample No.: 83

Sample Type: Kiln Dust

pH of TCLP Extract:12-13

EPA max. TCLP		<pre>TCLP Results (mo/L)</pre>			Total Results (mg/kg)		
ma/L	Leachate	MDL	MOL	Solid		MOL	
(none)	1.17	0.10	0.16	151	15	25	
5.0	<0.16	0.10	0.16	10.9	5	8.4	
100.0	1.41	0.1	0.2	323	10	20	
(none)	0.013	0.002	0.003	3.58	0.7	1	
1.0	0.051	0.0091	0.015	44.0	0.6	0.9	
5.0	0.082	0.008	0.01	35.6	1.3	2.2	
5.0	0.524	0.091	0.15	146	3	4	
0.2	0.000756	0.0003	0.0005	<0.005	0.005	0.01	
5.0	0.062	0.005	0,009	9.55	0.40	0.67	
(none)	0.70	0.07	0.1	85.1	4.7	7.8	
	concentration mg/L (none) 5.0 100.0 (none) 1.0 5.0 0.2 5.0	concentration mg/L leachate (none) 1.17 5.0 <0.16 100.0 1.41 (none) 0.013 1.0 0.051 5.0 0.082 5.0 0.624 0.2 0.000756 5.0 0.062	concentration mo/L leachate MDL (none) 1.17 0.10 5.0 <0.16 0.10 100.0 1.41 0.1 (none) 0.013 0.002 1.0 0.051 0.0091 5.0 0.082 0.008 5.0 0.624 0.091 0.2 0.000756 0.0003 5.0 0.062 0.005	concentration mg/L Leachate MDL MOL (none) 1.17 0.10 0.16 5.0 <0.16	concentration mo/L leachate MDL MQL Solid Sample (none) 1.17 0.10 0.16 151 5.0 <0.16	concentration mo/L Leachate MDL MOL Sample MDL S	

otes:

The number of significant figures in each result varies depending on the absolute absorbance read from the spectrometer.

[.] Tests performed in accordance with EPA TCLP protocols.

[.] EPA procedure 3050 used for total metals digestion, except Sb (3005) and Hg (7471).

[.] HDL (minimum detection limit) calculated from this analytical batch according to procedure, MDL = three sigma divided by slope of calibration curve.

MyL (minimum quantification limit) calculated from this analytical batch according to EPA procedure, MQL = five sigma divided by slope of calibration curve.

approximately 110 gallons of liquid waste solidified with kiln dust to yield one cubic yard of material for disposal in the cells; therefore, approximately 209 cubic yards of solidified material can be mixed in each half of the mixing basin. During current peak operations, 3 loads (per half of the mixing basin) can be mixed per day, totaling 627 cy of solidified material per day. Initially, CSI will construct only the first mixing basin. The second basin will be constructed as the market warrants, allowing them to process 1254 cy of solidified material per day during peak operations.

5.2.2 Solidification Area

The CSI solidification area is located in the northcentral portion of the site and includes the mixing basins and auxiliary structures. The auxiliary structures and layout are shown in Figure 2.3 and include the laboratory, kiln dust storage, tank farm, water process area, drum storage area, equipment storage, and truck wash bay located in the two-story operations building.

5.2.2.1 Location and Surface Water Control

The solidification area is arranged to minimize the effects of the wind and surface-water runon. Surface-water runon to this area will be controlled through the use of berms that will route water away from the area. Precipitation falling directly on the area will be routed off-site or will be collected and disposed of or used for dust control. Water in the mixing basin that is a direct result of precipitation will be minimal because of the raised walls of the basin. Precipitation on the mixing basin and primary drum storage apron will drain through seep holes located on the sidewalls of the mixing basin directly into the basin for solidification. The water that falls in the basins will be solidified and placed in the disposal cell with the other wastes. The drum storage pad and tank farm area will be bermed to route surface water runoff around these areas. Any direct precipitation onto the drum storage pad or into the tank farm diked area will be allowed to evaporate or will be collected and solidified. More detailed diversion information for the entire site is included in Section 5.4.

5.2.2.2 Mixing Basin Design

The mixing basins at the CSI facility will be used to mix the non-hazardous liquids with the solidification agent. Also, the mixing basin may incorporate the drum storage area as shown on Figure 5.1a. Initially, one mixing basin will be constructed

although a second basin may be constructed at a later date to allow increased mixing capacity, if the market warrants. location of the mixing basins are shown in the facility configuration in Figure 2.3. Each basin is capable of mixing two 23,000 gallon batches of material. The original basin specifications are included in Plates 9 and 10. specifications may be utilized for the second basin (Basin B), or if a different design is utilized, the specifications will be designated as Plates 12 and 13. The exterior dimensions of the basin will be approximately 100 feet by 20 feet, with a depth of 8.5 feet from the top of the basin walls to the basin bottom. The basin walls will be 6 inches above grade and 8 feet below The excavated substrate beneath the basin bottom will be sloped from the longitudinal center to each end to ensure positive drainage to the leachate detection underdrain while the actual concrete basin bottom will be constructed at a level The basin will be divided in the center by a concrete wall to allow CSI to more efficiently mix wastes. A 20-foot surface slab apron will surround the basin. All slabs and walls of the basins will be constructed of steel reinforced, 9-inch thick concrete. The concrete apron surrounding the basin will slope to the basins as described in Section 5.2.2.1. Curbs will be constructed around the mixing basin aprons to ensure drainage into the basin. All runoff collected in the basin will be solidified with other materials.

The entire mixing basin excavation will have the substrate prepared to ensure the synthetic liner will not be punctured. The base layer of material will consist of compacted solidification agent, if necessary. Otherwise, the excavated ground will act as the substrate. Above it, a synthetic High Density Polyethylene (HDPE) liner with a minimum thickness of 30 mils will be keyed into the ground surface beneath the concrete apron as shown in Plates 9 and 10. The synthetic liner will then be overlain with a minimum 6-inch, clean-sand drainage blanket. The entire underdrain system will be sloped to the underdrain wells located at each end of the basin, at minimum 1 percent grades to allow drainage and detection of potential leaks in the concrete basin.

Underdrain leachate detection wells will be located at the eastern and western ends of the basin. The underdrain leachate detection wells will consist of 6-inch, schedule 40, PVC pipe with the bottom 2.5 feet slotted, and will be protected with a steel cover and steel cap to be selected by the contractor and approved by a Registered Professional Engineer in the State of Colorado. The underdrain leachate detection wells are used as a leak detection and collection system for the basin.

5.2.2.3 Mixing Basin Construction

One of the two mixing basins (Basin A) has been constructed as per the specifications and drawing details in Plates 9 and 10. The construction of the basin has been documented in the Mixing Basin Construction Report. A second mixing basin (Basin B) will be constructed of an equal or better design than the existing mixing basin and may incorporate sloping walls and steel lining. If a different design is utilized, it will be designated as Plates 12 and 13. The base dimensions for the second basin will be approximately the same as the existing basin. The underdrain system will be separate from the underdrain system in the existing basin. Excavation for the existing mixing basin was to an approximate depth of 10 feet in order to place the underdrain system. The base dimension of the excavation is approximately 102 feet by 22 feet with a minimum 1 percent slope to the east and west from the center. At the edges of the excavation the slopes are 1 to 1, up to 6 inches below the ground surface to key the HDPE liner and minimize the potential for leaks. Areas at each end of the excavation are cut out to install the surface The drainage sumps are also underlain by the drainage sumps. HDPE liner.

Following excavation for the basin, the underdrain system was installed. The first layer of the underdrain system will be compacted kiln dust, if necessary. Compaction was completed by passing a smooth-drum roller a minimum of three times over the kiln dust. A HDPE liner, with a thickness of 30 mils, was then placed atop the prepared substrate sloping to the east and west at a minimum 1 percent grade and on the 1 to 1 slope to within 6 inches of the surface.

The base of the excavation was backfilled using clean sand as a drain material. The sand will form a base for the concrete basin bottom and was placed at a level grade. The sand material should have a maximum size of No. 4 sieve and a minimum size of No. 6 sieve and was placed at a minimum thickness of 6 inches to provide an adequate base for the concrete slab. No heavy equipment was allowed to operate on the liner or the sand above the liner, except for compaction purposes.

The mixing basin base slab was formed atop the sand layer using the specifications stated in Plates 9 and 10. All formwork conforms to these specifications and no stakes were driven into the sand to hold the forms in order to prevent potential penetration of the liner. The base slab was constructed to include a 2-inch deep by 4-inch wide key around the outside edge.

A PVC waterstop was placed continuously around the slab and the vertical walls were centered on a key. Also formed into the base slab was reinforcing steel to strengthen the vertical basin walls as shown in Plates 9 and 10. Immediately prior to pouring the concrete for the base, the sand layer was saturated with water to allow the concrete to cure at a slower rate.

After pouring the concrete, a white pigmented liquid curing compound was applied to the base slab within 30 minutes of finishing. A second coat was applied within 30 minutes of the first coat at a 90 degree angle to the first coat. The base slab was covered with 6 mil polyethylene film for a minimum of 48 hours after pouring to protect the "green" concrete from potential adverse weather conditions and allow it to cure at a slower rate. Construction joints in the slab were placed as soon as possible without causing the concrete to ravel. The joints were cleaned and filled with joint compound.

The basin walls were formed and cast in one pour to reduce the potential for cracking. Reinforcing bar (as per the specifications on Plates 9 and 10) was used to strengthen the walls. The walls were centered on the key constructed around the perimeter of the base slab. The concrete was placed in lifts not to exceed 15 inches and each lift was vibrated mechanically. Upon completion of pouring, the walls were covered with burlap and kept wet for a length of time adequate to ensure proper curing of the concrete.

Control tests for the base slab and the vertical walls were made by an independent concrete testing firm. The control tests for the floor slab consisted of 6 standard 6-inch test cylinders cast and cured in accordance with all standard testing procedures. The control tests for the walls include the same tests for the slab with the addition of 6 standard field cylinders that are field cured.

The area around the outside of the vertical walls and inside the liners was filled with a clean sand compacted in intervals not to exceed 8 inches. The sand was compacted to 95 percent of the maximum dry density as per ASTM D698 (Standard Proctor).

The aprons skirting the mixing basins were 8 inches in thickness and reinforced with steel. The apron slabs were be built atop a base of clean sand with a minimum thickness of 6 inches that is saturated immediately prior to placement of the concrete. Also prior to placement of the concrete, the surface drainage sump and the underdrain leachate detection wells were installed. Details of the surface drainage sump were provided by the contractor and

approved by the engineer. The underdrain wells consist of 6-inch PVC, schedule 40, pipe with 2.5 feet of slotted pipe at the bottom, extending to the synthetic liner. Both wells reach ground surface. The surface drainage sumps will be protected with heavy duty manhole gratings and the underdrain sump protected using a steel grate at the surface of the concrete. The concrete surface around the underdrain wells will be gradually sloped up to minimize the addition of surface water to the underdrain well. All slabs are formed to slope to weep holes in the walls of the basin to allow liquid to drain directly into the basin.

The finishing and curing of the aprons conform to the specifications set on Plate 9.

5.2.2.4 Auxiliary Structures

The auxiliary structures and designated areas in the facility operations area include the laboratory, administrative office, disposal office and maintenance shop, mixing basins, the sampling station, solidification agent storage building, truck wash bay, drum storage area, and tank farm. Additionally, there is an area set aside for equipment parking and employee parking. These structures and areas are shown in Figure 2.3.

Initially, the site was open only for cell disposal of solidified special wastes and asbestos wastes. The construction of the facility buildings and mixing basins and auxiliary structures followed at a later date. Permanent water and sewer facilities were placed at the site during construction of the buildings and operations area. Sewer facilities were constructed as an individual system with a septic tank and a leach field. of this water and sanitary system was approved by the proper state and county agencies and all necessary permits were obtained prior to their construction. All initial temporary buildings and sanitary facilities were taken out of commission as permanent structures were completed and will be removed from the site within 2 years of facility start-up. Excess drum storage is located east of the mixing basin area (Figure 5.1) and may be enclosed inside a building in the future. A diesel fuel tank inside a concrete vault is located south of the excess drum storage area.

The administrative office, disposal office and maintenance shop and wash bay are located in two buildings. The administrative offices houses CSI employees involved in overseeing the day-to-day affairs of the company including the invoicing, marketing,

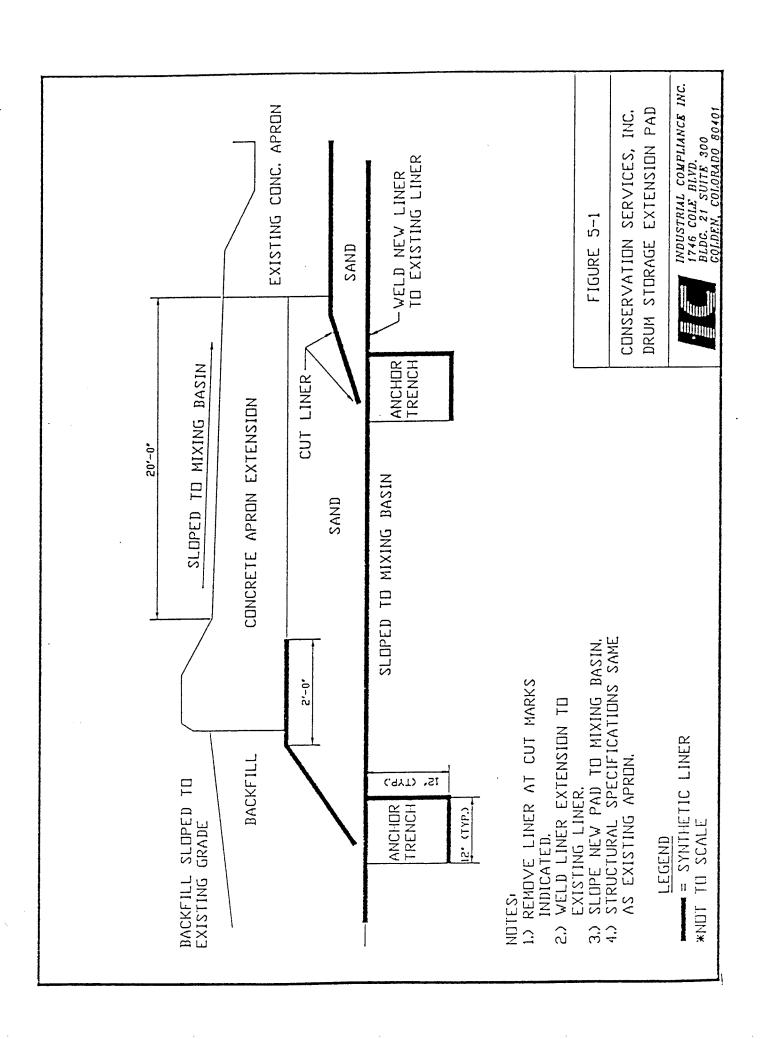
and overall project management work. The disposal office is used to track incoming shipments of waste, schedule solidification of the waste and keep records of the various customers, types of materials received, and amounts of materials solidified and disposed of. Screening of the incoming waste shipments is also performed in the disposal office. The maintenance shop and wash bay are part of the administrative office and is located north of the kiln dust storage building. The maintenance shop will house equipment and site-maintenance tools.

The sampling station is to be located inside the fenced facility and south of the offices to allow a convenient stopping point. The station is used to sample incoming waste streams previously analyzed and accepted by the CSI facility. Currently, the samples are collected on the mixing basin apron. The samples collected are screened to match the originally submitted sample. The sampling procedures and screening procedures are discussed in detail in Section 5.1.4.3.

The solidification agent storage area is an open-sided building located approximately 100 feet west of the mixing basins. The building will protect the solidification agent (kiln dust or future approved agent) from precipitation and wind. The building will be approximately 101 feet by 41 feet and is 24 feet in height. It is open at the front to allow unimpeded access with heavy equipment, and is sided on the remaining three sides. The storage building is sized large enough to hold approximately 300 tons of the agent. The solidification agent is restocked in the building on an as-needed basis.

The truck wash will be located in the northwest portion of the operational area in the administrative building. It consists of a concrete pad with a floor drain and underground holding tank to collect the wash water. All wash water collected will be placed in the mixing basin and solidified for disposal in the cells.

The primary concrete drum storage area is not yet constructed. It will be adjacent to the mixing basin on the west side. It will be designed to contain any releases or precipitation and to drain directly onto the mixing basin pad and into the mixing basin as shown on Figure 5.1. Drums will be stored on a bermed pad approximately 140 feet by 20 feet in area. The pad will overlay a synthetic liner similar to that used for the mixing basin and be directly and permanantly tied to the mixing basin pad underdrain system. The drum storage pad is expected to be able to hold approximately 500 drums. Drums stored in the



eastern 25 feet of the storage pad will be required to be set on pallets in the event there is standing water. Precipitation or released liquids which collect in the mixing basin will be solidified within 72 hours of its accumulation if it has not evaporated. Ramps will be built over the berms to allow access to the pad. As noted in Section 5.2.1, drums are required to have lids during transport to CSI and storage at CSI. can be plastic or steel but will prevent precipitation from entering the drum. Drums that are emptied will be either recycled or crushed and placed in a cell. Crushed drums will not be placed within two feet of the bottom, side slopes or final cap In the event a drum (or drums) is spilled, the of the cell. material and soil will be immediately removed and the soil will Removal of the soil will be supervised by the be replaced. Facility Manager or Chief Chemist. The incident will be fully A copy of the documented and reported to CDH/TCHD within 7 days. incident report will be kept at the site. Care will be taken to protect the HDPE liner but if it should be damaged it will be repaired prior to placement of any other drums within 10 feet of the damaged area. If material is spilled on the berm, that portion of the berm will be removed and rebuilt. where the adjacent properties are developed for uses other than agricultural, a three-sided building will be erected around the drum storage pad as a visual screen.

A diesel fuel tank enclosed in a concrete vault allowing easy inspection is located southeast of the mixing basins.

The tank farm will be located south of and in close proximity to the mixing basin. Tanks are of polyethylene construction and sit inside individual polyethylene surround containment systems. The tanks have a capacity of 6500 gallons each with a containment system capacity of 6600 gallons each. The tanks will sit on a level compacted sand bed. The containment system will be translucent and designed for ease of leak detection and corrective action measures. A Spill Prevention Control and Countermeasures Plan (SPCC) will be prepared and reviewed by the proper agencies.

An earthen berm will be constructed that will extend nearly the length of the site along 88th Avenue and also along the eastern boundary of the facility operations area (Plate 1). The berm adjacent to 88th Avenue will be set back 65 feet from the northern property line to allow for the future expansion of 88th Avenue. The berm will be an aesthetic feature as well as a runoff diversion berm and will be used to screen the operations at the site. It will be approximately 8 feet in height and will

have 1 to 1 side slopes. Following construction of the berm, it will be revegetated using trees and natural grasses and plants as outlined in Section 9.0. Upon closure of the site, the berm may be removed to blend with the topography of the site.

5.2.3 Solidified Waste Disposal Area

The solidified, non-hazardous liquid waste is removed from the mixing basins and transported to the waste disposal area where it is placed for final disposition. The solidified waste disposal area consists of individual disposal cells that cover approximately 115 acres of the site. Actual cell configuration may change in the future following approval by the appropriate regulatory agencies. In the southeast corner of the site cells are situated to remain a minimum of 50 feet from the Colorado Interstate Gas Company pipelines. As a condition of the permit, the solidified waste disposal cells are designed so that a minimum of 30-feet separation is maintained between the lowest point in the cell (top of clay liner) and ground-water beneath The hydrogeological characterization of the site has been used to design the cell depths.

5.2.3.1 Cell Quantities and Site Life

The CSI disposal area consists of individual disposal cells located to the south and west of the operational area. The cells vary in depth and dimension. The depths of the cells range from 8 feet to 40 feet following construction of the liner and leachate collection system. The side walls of the cells are constructed at a 3 to 1 slope. Plate 1/2 show the overall layout of the facility and the cell locations.

Cells range in volume from a low of 40,150 cubic yards to a high of 679,700 cubic yards. The total volume of disposal airspace is estimated to be 2,977,850 cubic yards. Based on the volume received since opening the facility, the total site life is approximately 20 to 25 years. Table 5.3 includes cell volumes. If further individual cells are requested by clients, cells may be divided and redesigned in conformance with the standards in this plan as confirmed by approval of regulatory agencies. An additional cell may also be added in the area shown on Plate 1 if a cell can be designed in conformance with standards in this plan as confirmed by approval of regulatory agencies.

Plate 1 shows the overall layout of the facility and the cell locations.

TABLE 5.3

CONSERVATION SERVICES INC. LANDFILL

APPROXIMATE CELL VOLUMES

SOLIDIFICATION DISPOSAL CELLS VOLUMES

CELL NUMBER	CELL VOLUME (cubic yards)			
1 2	131,200 134,900			
2 3	83,000			
4/5/8	302,200			
6/7/9	273,700			
10/11/12	269,200			
13*	40,150			
14*	164,250			
15*	165,700			
16*	77,100			
17*	69,500			
18/21/22/23	679,700			
19/20	321,450			
TOTAL	2,712,050			

ASBESTOS CELL VOLUME	169,100			
*********	*******			
TOTAL DISPOSAL VOLUME	2,881,150			

^{*} These cells may be combined into one or more larger cells. If combined, the single cell volume for the combined cell is 611,000 cubic yards which increases the Total Solidified Cell Volume to 2,806,350 cubic yards and a Total Disposal Volume of 2,975,450 cubic yards.

5.2.3.2 Cell Surface Water Control

Surface water runon to the disposal cell area will be directed through the use of temporary berms. The surface water diversion system will be phased in to allow the unused portion of the site to continue to be farmed during facility operations. Berms will be constructed around the open cell and also around subsequent cells as construction begins. Sections 5.2.3.7 and 5.4 includes additional surface water control information. The berms are trapezoidal in shape and are constructed to a minimum height of 3 feet. They are placed to divert water from the west and south of the cells to areas away from the filling face as shown in Plate 2. This plate includes diversions for only the initial 40 acres of filling area. As filling continues to the cells on the southern boundary of the site, the temporary diversion berms will be constructed to a minimum height of 3.5 feet.

The cells are constructed to allow direct rainfall into the cell to drain from the perimeter of the cell into the sump. The cells may be constructed in phases to allow segregation of runoff water and to minimize the possibility of liner failure. Water touching the working face will be collected and solidified. Water not touching the working face will be collected and used for dust control or will be solidified. If evaporation has not taken place within 72 hours, the standing water will be pumped from the cell to be used for dust control or solidified in the mixing basin, as appropriate. Section 5.2.3.5 includes additional information on the segregation and handling procedures of runoff.

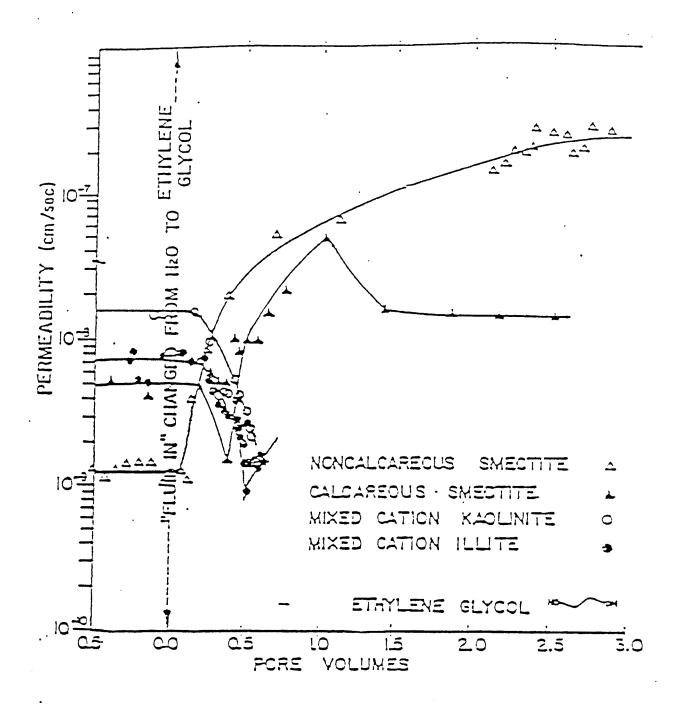
5.2.3.3 Disposal Cell Design and Construction

Disposal cells will be constructed that will vary dimensionally This section is a general discussion of typical and in depth. designs and construction criteria -- refer to the specifications in Appendix Q for more specific information. Individual cell dimensions may change. Prior to initiating cell construction, a cell design will be submitted to Adams County for approval. Depths will vary in order for the top of the clay liner in the sump to remain a minimum of 30 feet from water located in perched zones underlying the site. The average depth of the finished filling area ranges from 8 to 40 feet. There are also 4 triangular-shaped cells in the southeast corner of the site. Diagrams and construction details for the cell liners are included on Plate 7 and Appendix Q. Cells may be constructed in phases with the exception of triangular cells that will be completely constructed.

The cell liner construction will be compatible with the types of non-hazardous solidified wastes placed in the cells. Ethylene glycol is a typical substance that is solidified and disposed of at the CSI site. Ethylene glycol has not been shown to permeate high density polyethylene and clay liners in a liquid state as shown in the graph in Figure 5.2 (EPA, 1986). Compatibility information is discussed in Appendix N. The mixed cation kaolinite is a similar type of clay to that which will be used to construct the liner. The permeability of ethylene glycol through the clay actually decreases an order of magnitude from 10.8 to 10⁻⁹. In test data submitted to the Pennsylvania Department of Environmental Resources (PDER), the effects of a waste comprised of one part oil contaminated soils and four parts water are shown (EPA, 1986). All the results show permeabilities in the range of 10⁻⁷ or less (Appendix E). D'Appolonia Consulting Engineers, Inc. also completed permeability tests on four waste leachates used as permeants through a silty, clay soil. The results showed all permeabilities to be in the range of 10⁻⁷ and are included in The results showed Appendix E (EPA, 1986). Calculations included in Appendix M show 99.4 percent of the material reaching the liner draining to the leachate collection system enhancing the compatibilities of the solidified materials with the clay liner. If leachate is not allowed to remain standing on the liner, the clay barrier will not be penetrated and degraded. The use of the Paint Filter Test prior to removing the waste from the basin will ensure free liquids are not entering the disposal cells.

Each cell will be constructed with a low permeability clay liner system that incorporates a high density polyethylene (HDPE) synthetic liner and leachate collection system at the base of the cell. The liner system includes a 2-foot compacted clay liner underlain by a 3-inch sand blanket (defined in the specifications) used as a capillary barrier. A 30-mil (minimum thickness) HDPE synthetic liner will be placed on top of the 2-foot clay liner in the base only. A 6-inch leachate collection blanket will be placed on the synthetic liner material as per specifications.

The cell will be initially overexcavated approximately 3 feet deeper than the final filling depth to properly construct the liner system. Varying volumes of soil will be removed from each cell excavation and are approximately the same as airspace volumes shown in Table 5.3. Again, these may change depending on cell configuration. Of the soil excavated, topsoil will be



Source: Anderson, 1981

FIGURE 5-2

Conservation Services, Inc.

CLAY LINER COMPATIBILITY WITH ETHYLENE GLYCOL

Industrial Compliance inc.
511 Orchard Street
Solden Colorado 30401



stockpiled separately from the other soils for later use in the final cap. An additional amount of clay material will be set aside for use in the liner system as well as soils used to cap and close cells filled to capacity at the time of excavation. The side walls will also require excavation beyond the actual filling area of the cell but only to a depth of 2 feet. The excavation of the base of each cell will be sloped to ensure positive drainage to the leachate collection sump. These slopes range from approximately 1 percent to 3 percent throughout the site. In addition, each cell base will be sloped laterally to its longitudinal center at a minimum 1 percent grade to ensure positive drainage to the leachate collection pipe. The lowest portion of the base of each cell is overexcavated to incorporate the leachate collection sump.

It should be noted that the topsoil removed during the excavation process must be stockpiled for later use as a portion of the cell cover. An ample amount of clay material with the proper geotechnical properties is available to cap the filled cells as each cell is filled to capacity. It may be necessary to stockpile the specified soils only for construction of the final cell cap and use soils from each subsequent cell excavation to complete the cover of the finished cell. In this way, topsoils will only require stockpiling for a short time period.

Excavated soils will exceed the need for lining materials. Portions of this amount of excavated soil will be used to construct berms on the north boundary of the site, and construct temporary diversions for routing surface water runoff. Soil will also be used to bring the portions of the site between the cells up to the grade of the closed cells.

In addition, much of the excess soil will be placed on the western third of the site and graded from the high point of the site to the western site boundary. The western boundary will also include a permanent channel to route runoff along the site. The soils will be unspecified and will be placed in an uncompacted state to allow revegetations and future uses as approved. Erosion prevention measures will be applied to excavated and stockpiled soils as detailed in Section 9.0 of this document. Fertilizer requirements are included in Section 9.0. Plate 11 includes a typical final grading plan for the site. Actual final grading may be modified depending on the amount of soils available.

The bottom liner will consist of a layer of capillary material, the compacted clay liner, the HDPE synthetic liner, and the drainage bed with the leachate collection system (Plate 7). The capillary bed at the base of the excavation will be the initial layer placed. It is a minimum of 3 inches in depth and acts to reduce the capillary action of the underlying natural materials and maximize liner efficiency. The capillary bed will be placed and graded to ensure a positive slope is maintained to the longitudinal center and low portion of each cell.

The compacted clay liner will be placed on the sand bed to act as a barrier to any infiltration of liquids from the cells. A sump will be constructed to collect liquids accumulating in the cell as a result of precipitation. A drainage pipe will collect liquids from the edges of the cell and route the liquids to the sump. A minimum of 1 percent slope will be maintained from the edges of the cell to the drainage pipe. The base underlying the drainage pipe will be sloped approximately 1 to 3 percent to the sump.

The clay will be placed in lifts appropriate to the equipment used (approximately 6-inches) and will be compacted until it reaches a dry density equal to or greater than 95 percent of the Standard Proctor Density, as per ASTM-D-698. The moisture content of the material shall be maintained between optimum and +4 percent of optimum using the same testing criteria. The completed liner includes at least 2 feet of clay compacted to these specifications.

The material used for the clay liner will have a Unified Soil Classification (USC) of CL, CH, CL-CH, or SC and will have no more than 20 percent plus-200 mesh size material. In addition, the soil used for liner construction will have a plasticity index of no less than 10. Material which meets these specifications will have a recompacted permeability of 1 X 10⁻⁷ cm/sec or less.

Based on the geotechnical testing completed on samples collected from the site, there is ample material meeting these specifications available to complete the clay liner in each cell. Recompacted permeabilities on samples collected and tested were found to be in the range of 2.71 X 10⁻⁸ to 5.12 X 10⁻⁸ at 95 percent of Standard Proctor Density at optimum moisture content. Appendix E includes the geotechnical testing results of soil samples collected from the site.

The side walls of the cells will have approximately 3 to 1 slopes and will be lined with clay materials meeting the same specifications as those of the bottom liner. There will not be an underlying sand blanket, synthetic liner, or drainage layer for a leachate collection system on the side slopes.

The liner may be constructed over the cell in phases and will be protected from the elements by covering it with solidified waste or unspecified soil. Section 5.2.3.5 includes additional information regarding the phased construction of the cells. If the liner is left exposed for a period exceeding 60 days, it will be scarified, recompacted and retested.

A high density synthetic liner of minimum 30-mil thick polyethylene (HDPE) will be installed over the base of each disposal cell immediately above the compacted clay liner. The manufacturer and installation contractor shall meet the specific requirments as detailed in Appendix Q.

The clay liner will be graded and smoothed. This will assure that no protrusions, sharp grade changes, or anomolous features will interfere with the integrity of the synthetic liner.

The synthetic liner will be keyed into the side-walls of the disposal cells through the use of an anchor trench. Details of the anchor trench construction are found in the specifications in Appendix Q. The clay liner material excavated from the trench will be replaced on top of the synthetic liner, recompacted as necessary to ensure a minimum of two feet of compacted clay liner and brought up to grade.

Synthetic membrane panels will be visually inspected for tears, punctures, and thin spots before final placement. The owner or owner's agent shall have the right to reject any material deemed unsuitable for the intended use.

Seaming of the synthetic liner panels will be kept to a minimum and only approved field seaming methods will be accepted.

Stringent procedures will be followed during the installation of the HDPE synthetic liner. Both destructive and non-destructive tests will be performed on seams according to the methods and procedures outlined in the specifications. All repairs will be subject to non-destructive testing.

The entire facility operational area and opened cells will be fenced to prevent unauthorized access. Multiple cells may be enclosed within a single-fenced area. The fence may be removed following cap construction completion in order to allow farming to continue. If the land will not be farmed, the fence may remain and the land will be revegetated as necessary. When revegetation has been completed and healthy plantlife is growing, the fences may be removed and the land may be used for grazing or other approved uses by Adams County and the Colorado Department of Health.

5.2.3.4 Leachate Collection System

This is a general discussion--refer to Appendix Q in the specifications for more detail. The leachate collection system, installed on the top of the clay liner, will be constructed in three phases, as outlined in Section 5.2.3.5, and will ultimately drain any free liquids from the cell to a sump for removal. The hydration and pozzolanic processes taking place during solidification, much like that of concrete, results in the absence of free liquids. Therefore, the leachate collection system is designed to remove liquids that may be the result of heavy rainfall or precipitation events. Appendix M includes calculations used in determining the design criteria for the system.

The leachate collection system will be constructed on top of the synthetic liner. The initial excavation of the cell includes overexcavating the leachate collection sump in order to place a clay liner to a depth of 2 feet under the sump. The clay and synthetic liners will be sloped to drain leachate from the edges of the cell to the drain pipe and on to the sump. Typically, the sump is located at the eastern edge in the center of each cell as shown in Plate 7, but may be located elsewhere depending on the actual cell configuration. The collection system includes a 6inch drainage layer as defined in the specifications overlying the entire synthetic liner. The drainage material was used to fill the sumps in Cells #1 and #3 to an approximate depth of 2.5 Future sumps will use an appropriate material as approved by the regulatory agencies and as outlined in the specifications. A perforated pipe(s) will extend underneath the drainage layer

and be located according to the specific cell design. The pipe will be capped at the upgradient end and will extend to the sump. The sump itself is a drainage material-filled area approximately 3 feet by 3 feet and constructed to a depth of 2 feet. The dimensions of the sump may vary during construction but will meet requirements stated in the construction specifications. In addition, the pipe will be wrapped with a filter fabric to ensure it does not become plugged.

A collector well will be placed in the center of the sump. The well pipe will be slotted in the bottom portion of the sump to detect and remove any free liquids in the sump. The well will be placed to the ground surface following the 3 to 1 side slope or as necessary to assure integrity of the well. Completion will consist of securing the well stickup and placing steel protectors around the pipe. The collector well will be monitored on a periodic basis during filling of the cell and following capping of the cell as per Section 7.1. The pipe will be covered with a minimum of 3 feet of solidified material to protect it from heavy equipment.

The entire drainage layer will be overlain with a geotextile fabric to ensure the drainage layer system remains open and does not become filled with silt. The fabric material will be placed atop the drainage layer prior to placement of the solidified materials. In order to protect the fabric an appropriate amount of material will be placed on the fabric prior to allowing equipment on it.

The results of the calculations completed and presented in Appendix M indicate that the liner and leachate collection system described above will more than adequately protect the natural materials and any ground water beneath the cell from infiltration of liquids.

5.2.3.5 Phased Cell Construction

The liner systems of the cells may be constructed in a phased manner depending on final cell design configuration. Phase One of each cell will be in the upland portion of the cell and will be constructed following excavation. The liner system, including capillary bed, the clay and synthetic liners, and the leachate drainage system, will be placed and feathered down to existing natural materials at the base of the cell. A berm of appropriate height will be constructed on top of the clay portion of the liner system to collect any runoff that touches the working face. The leachate collection piping will be left exposed to allow the

next phase of the system to be constructed as Phase One nears filling capacity. As Phase One nears filling capacity, the construction of Phase Two will begin. Phase Two construction will include placement of the liner system over the next portions This phasing will continue until the liner system is placed over the remaining portions of the cell and sump area. The leachate collection portion of the system will be constructed and any temporary berm will be removed. Waters touching the working face will be removed within a 72-hour period and solidified in the mixing basin and will be analyzed two times per year for RCRA hazardous characteristics (including TCLP List, ignitability, corrosivity, and reactivity). Waters not touching the working face will be removed within a 72-hour period and will be either used for dust control or will be solidified in the mixing basin. Plate 7 includes typical cell construction and filling details.

5.2.3.6 Waste Placement (Not Including Non-Friable Asbestos)

Each of the cells at the site will be filled in basically the same manner. The cells will be constructed as outlined in Section 5.2.3.5 and Appendix Q and will be filled as appropriate to the actual cell design. Plate 7 includes typical cell construction details.

The waste will be transported by dump truck from the mixing basins to the disposal cells for final placement. The dump trucks will place the material in the cell at its edge and a bulldozer or front-end loader will place it into the cells by carefully pushing it into the cell in a manner that protects the liner system. The pozzolanic process of the material allows final solidification without using the typical compaction techniques normally required at sanitary landfills. Instead, a single pass over the material with the spreading equipment is sufficient for compaction. Waste materials will be placed in this manner to the top of the clay liner on the sideslopes of the cell. Waste may also temporarily be piled above ground level while the next cell is being constructed. Proper berming or routing will be performed to ensure any runoff is contained within the limits of the cell boundary.

As filling progresses, additional phases of the cell may be constructed as outlined in Section 5.2.3.5 and Appendix Q. As filling nears the sump area, the collector well will be constructed and the sump will be filled with drainage material. The initial slotted portion of the riser pipe will be placed at the time the sump is filled with drainage material and is

completed at the surface of the cell. A cell is completed when the waste material reaches the top of the clay liner.

The overall filling of the cells progresses from the cells in the eastern portion of the site to the cells in the west. shows the typical cell layout. Cells may be opened for disposal in order of necessity or as market conditions dictate. surface water diversion berms will be constructed as needed. During the life of the site, topsoil will be stockpiled for use in revegetation. Also, suitable soils for construction of the cell liner will be set aside during excavation of the cell. remaining materials used for the final cap and cover may be taken from the cell being excavated concurrently with the closure of The topsoil and other materials will be the previous cell. placed in segregated areas to ensure an adequate amount of topsoil for use in revegetating the cell caps. In addition, the topsoil stockpile will be revegetated to reduce erosion from wind and precipitation if it is expected to remain unused for a period exceeding six months.

Non-friable asbestos may be placed in the solidified waste disposal cells as per the CDH letter from Poul E. Poulsen, dated December 10, 1990.

5.2.3.7 Cell Cap and Cover Placement

Each cell will be capped to minimize the infiltration of surface water. The cells will be filled to the top of the clay liner and capped with a low permeability clay-cap system. Final cover over each cell will be a minimum of 4 feet in thickness, with the bottom 2-foot cap consisting of a compacted clay cover meeting the specifications presented in Appendix Q. The top 2 feet consists of 18 inches of uncompacted and unspecified soils overlain by 6 inches of topsoil. The final cover will allow for runoff and revegetation without construction of a crown. The topsoil will then be reseeded as per Section 9.1. As adjacent cells are filled, the areas between the cells may be filled with soil as necessary to smooth the site grades.

Clay cap construction may be completed in phases during the open period of a cell to help prevent infiltration of surface water into waste materials. Infiltration of surface water into waste materials will be controlled through: 1) sequencing of cell-filling; 2) covering areas filled to final grade with 6-inches of compacted clay cap material to minimize infiltration and promote run-off; 3) construction of temporary diversion berms to route surface water away from the cell and working face; 4) placement

of unspecified fill material on the uncapped portion of the cell to prevent trucks in transit to the waste unloading area from picking up contaminated materials. Details of each of these components follows.

- 1) Filling shall begin in the topographically highest side of the cell which is completely lined and ready to accept waste. This will minimize the area of waste exposed to precipitation.
- 2) Once approximately one-half of the area of the cell approved for filling has been filled to final grade, approximately 6-inches of clay cap material will be compacted on top of the first one-third filled portion of the cell area of the cell approved for filling. When approximately three-quarters of that portion of the cell approved for filling has been filled to final grade, the next one-third of the filled portion of the cell area approved for filling shall receive the 6-inch clay cap In all cases sufficient working area in front material. of the face (50'-100'±) shall be left uncapped. as possible, disposal truck and equipment traffic shall be kept off such temporarily capped area. The thickness of the clay cap material will be controlled and verified with construction fill stakes. Material used for this partial cap will meet the requirements for clay cap material as presented in the construction specifications in Appendix Q. Because this is a short-term water control solution, compaction will be limited to the energy transmitted by the scrapers and other equipment during placement of the material. Specific compaction and density testing is not necessary. When final capping of the cell is started, this 6-inch layer will be scarified, processed, recompacted, and tested to meet the requirements for clay cap presented in the construction specifications.
- 3) Temporary berms will be constructed as necessary around the perimeter of the cell to prevent run-on to the cell (Section 5.4). Temporary berms will also be constructed on the cell to prevent precipitation that falls directly on the cell from contacting the working face and to promote run-off from the cell. The height and alignment of the berms will depend on their location and surrounding topography. Berms will be designed to route the 5-year, 24-hour storm safely off the area.

4) A minimum of 6-inches of unspecified soils will be placed on top of the 6-inches of clay cap to protect it from dessication. Unspecified soils will also be placed as necessary on the uncapped portions of the cell to minimize the potential for vehicles to pick up waste materials on the tires and transport it to capped portions of the cell. Vehicle traffic will also be routed in such a way to minimize the potential for vehicle damage to the temporary cap. The unspecified soils will be removed prior to recompaction of the underlying 6-inches of clay cap material for final capping.

The size and configuration of partial caps and temporary berms will be dependant on individual cell design and dimensions.

5.3 Asbestos Disposal Cell

The CSI facility is designed to include an asbestos disposal cell. The asbestos cell is a separate cell from the solidified non-hazardous waste cells and will be used only for the disposal of asbestos-containing materials. As required by Colorado regulations, all friable and uncontained non-friable asbestos will be disposed only in this cell. All asbestos-containing materials will be handled according to appropriate regulations.

A pad may be constructed for temporary storage of containerized asbestos waste before placement into the disposal cell. Holding times will not exceed regulatory time limits. The asbestos disposal cell is located in the northeastern corner of the operations area, immediately east of the facilities area and immediately north of Solidified Waste Cell #1 (see revised Plate The cell will be approximately 555 feet by 400 feet (surface dimensions) with an average depth of approximately 27 feet to the base of the cell (top of the clay liner). Although asbestos is not a potential ground-water contaminant, a two-feet thick clay liner will be constructed in the bottom of this cell to minimize the potential for any water which may occur to migrate from the cell. Revised Plate 1 shows the layout of all the cells at this site and revised Plate 8 shows a generalized design and fill plan for the cell.

The asbestos cell will have a total airspace capacity of approximately 169,100 cubic yards. This cell will be capable of containing approximately 326,120 55-gallon containers. The

estimated life of this cell is dependent on the rate of disposal, but will not exceed the proposed 20 to 25 year life of the site.

5.3.1 Asbestos Cell Design and Construction

The asbestos cell will be excavated to an average depth of approximately 29 feet (approximately two feet below the finished base of the cell). The bottom of the cell will be sloped to the east at a grade of approximately three percent (3%) to facilitate drainage toward a sump which will be constructed in the east end of the cell. The sides of the cell will be sloped at approximately 2 (horizontal) to 1 (vertical) (2:1, see revised Plate 8). The cell may be expanded in the future with the approval of Adams County.

A clay liner and a leachate collection system will be constructed on the base of this cell (on top of the clay liner). After each phase of the cell is excavated to grade, a three-inch thick capillary barrier of sandy material will be placed in the bottom of the excavation. A 2-feet thick clay liner will be constructed over the capillary barrier. The clay liner will be constructed to provide the minimum grades of one percent from the sides of the cell toward the middle and one to three percent from the west end of the cell toward the east end. Although asbestos is not considered a potential ground-water contaminant and water is not expected in the cell, a 6-inch thick layer of drainage material (permeability >1x10⁻² cm/sec) will be placed over the clay liner. A drainage pipe will be placed in the center of the cell to facilitate the transport of any collected liquids toward the east end of this cell. A filter fabric will be placed over the drainage layer prior to placing any waste in this cell.

Initially, any waters collected in the western (disposal area) portion of this cell will be disposed as described for the solidified waste cells (removed after 72 hours, solidified and disposed in one of the solidified waste cells). In the future, CSI may conduct studies which could determine if waters ponding in the cell may be disposed in other ways. These studies could include such things as:

* A study of the potential contaminants which may be associated with asbestos waste materials (surfactants and other products which may have been used for the application and removal of asbestos-containing materials); and,

* Chemical analyses of waters which accumulate in the cell to determine if solidification is required.

The results of any studies supporting alternate disposal will be submitted to the appropriate regulatory agencies. Alternate methods for disposing of such waters will not be used without approval by the appropriate regulatory agencies.

The cell will be excavated and filled in two phases. The cell will be filled using approximately five to eight lifts, depending on the shape and size of the rigid containers, if the containers are buried on end or on their sides, and the amount of fill placed between lifts. The first phase of excavation has been completed and includes approximately the western two-thirds of the cell. The clay liner and leachate collection system are constructed over approximately one-half to two-thirds of this first phase (see revised Plate 8). As Phase One fills to capacity, the remainder of the cell will be excavated and the rest of the clay liner and leachate control systems constructed in the bottom of the cell. Phase Two filling will include the remainder of the cell.

The asbestos cell's perimeter has been entirely fenced as per the Colorado regulations. Access to the cell will be through a gate at the southwest corner of the cell. A storage area will be set in the southeastern corner of the fenced area (near the southeast corner of the Phase Two excavation - see revised Plate 8) and will be segregated with hazardous warning tape or a small fence with signs or warning tape. The storage area will be moved from the Phase Two area to the Phase One area when construction begins on Phase Two.

5.3.2 Waste Placement

Friable and non-friable asbestos wastes will be accepted for disposal at the CSI facility. The friable and uncontained non-friable waste will be placed in the asbestos disposal area which is in a permanently segregated area away from the non-hazardous solidified waste disposal areas.

Non-friable asbestos waste will be accepted in either a contained or uncontained state. If it is contained in plastic 6-mil (or similar) bags or in rigid containers, the non-friable asbestos can be kept in the storage area prior to being placed in a cell. Any uncontained non-friable asbestos accepted at the facility will immediately be placed in the cell, covered with a minimum of six inches of soil, and compacted. Bagged non-friable asbestos

also will be covered with a minimum of 6 inches of soil. Rigid containers will not be compacted. The handling of the non-friable asbestos will be done in such a way as to minimize any increase in friability of the waste.

Friable asbestos will be accepted only if the waste is packaged in containers as required by the Colorado regulations. The containers are to be labeled with the following words:

CAUTION

Contains Asbestos
Avoid Opening or Breaking Container
Breathing Asbestos is Hazardous to
Your Health

The containers will be covered with a minimum of six inches of soil within three days of placement in the cell. The rigid containers with friable asbestos will not be compacted.

All asbestos waste will be accepted on a pre-arranged schedule only. The waste will be unloaded from the transport vehicle at the storage area (see above). If the asbestos is non-friable and uncontained, it will be taken immediately into the asbestos disposal cell and covered with a minimum of six inches of soil. If the non-friable waste is packaged for disposal, it can be held in the storage area for up to 20 days before being placed in a cell. If the waste is friable, it must be prepackaged in containers such as required by the Colorado regulations. These containers may be held in the storage area for up to 20 days before being placed in the cell.

Wastes will be placed in the cell as shown on revised Plate 8.

- * From the storage area, wastes will be transported down the east side of the cell to the bottom of the cell.
- * Ramps will be constructed over the water control berm and over the drainage layer along the north and west sides of the cell to the southwest corner of the cell. The "ramps" along the north and west sides of the cell over the drainage sand and filter fabric will be approximately 6 inches thick and will be constructed of common fill material. These "ramps" will protect the filter fabric and drainage layer from damage during the transport and placement of asbestos waste in the first lift.

* Asbestos wastes initially will be placed in the southwest corner of the cell. A "pad", constructed of common fill material to a thickness of approximately six inches, will be built in this corner of the cell. Asbestos materials will be placed on this "pad". As the "pad" is filled, it will be enlarged to accommodate additional wastes. Drainage fabric will be placed over the drainage sand only as the "pad" is enlarged. This method of placing the fabric will minimize the potential for the fabric to be damaged by wind and weather.

Containers (usually 55-gallon drums) will be placed vertically; vertical placement is expected to minimize the potential for damage to the containers and to increase the potential for filling any void spaces between the containers. All waste materials and containers will be covered as required by Section 8 of the Colorado Waste Facility Siting Rules (CCR Title 6, Chapter 1007, Article 2). The fine-grained common materials used to cover the asbestos wastes and containers will likely fill much of the space between the containers.

The first lift of waste materials will likely be completed before the second lift is started. The ramp(s) will be elevated to allow access to the second lift. Asbestos wastes and containers will be transported over the first lift using smaller loaders and other equipment with low ground pressures to minimize the potential for crushing containers placed in the first lift. Construction of the second lift also will likely begin in the southwest corner of the cell. Subsequent lifts will be constructed in the same manner as the second lift.

5.3.3 Waste Placement Revisions

Revisions to the method of waste placement may be made based upon on-site experience. Conditions which could result in changes to the proposed method of placement include, but are not limited to, excessive crushing of containers and excessive loss of air-space due to ramp construction. Revisions which may be made include, but are not limited to, constructing additional ramps to minimize travel distances across previous lifts, reducing the number of ramps to reduce loss of air-space, beginning placement of higher lifts before a lower lift is completed, placing waste in several areas (e.g., the northwest and southwest corners of a lift) at the same time, and placing containers on their sides instead of

vertically. Revisions to the proposed plan for filling the asbestos cell will be discussed with the regulatory agencies prior to their implementation. A written description of the revisions will be provided to the regulatory agencies.

All CSI employees handling asbestos wastes within the fenced asbestos area will be required to wear the appropriate respiratory protection and required environmental and medical surveillance as required by OSHA regulations 29 CFR, Parts 1910 and 1926. Asbestos wastes will be placed in the cell only by CSI employees.

The asbestos cell will be capped following filling to capacity. The cap will consist of two feet of compacted, unspecified soils, an additional 1½ feet unspecified soil, and six inches of topsoil. The cap will be brought up to grades similar to those across the site and revegetated as per the specifications used for constructing the cells at this site.

5.4 Surface Water Control

Surface water will be controlled to ensure that runon is not allowed to enter the cell's working face and to ensure that water falling on the working face is not allowed to leave the site. Surface water runon to the site will be controlled through the use of diversion berms. A permanent berm has been constructed on the site adjacent to 88th Avenue. The berm begins a minimum of 65 feet from the property line adjacent to 88th Avenue and will extend along the entire length of the site with the exception of approximately 680 feet in the northwest corner of the site. The berm will route runoff from the site and also act as a visual screen.

CSI is proposing to use temporary berms during filling activities to divert surface water from cells that are open (see Section 5.2.3.7). The use of berms, as opposed to the use of a permanent diversion channel around the entire site, will allow the undisturbed portions of the site to be farmed during operations. This will allow the site to remain compatible with the surrounding property uses. Temporary diversion berms will be constructed prior to cell construction, and following completion of the cell cap and cover, the berm will be removed so the area can be farmed again. The berms will be constructed to route water around all open cells. The berms to be constructed on the initial 40 acres of filling area are shown on Plate 2. The subsequent berms will be located to route runoff from the cells in a similar manner. The berm used to route runoff from cells on

the southern boundary of the site will be constructed to a minimum height of 3.5 feet. This will control potential runon from off-site.

It may be necessary to construct berms or ditches around waste materials temporarily stockpiled above ground level on active cells to contain runoff within the cell boundary.

A berm will also be constructed to route water from the asbestos cells. This berm will be maintained during the entire life of the cells and will be removed following completion of capping the cells. The berm will be constructed outside the fencing. The berms will be a minimum of 3 feet in height with 2-to-1 side slopes, with the exception of the berm used to route water from the open cells on the southern portion of the site. It will be a minimum of 3.5 feet in height with 2 to 1 side slopes. All temporary diversion berms will be constructed of unspecified soils. Before each subsequent cell is opened, a new berm will be built or the present berm will be added to. The berms will be inspected monthly and following each heavy rainstorm to ensure their integrity. Berm construction and location will also be noted in the as-built drawing required for each cell.

6.0 FACILITY OPERATION

6.1 Site Management

6.1.1 Operations Schedule

The facility will generally operate and receive wastes between the hours of 7:00 A.M. and 5:30 P.M., Monday through Friday. There is the possibility that increases in incoming volumes may require crews to work weekends or for extended hours during the week, although this is not anticipated at this time. The facility will be closed for all major legal holidays.

6.1.2 Personnel and Equipment Requirements

The CSI facility will employ approximately twenty people at the site. A more complete job description for each of those listed below is in Appendix L.

- * Facility Manager
- * Service Supervisor
- * Chemist
- * Lab Technician
- * Drivers/Operators
- * Laborers/Technicians

The facility has at its disposal the following equipment or equivalent available for use in operational activities:

- * D8 Dozer Ripper
- * 145 627 Scraper
- * 155 627 Scraper
- * #16 Blade
- * 815 Caterpillar Compactor
- * Kamatsu Backhoe
- * John Deere Loader/Backhoe
- * IT Loader 18 w/Fork lift, Boom, Blade, Bucket
- * Clark Loader
- * 1 Water Truck 3000 gallons
- * Welding Truck
- * G.D. Air Compressor
- * Generator Cat Set
- * Rome Disk
- * John Deere Disk

- * 3" Diaphragm Water Pump
- * 6" Trash Pump Water
- * Pump Master Water Pump
- * 2 Dump Trucks
- * R 30 Rock Truck

The personnel and equipment requirements may change as the amount of material being disposed at CSI increases.

6.1.3 Facility Inspections and Daily Logs

Visual inspections of the facility will be done on a daily basis with the results of the inspections recorded in a daily inspection log. A copy of the daily facility inspection log is included in Appendix K. The daily inspection will be done by the Facility Manager and/or the Chief Chemist and includes:

- * General housekeeping
- * Site security
- * Fencing
- * Lighting
- * Warning signs
- * Mixing basins
- * Tank farm
- * Dust storage
- * Truck wash pad
- * Unloading and loading areas
- * Asbestos storage area
- * Treatment and solidification procedures
- * Special handling procedures

In addition to the daily inspections and written log, CSI employees will inspect the safety equipment maintained for personal and company use. The inspections for personal safety equipment will be completed routinely by the owners of the equipment. These include inspections of respirators and other protective safety gear both before and after use. Supervisors will periodically inspect employees' safety gear to ensure proper maintenance.

The daily inspection logs will be kept at the facility for a minimum of three years. The inspection parameters may change as per the needs of the facility and requirements of state and federal regulations.

6.1.4 Recordkeeping

The operations at the CSI facility require an increased number of records as compared to a more typical sanitary landfill. This is, in particular, because of the amount of analytical work required prior to acceptance or rejection of a waste stream. Additionally, adequate records must be kept to track the amount of asbestos materials disposed of at the site. Other operating records commonly kept at sanitary landfills are included at the CSI facility. All records will be made available to the Adams County Board of Commissioners, Tri-County Health Department (TCHD) and CDH upon their request. These forms are included in Appendix K.

6.1.4.1 Non-Hazardous Liquid Waste Recordkeeping

Records will be kept on the amounts of incoming non-hazardous liquid wastes to be solidified. These records will be kept on a daily basis and will include the type of material (analysis results), amount of material (gallons, tons or cubic yards), amount of material following solidification (cubic yards), and the cell the material was placed in. The records will also include the date the material was delivered to the site, solidified and placed in the cell, and the generator of the waste stream. These records will be kept for a minimum of 3 years.

In addition to records of the wastes accepted at the site, written documentation will be kept on wastes not accepted at the site. This documentation will include the application date, generator of the waste, analytical testing results, and reason for the non-acceptance of the waste. The types of analytical recordkeeping required are included in Section 5.1.

Each solidified waste disposal cell location will be surveyed and documented into a permanent record that CSI will retain throughout the life of the facility. A permanent benchmark will be placed on the site in order to correctly locate and survey the cell dimensions and accurately reproduce the results. The survey will include an as-built drawing of each cell with the perimeter of the cell at ground level, the slope of the side walls of the cell, the cell liner construction details, the leachate collection system details, and the total filling depth of the cell. As-built drawings will be submitted to CDH, TCHD, and Adams County within 30 days of construction completion. The cell record will also include the beginning filling date, the date the final load of material was placed in the cell, and the date the cell cap and cover was completed.

6.1.4.2 Analytical Recordkeeping

Recordkeeping requirements for the analytical results of the samples are included in Section 5.1.

6.1.4.3 Asbestos Waste Recordkeeping

Records of the asbestos disposal cells will be kept and retained at the CSI facility for the life of the facility. The permanent record will include:

- * As-built drawing of the disposal cell with survey coordinates
- * Quantity of incoming waste
- * Generator of the waste
- * Date the waste was received

The cell depth, date of placement of the initial load, date of placement of the final load, total amount of material placed in the cell, and date of the cell cap completion will also be recorded. The records will be submitted to the Adams County Commissioners within 30 days after closure of each cell.

6.1.4.4 Miscellaneous Recordkeeping

Records will be kept at CSI for other miscellaneous operations and occurrences at the CSI site such as:

- * Personal injury on-site or during the course of work on a CSI project
- * Spills of materials on-site in excess of 50 gallons
- * Detection of leachate in the cells
- * All monitoring records for ground-water sampling
- * Liner damage and all actions completed to repair the liner
- * Liner integrity during facility inspections

6.1.5 Site Security

The entire perimeter of the facility operations area and the asbestos cells will be permanently fenced for the life of the facility to prevent unauthorized access. The fence will be a minimum of 8 feet. The main gate will be located on the access road into the facility from 88th Avenue (Figure 2.3). The main gate will be unlocked and kept open during business hours. Keys to the gate will be given to the area fire department in the event of an emergency. The facility operations area will be

lighted to provide additional security. Nighttime security lighting will include a light pole in the northcentral portion of the facility operations area, a light mounted on the north side of the kiln dust storage shed, and lights mounted on the east and west sides of the administrative office which illuminate the entrance gate. Additional lighting is provided by four lights mounted on the open side of the kiln dust storage shed and flagpole lighting near the entrance gate. All cells opened and available for disposal will also be within a fenced area for safety and security purposes. The entire site perimeter will not be fenced in order to allow the unused portions of the site to be farmed during the life of the site. CSI will also remove the fences from the closed cells if the land is farmed again. farming in the closed cells is not suitable, the cell will remain Fencing specifications for the cells will be the same as for the operations facility.

The asbestos disposal cell within the CSI site will be fenced off from the remainder of the site for safety and security reasons. The asbestos disposal areas barrier will include a portion of the site perimeter fence along the north and east boundaries and another fence that will be constructed and maintained around the remaining perimeter of the cell. The asbestos storage area is located within this secure area and as such, does not require a separate barrier, however, when asbestos is stored in the area, the containers are marked (as shown in Section 5.3.2) and the storage area perimeter will be outlined using yellow caution tape and placing warning signs. The fenceline outlining the asbestos disposal area includes two signs on each of the boundaries. rectangular warning signs on the perimeter fencing will be located within 300 feet of each other and have the minimum measurements of 20 inches by 14 inches. The legend on each sign will conform to standard regulations.

All spacing between any two lines must be at least equal to the height of the upper of the two lines. The storage area is posted on the caution tape with warning signs as outlined above with exception that the posted sign says:

Asbestos Waste Storage

instead of "Asbestos Waste Disposal Area".

Access to the asbestos disposal area will be through a gate along the western fenceline of the operations area. Asbestos materials will not be stored in the facility operations area. The storage area for asbestos is located in the northeastern corner of the fenced cell area. CSI will accept friable asbestos only if it is packaged in rigid containers and properly labeled. The gate is open only during unloading and when CSI personnel are working in the asbestos disposal area.

Additional security measures may be taken to protect employees, visitors and equipment at the site as is deemed necessary by the CSI Facility Manager.

6.2 Control of Nuisance Situations

The nuisance situations created by the type of disposal taking place at the CSI facility are substantially less than those found at typical sanitary landfills. There is no waste being disposed of at the CSI facility which could cause a vector problem, and as such, vector, bird and rodent control measures are not necessary. Litter problems are not addressed in this section for the same reason. The conditions that are addressed in this section are dust, odor, fire prevention, and controlling water in open cells.

6.2.1 Dust Control

Dust will be created at the site because of the use of gravel access roads and the use of the solidification agents. Dust on the site roads will be controlled by watering the roads with the water truck. Water for dust control will either be purchased, pumped from water collected in the "clean" portions of the cells, or taken from a well (Sections 5.2.3.2 and 5.4). In addition to the use of water, the site supervisor may also choose to place calcium chloride on the roads for dust control. Application of these materials is discretionary and dust control is the responsibility of the Facility Manager.

The solidification agent dust is controlled by storing the agent in a building enclosed on three sides. There will not be any solidification agent stored outside the boundary of the building. Care will be taken in transporting the solidification agent between the storage building and the mixing basin to minimize spillage of the agent. Any solidification agent that is spilled and exposed to potential wind will be cleaned up and either replaced in the storage building or put in the mixing basin. The

area between the storage building and the mixing basin will be watered as necessary to control dust. A fourth wall on the east side of the storage building will be added if other controls prove to be ineffective.

The winds at the site will not create the same types of blowing litter problems as typical sanitary landfills. Therefore, there is no wind speed parameter set for shutting down operations. Should wind cause a dust problem at the site, it is the Facility Manager's responsibility to determine whether operations will be halted until the wind subsides. CSI will obtain a fugitive dust emissions permit from the CDH prior to beginning operations at the site.

6.2.2 Odor Control

Odors are not a frequent problem at the CSI facility, although control of odors is addressed in this section for the occasional If a particular waste stream creates an times the need arises. odor problem, it is immediately placed in the basin, mixed with the solidification agent and removed to the disposal cell. for some reason the material cannot be solidified immediately, it will be placed in an enclosed holding tank in the tank farm to limit escape of the odors. Materials that are placed in the disposal cell with a persistent odor will be covered with the amount of soil necessary to alleviate the problem. The amount of cover soil used is left to the discretion of the operator and may Typically, cover soil is not required vary with materials. during placement of material in the cells, and it is possible that odor problems will be controlled through the placement of additional solidified material over the odorous material. odors typical at the site are expected to dissipate within a few hundred feet of the cell.

6.2.3 Fire Prevention

The CSI facility is within the boundaries of the Bennett fire District (BFD). The BFD will be notified of the types of services offered at CSI and the potential fire hazards involved, prior to beginning operation of the facility. The BFD will also be supplied with a blueprint of the facility showing the location of the various offices, the laboratory, and maintenance shop as well as being notified about the hazards associated with the materials stored in each (such as lab chemicals and cleaning agents). Keys to the facility gate will be kept with BFD to

allow entry during the hours CSI employees are not on-site. CSI will conduct a site tour to familiarize BFD firemen with the facility and will make every effort to keep BFD informed about the site's progress.

Although there will be no burning of waste on-site, each piece of heavy equipment used at the site will have a fire extinguisher on-board to use in the event a small fire occurs. Equipment operators will be trained in the use of the extinguishers. Other fires can be extinguished by excavating the burning area and covering it with soil.

6.2.4 Controlling Water in Open Cells

Water will accumulate in cells from precipitation. This water will be segregated into water touching the working face and water not touching the working face (see Section 5.2.3.7). If weather conditions permit, runoff from the working face will be evaporated to the atmosphere. If water has not evaporated within 72 hours from the time it fell, it will be collected, solidified, and disposed of into the cells. Water touching the working face will be analyzed 2 times per year for RCRA hazardous characteristics including TCLP List, corrosivity, ignitability, and reactivity. The results will be evaluated and may dictate the disposition of the water. Clean runoff, that has not touched the working face, will be pumped if it has not evaporated within 72 hours and will be used for dust control or will be solidified.

6.3 Health and Safety

The operation of the CSI facility and the services provided by CSI make it necessary for the employees to have an understanding of health and safety requirements. This understanding or knowledge is gained in part by attending training programs and through field experience. It is the intention of CSI to hire qualified individuals to fill positions. When qualified individuals are not available to fill a particular position, the hiree will be trained for the position and closely supervised during an initial probationary period.

CSI has in place formal Contingency Plans and Safety Rules and Guidelines which cover emergency operations as well as everyday operations at the site.

The Contingency Plan is a preparedness and prevention plan designed to minimize the possibility of fire, explosion, or unplanned sudden or non-sudden release of potentially dangerous,

hazardous, or environmentally threatening materials. The plan describes the action which will be taken by company personnel in response to an emergency. A list of emergency coordinators, state and local authority contacts, responsibilities, and procedures is included in the plan.

The Safety Rules and Guidelines are designed to prevent accidents, emergencies, and unsafe conditions. CSI believes that safety is essential for the smooth operation of any job and expects employees to contribute to the program by knowing and following all safety procedures.

Below are the training and health monitoring programs in place for employees of CSI:

Operator Training:

- * 40 hr. OSHA training
- * 8 hr. OSHA update once a year
- * D.O.T. certification
- * Yearly physical
- * Yearly driving test
- * Monthly safety meetings: spill control procedures, SCBA use, confined space entry procedures, etc.
- * Special classes: asbestos workers class, recognizing and identifying hazardous materials, fire fighting training, etc.

Field and Lab Technicians Training:

- * 40 hr. OSHA training
- * 8 hr. OSHA training update once a year
- * Yearly physical
- * Monthly safety meetings
- * Special classes

New Employees:

- * 40 hr. OSHA training
- * D.O.T. certification
- * Physical
- * Driving test
- * 3 months on-the-job training with experienced operator or field tech

- * Orientation on Safety Rules and Guidelines and Contingency Plan
- * New employee's role in emergency response and clean up procedures
- * Special classes

The yearly physical will include monitoring for exposure to Polychlorinated Biphenyls (PCBs).

CSI employees are trained to use the proper safety gear according to the particular hazard associated with the task being undertaken. During many instances, CSI employees are required to wear EPA-Level C safety gear. This consists of:

- * A half-face respirator with the proper cartridge
- * Tyvek or Saranex protective suit
- * Rubber gloves
- * Rubber boots
- * Hard hat with a face shield
- * Eye protection

CSI employees are responsible to ensure that their safety gear is adequate for the particular task and that the safety gear is in working order at all times. Respirators will be initially fitted by an individual experienced in the use and fitting of respirators and the CSI employee will be shown the proper techniques used to care for the equipment. Respirators are used during many of the operations at the facility including:

- * Mixing kiln dust and liquids
- * Placing kiln dust in the mixing basin
- * Unloading asbestos waste containers
- * Placing asbestos containers in cells
- * Unloading liquids to the mixing basin
- * Handling hazardous waste and materials
- * Other tasks as required by supervisor

The overall health and safety of the employees is of prime concern to CSI and is the responsibility of the Facility Manager. Periodic inspections of safety equipment and health and safety training programs will be completed by the Facility Manager, Chief Chemist or Supervisor.

Personnel (including truck drivers) not trained in the handling of hazardous materials will be excluded from participating in cleanups or other operational activities which may pose a risk to health and safety. CSI drivers and response personnel will

Revised July 1991

cooperate fully with the local fire, police and environmental agencies and their representatives.

Health and safety is the responsibility of the Project Supervisor during a project being completed off-site. The Project Supervisor must be trained and experienced in health and safety procedures. A new hiree with little or no experience in the field will not be sent to complete any field work without adequate supervision.

7.0 ENVIRONMENTAL MONITORING

Monitoring for leachate, ground-water contamination and overall process monitoring will be completed at the CSI facility on a periodic basis. Methane gas, although typically monitored at sanitary landfills, will not be monitored at CSI because the operation and the materials disposed of do not form the gas. All monitoring is to be conducted to ensure the facility is performing as designed.

7.1 Leachate Monitoring

The leachate collection system constructed in the base of the solidified material disposal cells will be used to monitor the formation of liquids in the cells. Each cell at the facility will be checked for the formation of liquids on a monthly basis following installation of the leachate collection well and removal system but prior to final capping. The combination of the liner system, and the fact that none of the solidified waste disposal cells are closer than a minimum 30 feet from the top of clay in the sump to any measured ground-water level, makes it very doubtful that any leachate would ever impact the natural If liquids are in a sump in a removable quantity, they will be pumped or bailed out and solidified. Monthly monitoring for the presence of liquids will preclude the possibility of any liquids remaining on the liner for any substantial length of Monitoring of the leachate collection system will be done on a quarterly basis, corresponding to the scheduled ground-water monitoring quarters, following final capping of the cell. capping is defined as the date that the 2-foot of compacted clay and the 1.5-feet of unspecified soils has been placed on the cell and verified. Prior to final capping of each cell it is likely that water will be collected as a result of precipitation events. Water found in the leachate collection system prior to, and including, the two consecutive scheduled ground-water monitoring quarters immediately following final capping of the cell will not be defined as significant leachate (Section 7.4.1). The leachate collection well and removal system is discussed in Section 5.2.3.4 and closure of the wells is discussed in Section 9.3.

The underdrain system for the mixing basin will be monitored on a monthly basis to ensure that if leachate were to form, it would not remain on the liner for any substantial period of time. The underdrain system is shown in the mixing basin design specifications on Plates 9 and 10.

7.2 Ground-Water Monitoring

The ground-water monitoring system was installed prior to placement of fill at the site and includes 12 monitoring wells. Plate 2 shows the locations of the monitor wells.

The wells are placed and constructed to monitor three situations present at the site:

- 1) Perched ground water: Wells 101-108 were completed into perched water zones on the edges of the site as shown on Plate 2. The water in these zones are hydraulically separated and chemically unequilibrated from each other. The wells provide detection information if a release from outer cells were to occur.
- 2) Alluvial ground water: The western portion of the site includes water at shallower depths related to an alluvial system. Monitor wells 201 and 202 were installed into this shallow ground water.
- 3) Denver Aquifer ground water: Well 301 was completed to a depth of 78.4-feet. Since well 301 contained water from the Denver Aquifer, well 302 was placed approximately 2000 feet east and was also completed into the Denver Aquifer at a depth of 89.8-feet.

The location of these monitor wells appears sufficient to determine if a release has occurred. Because the materials CSI disposes of are typically lighter than water or water soluble, the wells will be screened to include the upper zone of saturated material. In the perched zones, the saturated interval will likely be less than 10 foot in thickness and in these cases the entire interval will be screened. The slotted length of pipe was placed to approximately 3 feet above the top of the uppermost saturated interval in case of water level fluctuations. A typical diagram of the construction of a ground-water monitor well is included in Figure 7.1.

Temporal data was collected by sampling all wells a minimum of 4 times prior to beginning filling at the site. This data will be used in statistical analyses of subsequent monitoring episodes.

The wells will be sampled in accordance with Environmental Protection Agency (EPA) sampling protocols. Proper chain-of-custody procedures will be followed during each sampling event.

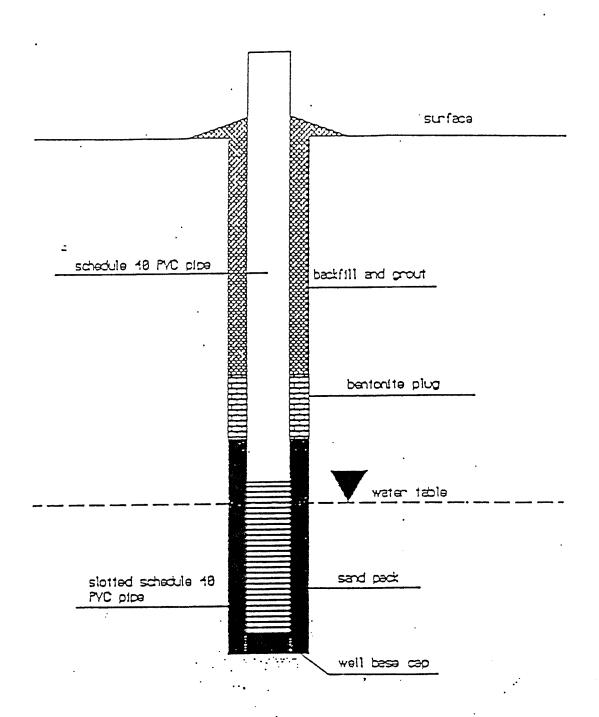


FIGURE 7-1 .

Conservation Services, Inc.

TYPICAL GROUND WATER

MONITOR WELL CONSTRUCTION

Industrial Compliance Inc. 511 Orchard Street Galden, Colorado : 30401



All water samples will be collected and analyzed for the following parameters four times per year to gather background water-quality data:

Arsenic Barium Cadmium Calcium Sodium Chloride Ammonia Specific Conductance (field) Total Organic Halogen (TOX) Magnesium Potassium Sulfate Total Dissolved Solids (TDS) Chemical Oxygen Demand (COD) Total Organic Carbon (TOC) Lead Iron Alkalinity Nitrate-Nitrite pH (field) Phenols

The operator of the facility shall make justifiable changes in the monitoring plan as needed if the appropriate regulatory agencies would like to amend the sampling parameter list. Once background data is established using the above parameter list, a reduced parameter list for quarterly monitoring may be established with regulatory approval.

7.3 Operational Monitoring

Site inspections and the operational monitoring at the facility are completed on a daily basis and the results are entered into a logbook as outlined in Section 6.1.3.

As an additional set of operational monitoring criteria, CSI will complete the following:

* Two times per year, for five years following approval of the revised D & O, a sample of High Organic Content Waste (HOCW) will be collected and analyzed to determine the effectiveness of the solidification process on HOCW. HOCW will be defined on a selection

process as follows: the sample selected will correspond to scheduled deliveries of known waste streams that contain approximately 2% to 50% oil, grease, or organic materials. The onsite laboratory will notify the CSI chemist when sufficient amounts of this waste stream exist in the mixing basin. A sample will be taken from the entire batch of this waste in the basin (app. 20,000-25,000 gallons) both prior to solidification and after solidification. The samples will be analyzed for TCLP (all compounds on the TCLP list), Oil and Grease, Total Organic Carbons, and Total Organic Halides. This HOCW study will be completed as a condition of the Adams County Permit.

- * A solidified waste sample will be collected and a geotechnical analysis will be performed to determine whether leachates are released under pressure. This test will be completed annually during the first two years of operations.
- * A sample of solidified material will be collected from a disposal cell and will be analyzed using the Toxicity Characteristic Leachate Procedure (TCLP) test. This analysis will be performed semi-annually during the initial 5 years of site operations.

The results of these tests will be evaluated and forwarded to the appropriate regulatory agencies.

In addition, the waste analysis may be amended to include other analyses if required by the EPA. The waste analysis plan may be further modified if it is determined by CSI, or appropriate regulatory agencies that the proposed analysis is inadequate to detect all hazardous materials.

The temporary berms constructed will also be inspected to ensure they are effective. Monitoring will be done to check for potential erosional damage and breeches in the construction. This will be completed on a monthly basis and after any substantial precipitation event.

7.4 Negative Monitoring Results

7.4.1 Definition of Negative Monitoring Results

Negative monitoring results for ground-water monitor wells at the CSI facility are defined as two situations. The first situation is if water is found in monitor wells that have previously been dry.

The second situation defined as negative monitoring results is taken from 40 CFR, Part 264.97(h). This section discusses statistical determinations relating to background sampling analytical results for hazardous waste facilities. Although CSI is not a hazardous waste facility, these methods, or similar methods of equal validity, is a generally accepted standard and also a conservative measure for the CSI facility. Because there is no up-gradient vs. down-gradient ground water system at the site, the method listed under Part 264.97 (h)(1)(i) is not appropriate. Part 264.97 (h)(1)(ii) and Part 264.97 (h)(2) allows for the use of equivalent statistical methods to allow for site-specific ground-water conditions. A statistical method negotiated with, and approved by the appropriate regulatory agencies will be used to define a negative monitoring event. background data used to analyze the parameters will be collected from four monitoring episodes prior to filling operations at the facility.

Additionally, the leachate collection wells in the cells are monitored and negative monitoring results are defined as the detection of significant leachate in the wells. It is, however, very likely that precipitation events will result in waters being collected in the sumps before they are covered with solidified materials. The removal of these waters will continue during the life of the cell and the post-closure period but notifiable monitoring will begin only after the sump can no longer be visually inspected.

Leachate detected in the underdrain system of the mixing basin also will be defined as negative monitoring results.

Any visible amounts of asbestos fibers released to the atmosphere will be defined as negative monitoring results.

7.4.2 Proposed Action in the Event of Negative Monitoring Results

If, at any time during the life of the site or during the postclosure period of the site negative monitoring results are obtained, the appropriate regulatory agencies will be notified within 5 working days of determination of negative monitoring The operator will request to meet with these agencies to discuss the results. If necessary, confirmation sampling and If the negative monitoring results testing will be conducted. are confirmed, the operator will evaluate the data and present a plan of action to the county and the state within 30 days. plan will include specific actions to be undertaken and a time schedule required to correct the situation. The plan will be implemented on approval by the appropriate regulatory agencies. Any situations that pose an immediate threat to human health or the environment will be corrected as quickly as possible. If leachate is detected in the leachate collection system, the appropriate regulatory agencies will be notified within five days from the date of detection. The leachate will be analyzed and probable source of origin will be interpreted. The suitability for onsite solidification and disposal will be subject to regulatory approval. Upon analysis, a plan of action will be submitted to the appropriate regulatory agencies which will include the amount of leachate, determination of whether the leachate represents a long-term or short-term problem, and corrective action to be taken. Regardless of formal requests by the regulatory agencies, Subtitle C corrective action procedures may be used if applicable.

If leachate is found in the underdrain system of the mixing basin, the basin will be immediately emptied and inspected for cracks or leaks. The cracks or leaks will be sealed prior to returning the basin to use. County Zoning and Certificate of Designation Regulations, State Health Department Rules and Regulations, Federal Rules and Regulations (including Subtitle C, if applicable), and any other rules and regulations by appropriate agencies that have jurisdiction at the time negative monitoring results occur, shall all be consulted to determine the appropriate procedures and corrective actions to be used.

Negative monitoring of the asbestos storage area or disposal cell will be corrected immediately. Any leaking containers detected in the storage area will be immediately placed in the cell and covered with a minimum of 6 inches of cover material. If leaks are detected emitting from the cell, an adequate amount of cover will be placed over the containers to abate the release.

8.0 CONSTRUCTION QUALITY ASSURANCE AND CONTROL

Construction quality control will be used to ensure that facility designs are implemented and the cells are built in accordance with the plan. This section will focus on the materials used to construct the solidified waste disposal cell liners and the caps for both the solidified waste disposal cells and asbestos waste disposal cell. More specific details of the construction Quality Assurance/Quality Control (QA/QC) are found in the construction specifications in Appendix Q.

8.1 Material Specifications

Different materials are used to construct the liners in the solidified waste disposal cells. These include clay for the base and side slope liner and cell cap; a synthetic liner on top of the base clay liner; material for a capillary barrier beneath the clay liner; drainage material to ensure proper drainage to the leachate collection system in each cell; a filter fabric to ensure the drainage material remains open for drainage; an unspecified soil to increase the potential for revegetation on the cell caps and protect the clay cap; and topsoil for revegetation of the cap. The specifications for these materials are outlined in Section 5.2.3.3 and in Appendix Q.

8.2 Excavation Inspections

To ensure that excavations for the cells are completed in accordance with the design standards, and that conditions met are consistent with those described in this Design and Operations Plan, excavation inspections will be made for each cell. The inspections will be conducted by an independent consultant who will then produce a written report that includes all observations made in each newly excavated cell. Copies of the excavation report will be submitted to both Adams County and CDH. The excavation inspections will be completed prior to placement of the liner in the solidified cells and prior to placement of any asbestos materials in the asbestos disposal cell. In addition, the configuration of the cell will be measured by a registered professional land surveyor, with the results included on an asbuilt diagram.

8.3 Clay Liner and Final Cell Cap Inspections

To ensure that the clay liners and cell caps for the solidified waste cells are constructed in accordance with the specifications contained in this report, an independent soils and testing firm will be employed to observe and test the materials prior to placement and following compaction. The program will consist of initial material classification tests for every 3000 cy of clay material placed in the liner and for every 2000 cy of clay material placed in the cap. This type of testing will ensure that the materials placed are suitable for their intended use. In addition, compaction and moisture content testing will be conducted on the compacted clay liners and cover. Specific requirments for the construction and testing can be found in the Specification in Appendix Q. Initial filling of the cells over the lined portions will not be allowed without an engineering report from the construction quality control contractor indicating the materials have been placed in accordance with the specifications noted in this report. Any portions of the clay liner left exposed to the elements for more than 60 days will be scarified, recompacted, and retested to ensure they meet the specifications. Copies of the engineering reports will be provided to Adams County and CDH.

The asbestos cells only require certification that a minimum of 1.5 feet of compacted, unspecified soils and 6 inches of topsoil have been placed on the cap. Testing procedures specified for the solidified waste disposal cells are not required to be met when capping the asbestos cell.

8.4 As-Built Diagrams

Each cell will be required to have an as-built diagram completed and submitted to the regulatory agencies within 30-days following completion of construction. The diagram will include:

- * Total cell depth
- * Surface dimensions of the cell
- * Side slope grades
- * Cell base grades both laterally and longitudinally
- * Surface water diversion berms construction and location

9.0 CLOSURE AND POST-CLOSURE REQUIREMENTS

9.1 Closure

Closure at the facility will be completed during filling of the final disposal cell and will conform to all State and Federal regulations. CDH and the Adams County Commissioners will be notified at least sixty days in advance of the final closure date in addition to placing signs at the facility entrance to notify the public of closure. The buildings at the site may be removed or saved for reuse upon approval by the appropriate agencies. The tanks at the site may also be removed for reuse. The mixing basins at the site will be demolished and placed in the last cell prior to closure or they will be decontaminated, demolished, and placed in a suitable demolition landfill off-site. The decontamination will be completed using a non-hazardous cleansing agent and a high pressure wash. Any wash water generated will be properly disposed of.

Foundations that may remain at the site will be removed and placed in the final cell or, if they are not contaminated, removed to a demolition landfill.

The drum storage area components will be excavated and placed in the final cell, if contaminated. Uncontaminated components will be properly disposed of or reused. This will include the sand bed, synthetic liner and overlying soil bed. Any berms surrounding the tank farm will be removed. If there is visible soil staining, the soil will be placed in the final cell. If there is no visible staining, the soils will be graded to match the contours of the site.

The final cell will be capped and revegetated following removal of the materials in the operations area. Water balance calculations are included in Appendix O. It is anticipated that the final site configuration will be very similar to the area's appearance prior to operations. The topography will approximate the same contours and slope at similar grades to the existing site, though the entire site elevation will be raised approximately 4 feet from the initial elevations as noted on Plate 1. Following capping of the final cell, the remaining disturbed portions of the site will be brought to similar grades, revegetated and post-closure maintenance procedures will begin. Plate 11 shows the approximate topography of the closed site. The soils excavated from the cells will be spread over the unused portions of the site and feathered to meet the existing

topography on the western site boundary. The fill will result in grades being reduced from the steepest at approximately 6 percent to 3 percent.

The total amount of soil that will be available after cell construction is expected to be minimal. Any additional soil will be sold or removed off-site during the life of the facility, or placed in the area designated on Plate 1. If additional soil for clay construction is needed, it may be imported from off site from the adjacent property to the south or other property in the area. Prior to such importation, the appropriate agencies shall be consulted on what approvals are needed for such importation. Final grading plans shown on the grading plan may be modified depending on the availability of soils at closure and potential post-closure uses. A final grading plan will be submitted upon closure for review and approval of appropriate agencies.

Soils will be spread only after harvest has been completed so interference with farming activities will be minimized. The topsoil will be stripped and stockpiled prior to spreading the excavated soil. After spreading is complete, the topsoil will be replaced and the farming activities will continue. It is possible that the additional soil will require a change from the normal soil preparation. Soils will be tested by an agricultural lab to determine the proper types and amounts of fertilizer to be used for farming and revegetation prior to seeding.

9.2 Reclamation and Revegetation

All disturbed areas of the site will be reclaimed in accordance with this closure plan. The cells will be capped and reclaimed following completion of filling. In this way, the site will remain as close as possible to its original state. The reclamation process includes the procedures specified in this section for topsoil storage, fertilization, seed bed preparation, and seeding. CSI reserves the right to use the land for any uses that may be approved by the appropriate agencies.

During the excavation of the cells, a minimum of 6 inches of topsoil will be removed and stockpiled separately from the other excavated materials. The stockpile location will depend on the cell to be excavated, but it will generally be on undisturbed ground in the vicinity of the excavated cell. It will be placed as not to disturb the CSI or the farming operations. The topsoil will be stockpiled for varying lengths of time per cell. Cells

that are expected to remain open for periods exceeding six months will require their topsoil stockpiles to be revegetated with native grasses. This will minimize the potential for erosion.

Interim reclamation will be employed until all portions of the property have been filled; final reclamation will be undertaken once final grades are achieved for an area. The initial 40 acres contains the facility area, the asbestos cell, and four waste cells; the balance of the site, except for topsoil and overburden stockpiles and excess overburden deposition area on western portions of the property, will remain in agricultural production. As other areas are excavated for cell use and taken out of agricultural production, previously filled areas will receive interim revegetation or final revegetation depending upon whether final grades have been achieved. Generally, approximately 40 acres of property will be disturbed at any one time and will not be in crop production, lying fallow, and will not be revegetated with either interim or final revegetation. The balance will be in topsoil and overburden stockpiles, excess overburden deposition area and previously filled areas which have received interim or final revegetation, and in unexcavated and unfilled areas still being farmed.

As areas are excavated, topsoil and overburden will be stored in piles and receive interim revegetation if they will be left undisturbed for more than 6 months. Overburden may also be exported offsite in trucks that bring in waste, or sold off for projects that need fill material. Since the exact final grades will be determined by the amount of overburden available, final grading and revegetation may not occur until the end of the life of the facility on all or portions of the property. Both interim and final revegetation will be an ongoing process, and annual evaluations and a final evaluation will be made to determine if areas need to be revegetated during the site's operation and prior to final closure.

Interim and final reclamation will proceed according to the standards outlined below.

* Interim revegetation will consist of seeding as follows:

<u>Grass</u> <u>% of mix PLS/Acre Seeding Rate</u>

9.0

Pubescent Wheatgrass 100

All rates above are for drill seeding; rates should be doubled when broadcast seeding.

- * All topsoil and overburden stockpiles that will be left exposed for 6 months or more, all disturbed areas intended for later grading prior to final grading, overburden deposition areas that aren't at final grade, and all temporary and permanent drainage channel areas shall be seeded as outlined above. For areas larger than 10 acres in size, 1/2 ton per acre of hay, straw, or barnyard manure should be employed as mulch to protect the soil from erosion.
- * For interim revegetation, all areas that were subject to traffic, internal haul routes, or other compaction in excess of natural conditions, shall be scarified to a depth of 4-6 inches prior to seeding. The same scarification should also occur for final revegetation prior to topsoil replacement, except that over fill areas the scarification should only be undertaken after the minimum of 18 inches of overburden has been placed over any filled areas where the clay cap is in place.
- * Upon completion of any area where final revegetation is proposed, topsoil will be replaced over an area at a minimum thickness of 6 inches over a minimum 18 inches of unspecified material, over a minimum 2-foot thickness compacted clay cap. The ground surface beneath the topsoil will be prepared prior to placement of the topsoil, although some consolidation of the surface is to be expected due to the equipment used for revegetation and to natural processes during precipitation.
- * Final revegetation will consist of seeding as follows:

<u>Grass</u>	Rate %	of mix	<u>PLS/acre</u> <u>Seeding Rate</u>
Pastura Blue Grama Vaughn Side Oats Grama Arriba Western Wheat Grass Goshen Prairie Sandreed Prairie Clover	1.5 4.5 8.0 3.5 Trace	20 25 25 30	0.3 10/25 2.0 1.05

All above rates are for drill seeding; rates should be doubled when broadcast seeded. Prior to final revegetation, soil tests will be undertaken to determine fertilizer needs. Barnyard manure may also be employed both for mulch and to replace part of the fertilizer need.

- * If possible, prior to final revegetation, especially in the overburden deposition areas and for all large areas (100+ acres) proposed for revegetation at one time, either milo or sterile sorghum should be planted between May 15 and June 30 as a cover crop at a rate of 10 pounds per acre. Drill seeding of the final revegetation seed should then occur between November 1 and April 1 when the soil is not wet or frozen. If a cover crop has not been employed for areas larger than 10 acres in size, 1/2 ton per acre of hay, straw, or barnyard manure should be employed as mulch to protect the soil from erosion.
- * Weed control to prevent undesirable plants such as Canadian Thistle, Mush Thistle, Knapweed (all varieties), Leafy Spurge and Russian Thistle shall be controlled by cutting and chemical spraying. For interim revegetation areas, mowing should be employed several times during the summer months when weeds reach a height of approximately 18 inches. For final revegetation, an initial application of "Glean" or similar chemical shall be used, then mowing as needed until a satisfactory stand of grass is established and is in the 5-leaf or larger stage of growth.

Upon completion of filling each cell, it will be capped and revegetated. The cap will be constructed according to the specifications included in Section 5.2.3.6 and Appendix Q of this report. The entire cap, with the exception of the topsoil, may be placed immediately upon reaching cell capacity or in phases as filling progresses. The topsoil will be placed immediately prior to fertilizing and seeding.

This revegetation plan is according to guidance from the Soil Conservation Survey. Modifications to these reseeding specifications can be made, if necessary, following the inspection of progress on the initial cell revegetation.

9.3 Post-Closure Monitoring

The ground-water monitoring wells installed at the site will be sampled and analyzed for the parameters specified in Section 7.2 for a period of ten years following site closure. Site post-closure monitoring will take place following completion of the final cap and cover of the last cell. If, at any time during the post-closure period, negative monitoring results are confirmed, the actions proposed in Section 7.4 will be undertaken.

In addition to the ground-water monitoring well sampling, the leachate collection wells in each cell will be monitored on a quarterly basis for a period of 10 years following cell closure. If any leachate is found to exist, CSI will notify CDH and Adams County within 5 working days of discovery and request a meeting with the agencies. The leachate will be analyzed for the same parameters as the ground-water monitor wells to determine a potential source. A plan of action will be presented to the state which may include increasing the frequency of monitoring and a work plan which includes volumes to be pumped and analytical results. The leachate will be pumped and transported to the mixing basin for solidification and disposal with the approval of the county. The county may determine that additional analysis of the leachate is warranted to determine its suitability for onsite disposal. If this determination is made, the operator shall perform such additional analysis prior to solidification. If leachate is continually being generated following 6 months of monitoring, CSI will notify CDH and Adams County and prepare a corrective action plan to remediate the The cells are not expected to produce any leachate following placement of the final cap and a 10-year monitoring period is sufficient to detect any potential problems with the If the cells remain dry during this monitoring period, the leachate collection wells will be abandoned to allow redevelopment of the site. Leachate post-closure monitoring results will be submitted to the appropriate agencies.

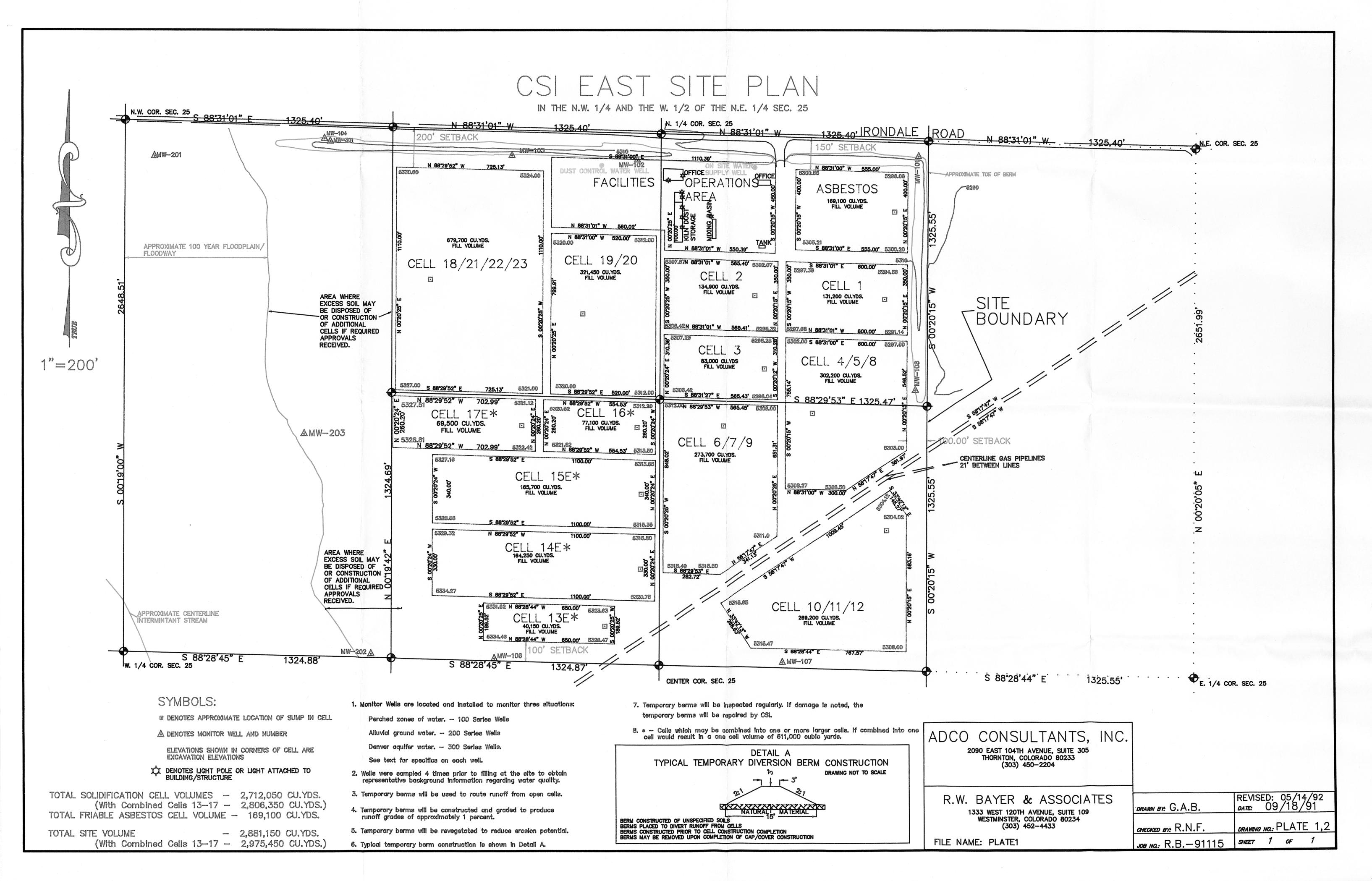
9.4 Post-Closure Inspections and Maintenance

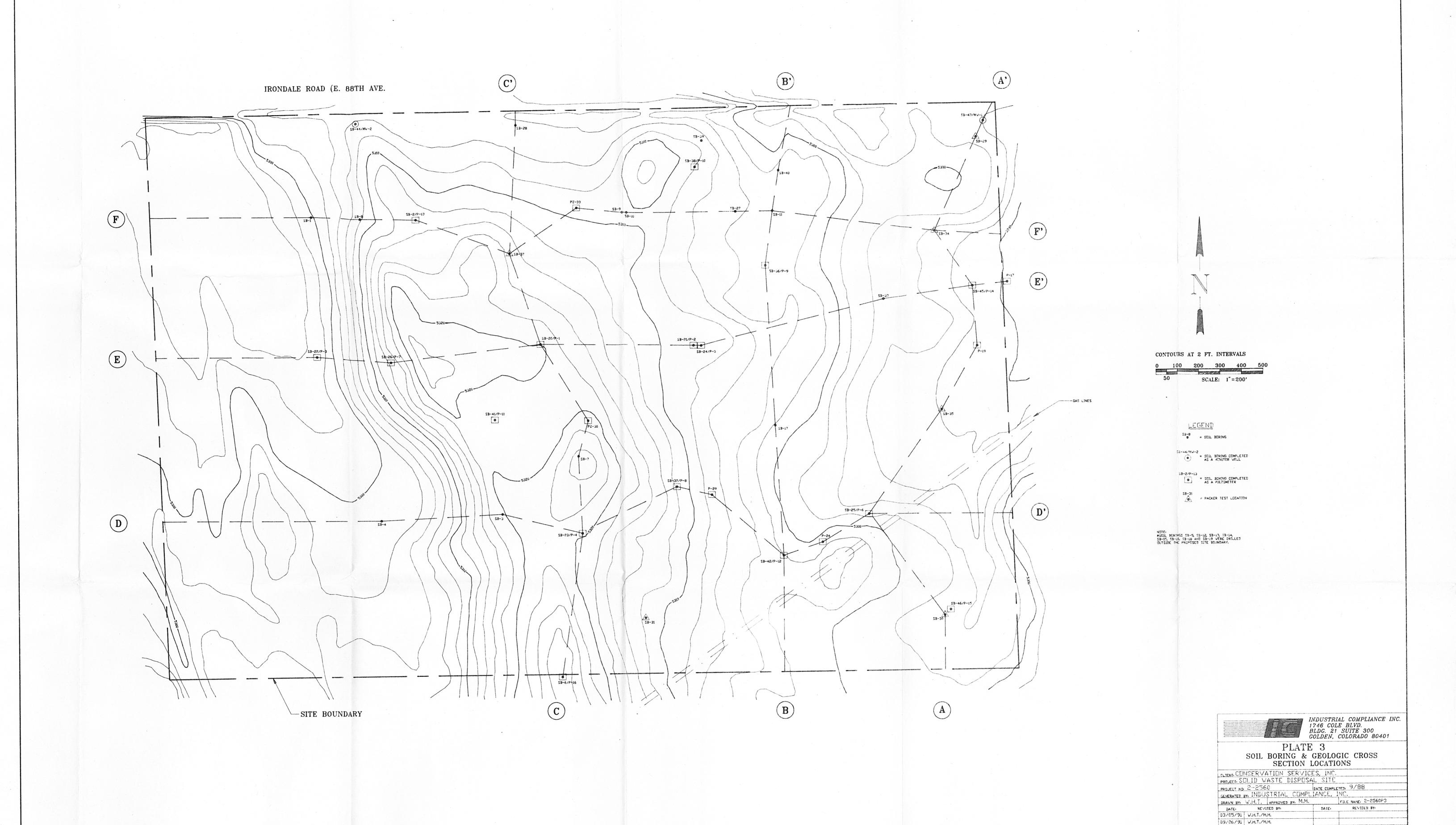
Post-Closure inspections at solid waste landfill sites are typically conducted to detect possible settling of the refuse and cracking occurrences in the clay caps. The solidified materials disposed of in the cells at the CSI site have much less potential for significant settling and therefore settlement inspections will be maintained on an annual basis during the post-closure period.

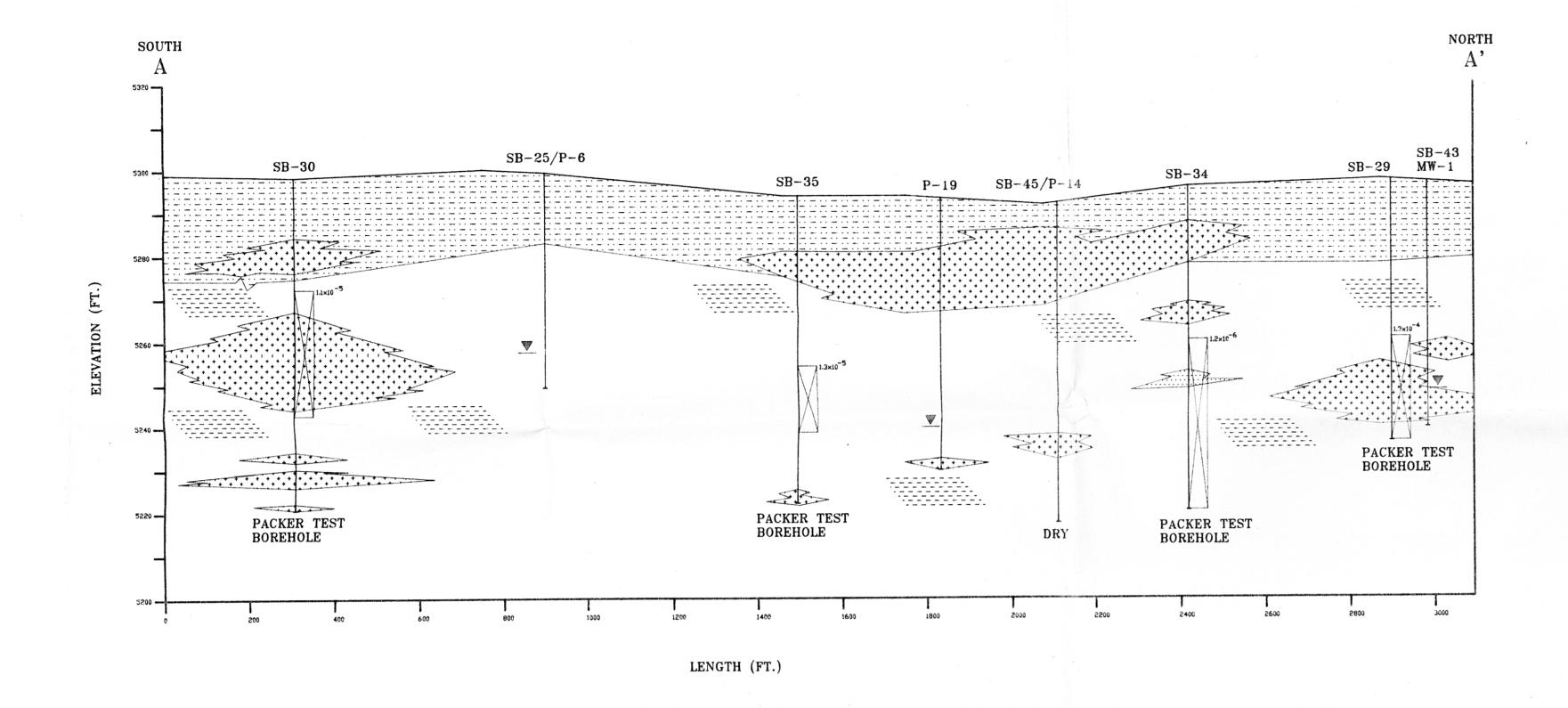
The caps of the cells will be inspected for cracking, erosion and condition of the vegetation on a semi-annual basis with one of the inspections being completed in the late spring when thaw and runoff is most likely to cause damage. The erosion potential for the area is minimal as evidenced by the soil loss calculations completed and included as Appendix P. Any damage detected will be promptly repaired.

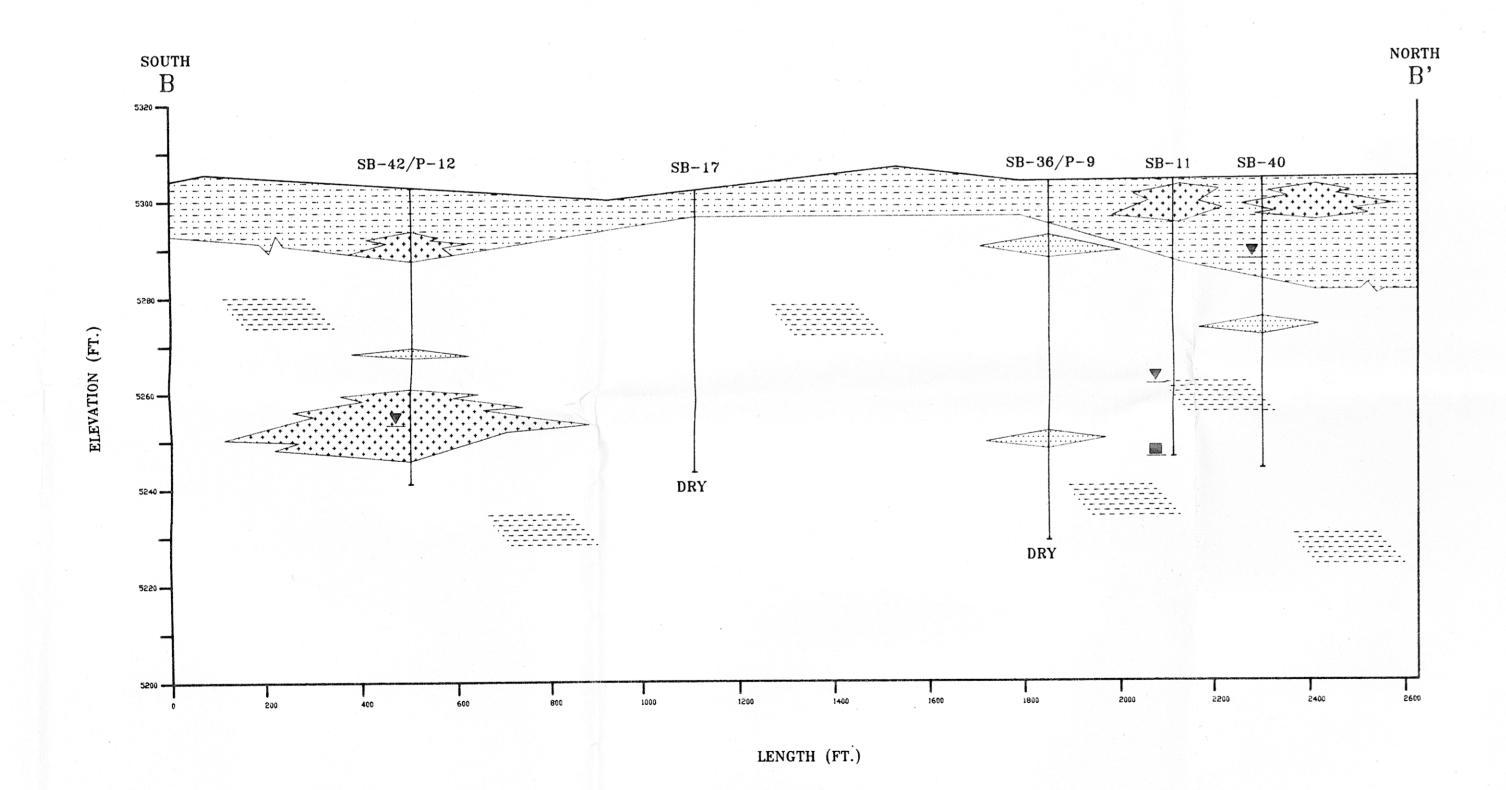
9.5 Projected Post-Closure Land Use

Future land use at the CSI site will be determined with the completion of a revised Adams County Comprehensive Plan. are no specific redevelopment plans existing at this time although the type of disposal that has occurred at the site does not preclude redevelopment, as is sometimes the case at typical sanitary landfills. Geotechnical analyses completed on the materials suggest that the materials are suitable to support typical foundation loads of 1000 to 2000 pounds per square foot. The materials placed in the cells will support loads typically associated with construction of light commercial or industrial buildings. Construction of buildings for open space and recreational uses is also feasible. Methane gas generation, severe slopes, and settling problems are not likely to occur at this site, making it possible to develop the land in a number of ways following the site closure. Adams County, TCHD, CDH, and local governing bodies will be notified of post-closure land use and will be involved in determining whether that use could impact the integrity of the closed facility.









= SILTY SANDY CLAY

= SILTY SANDY CLAY

= SILTY SAND

= SANDSTONE

= GRAVELS

= CLAYSTONE

= CLAYSTONE

= ASSOCIATED PERHEABILITY (Ch/S)

SB = SOIL BORING

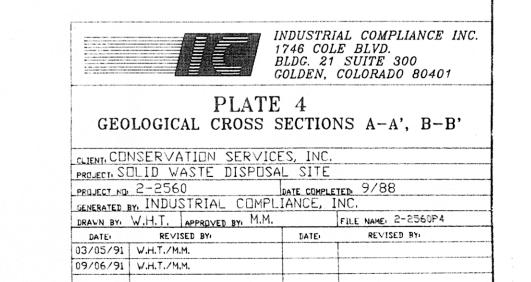
P = PIEZDAMETER

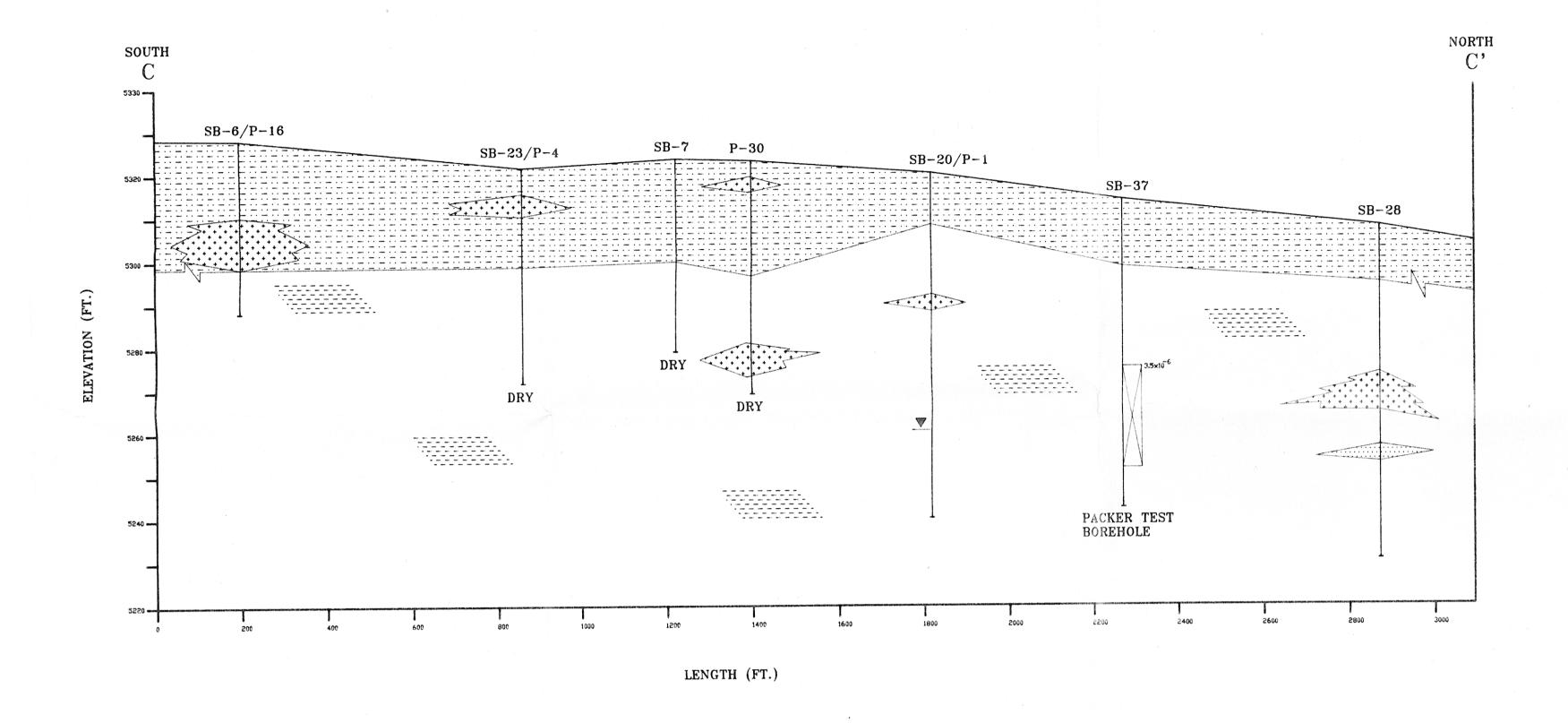
MW = MONITOR VELL

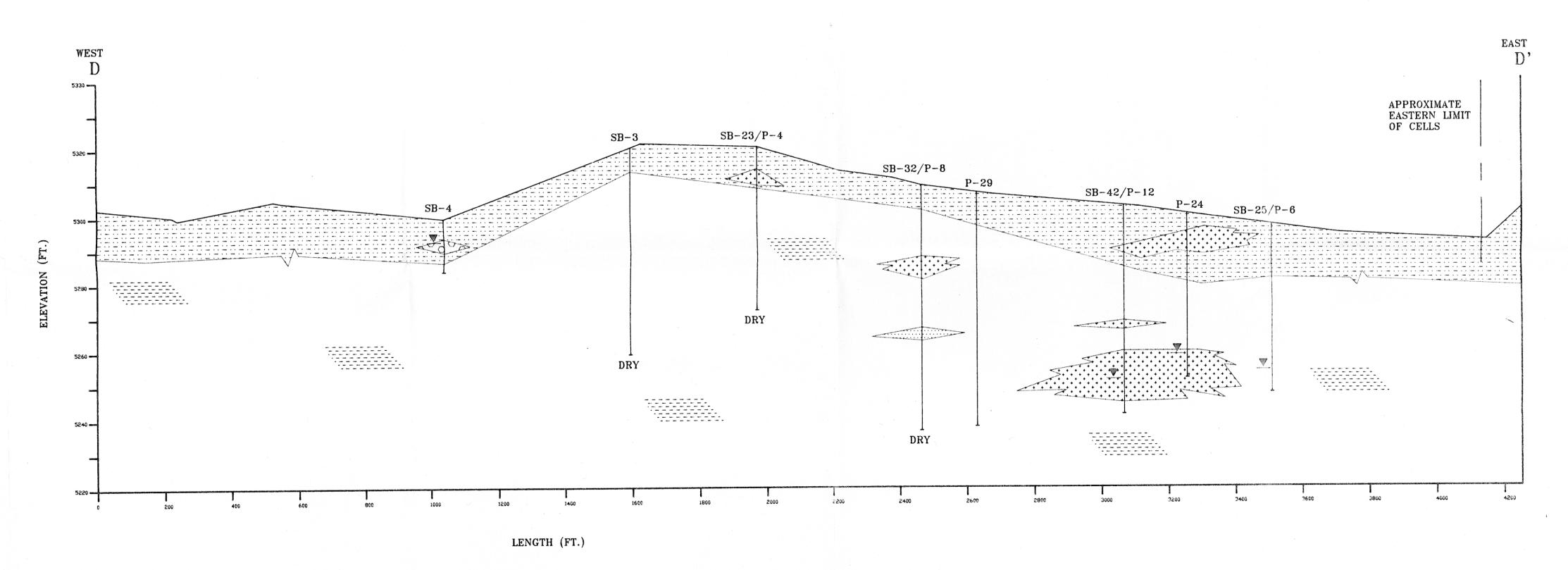
= INFERRED GEOLOGIC CONTACT

NOTE:

#SOIL BORINGS SB-29, SB-30, SB-34, AND SB-35
VERE USED FOR PACKER TESTING, THEREFORE, VATER LEVEL INFORMATION AFTER DRILLING







= SILTY SANDY CLAY

= SILTY SAND

= SANDSTONE

= CLAYSTONE

= STABILIZED VATER LEVEL

SB = SOIL BORING

P = PIEZOMMETER

MW = MONITOR VELL

= INFERRED GEDLUGIC CONTACT

NOTE: #SOIL BURINGS SB-29, SB-30, SB-34, AND SB-35 WERE USED FOR PACKER TESTING, THEREFORE, WATER LEVEL INFORMATION AFTER DRILLING WAS NOT USED.

INDUSTRIAL COMPLIANCE INC. 1746 COLE BLVD. BLDG. 21 SUITE 300 GOLDEN, COLORADO 80401

PLATE 5
GEOLOGICAL CROSS SECTIONS C-C', D-D'

CLIENT, CONSERVATION SERVICES, INC.

PROJECT, SOLID WASTE DISPOSAL SITE

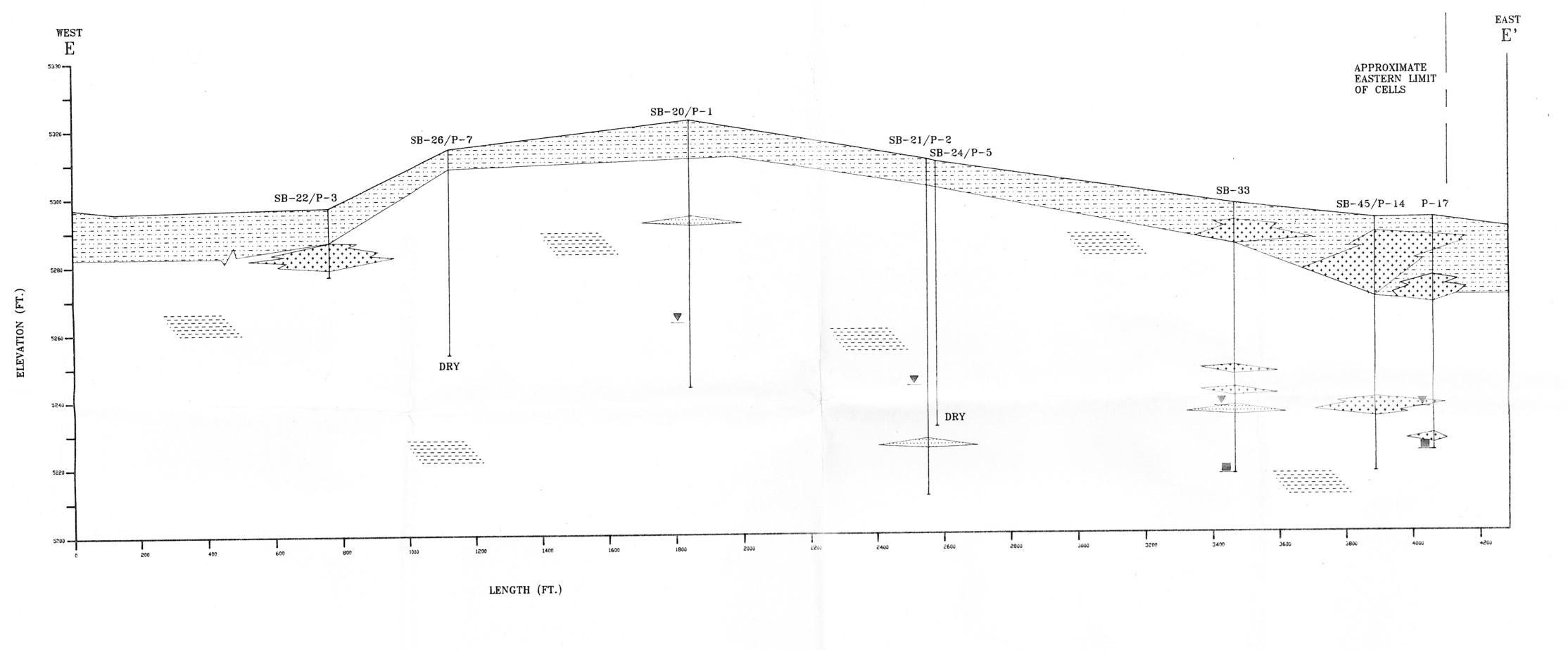
PROJECT NO. 2-2560

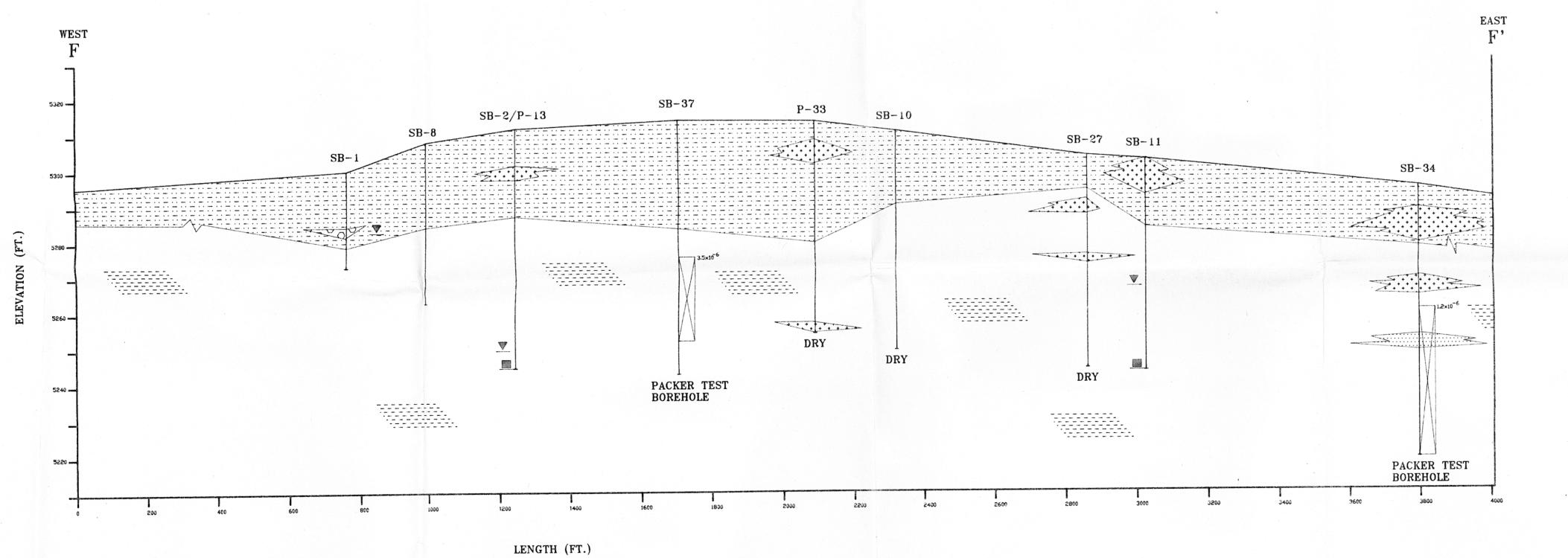
GENERATED BY, INDUSTRIAL COMPLIANCE, INC.

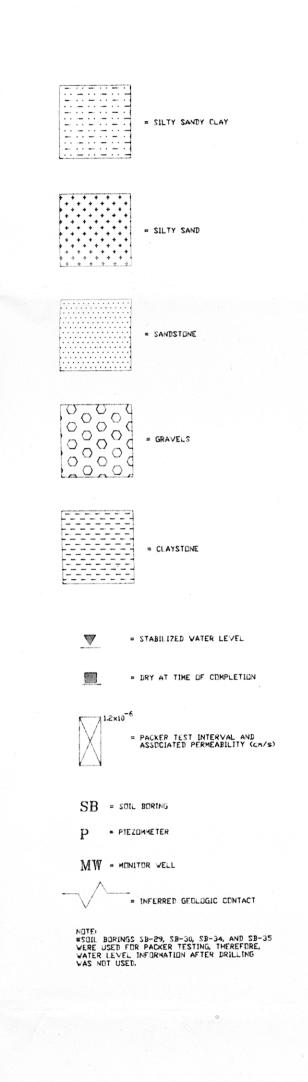
DRAVN BY, W.H.T. APPROVED BY, M.M. FILE NAME: 2-2560P5

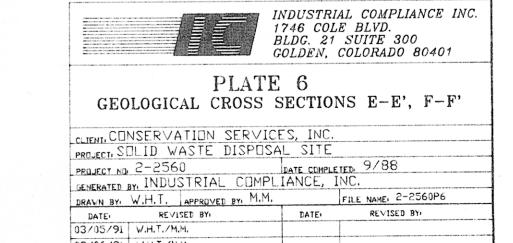
DATE: REVISED BY: DATE: REVISED BY:

03/05/91 W.H.T./M.M.







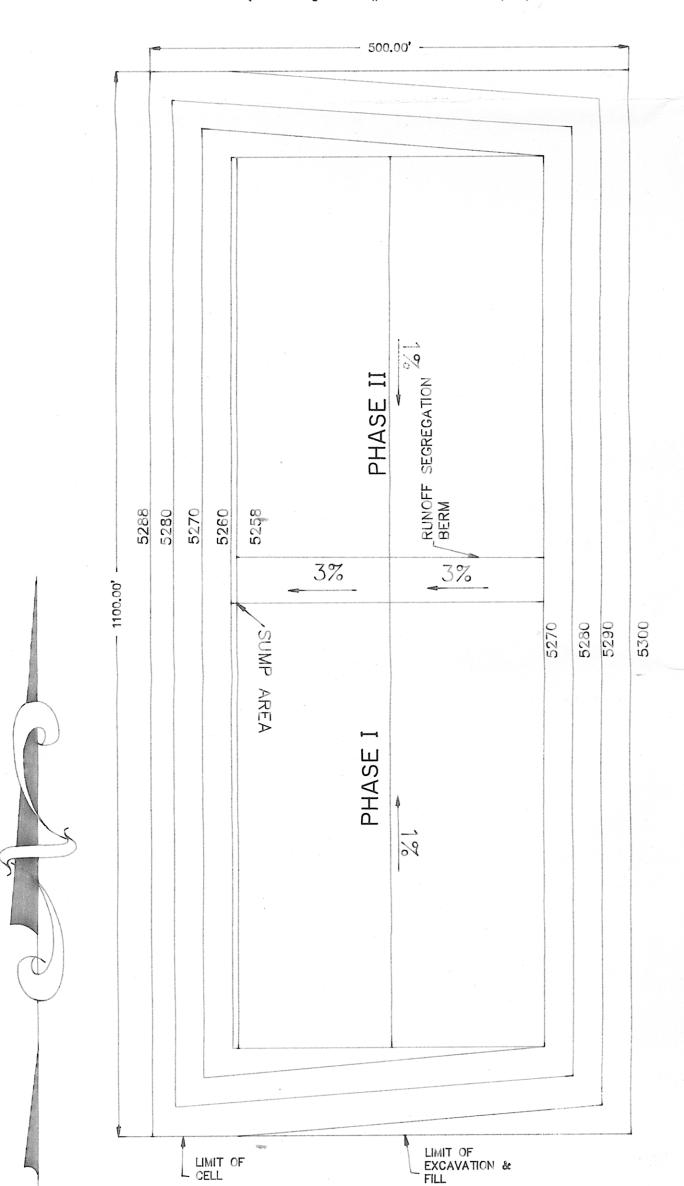


09/06/91 V.H.T./M.M.

REVISED BY

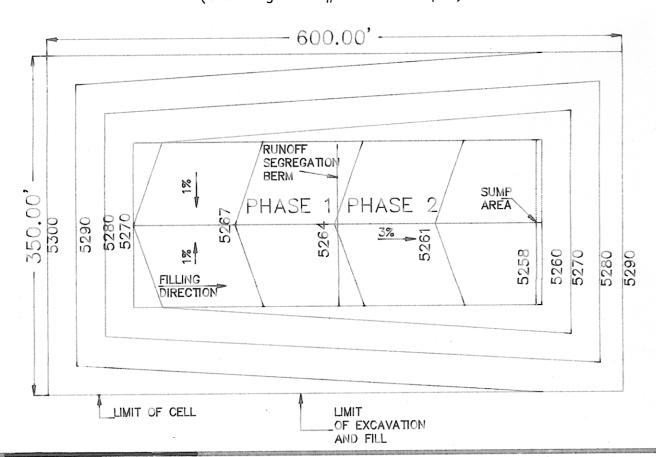
CSI EAST SOLIDIFIED WASTE DISPOSAL CELL DESIGN

TYPICAL FILLING PLAN FOR CELL WITH SIDE SUMP (Utilizing Cell # 24 as Example)

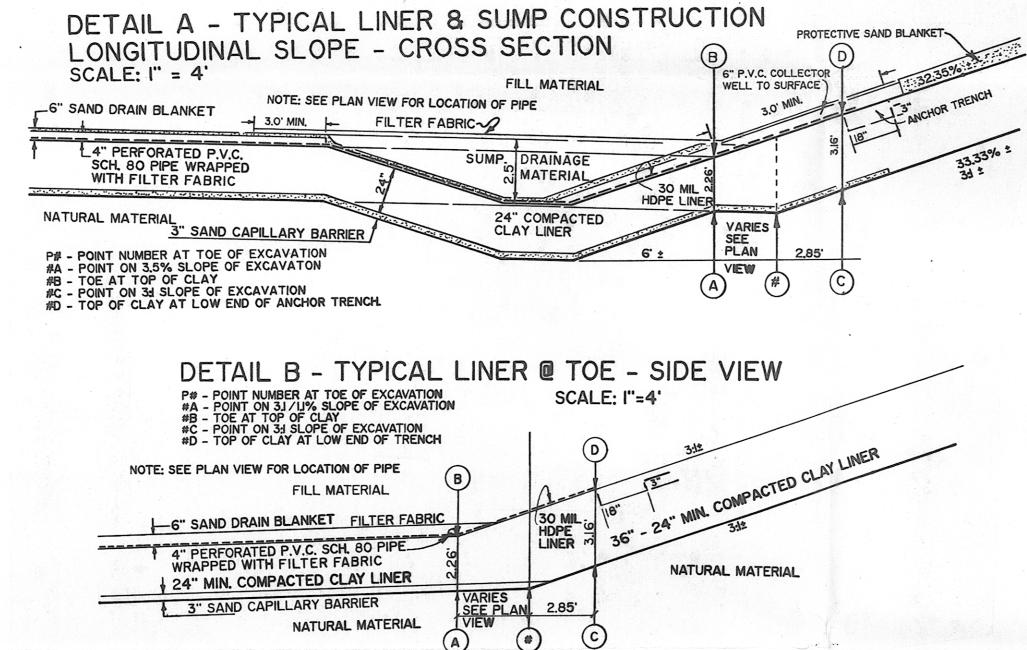


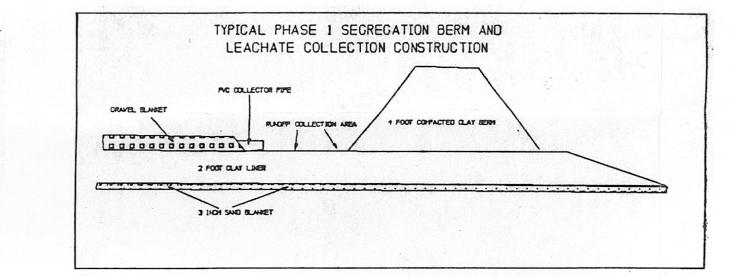
TYPICAL FILLING PLAN FOR CELL WITH END SUMP (Utilizing Cell #1 as Example)

SCALE: 1"=100'



IN THE N. 1/2 OF SECTION 25, T.2S., R.64W.





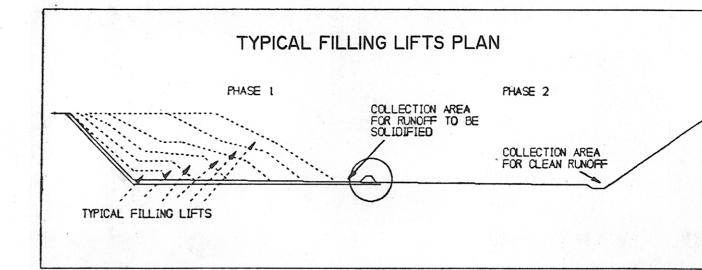


PLATE 7 NOTES:

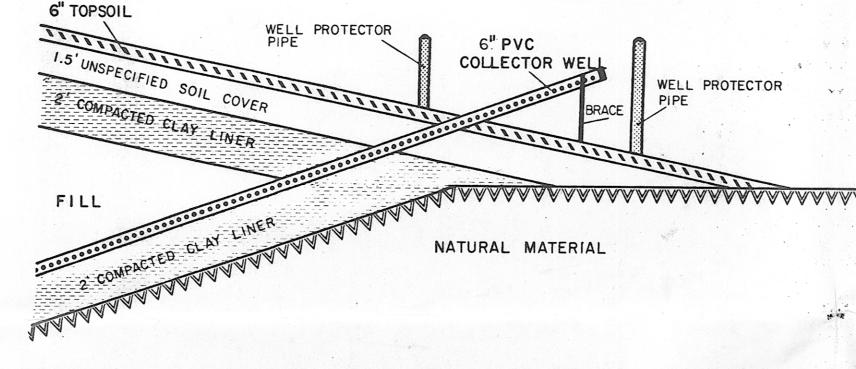
- 1) Cell designs are typical for all the solidified waste disposal cells.
- 2) Cell dimensions and depths will vary according to site conditions.
- 3) All cells will have 3 to 1 side slopes.
- 4) All solidified waste disposal cells will have liner systems including:
- * A minimum of 3 inches of sand as a capillary barrier.
- * 2 feet of compacted clay.
- * sump drainage material over sand blanket in sump area.
- * 6 inches of sand blanket.
- * A PVC leachate collection pipe wrapped
- with a geotextile filter fabric. * A filter fabric overlying the sand
- blanket & sump drainage material. 5) Cell bases will slope laterally to the
- center or side of the cell. 6) A minimum of 18 inches of solified material
- will be placed on the filter fabric prior to driving on it.
- 7) Cells will have waters collected if it has not evaporated within 72 hours. Waters collected will be solidified and disposed
- 8) Filling direction will be from higher elevations to the base of a cell to
- 9) The clay liner will be protected by covering it with either waste materials or unspecified soils within 60 days of its placement. If this is not done the liner
- will be scarrified, recompacted and retested. 10) The cap and cover system will be a minimum
- of 4 feet in thickness.
- 11) Cells will be filled to the elevation of the proposed final grades.

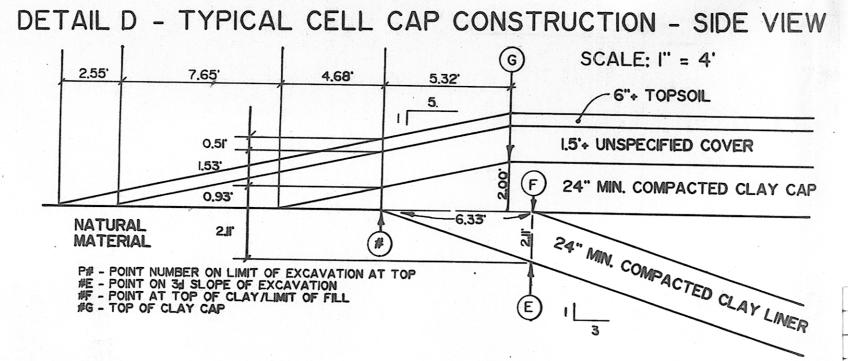
09/20/91

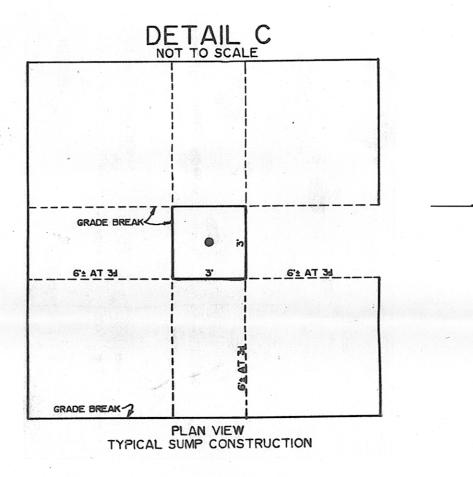
DRAWING NO .: PLATE 7

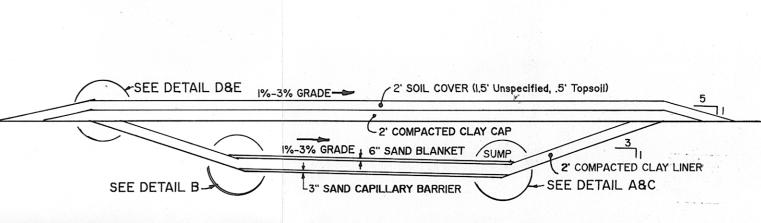
- 12) Elevations on the plates are used only as a reference for a typical cell design.
- 13) Open cells will be sloped and partially
- temporarily capped to minimize storm water infiltration of waste material.







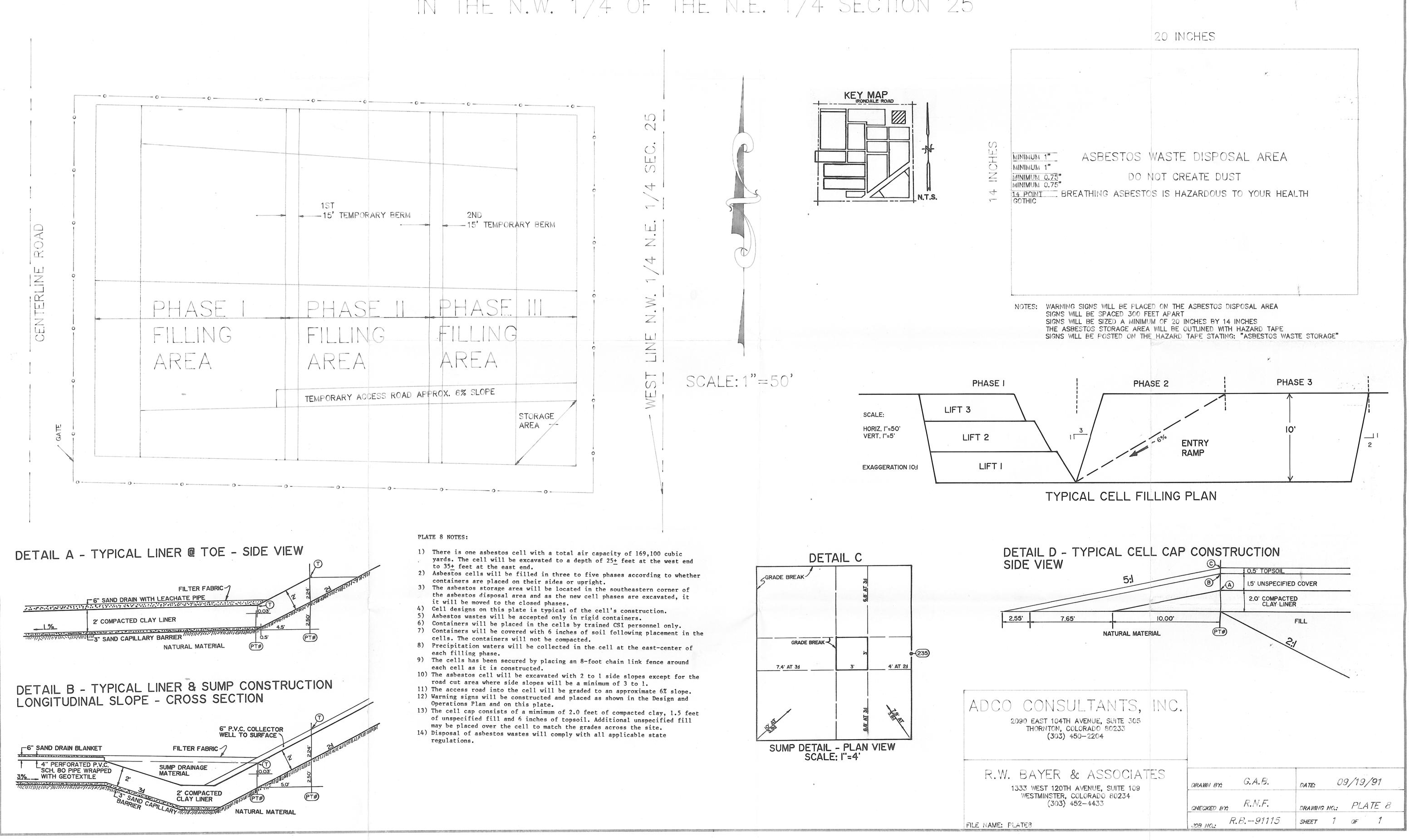


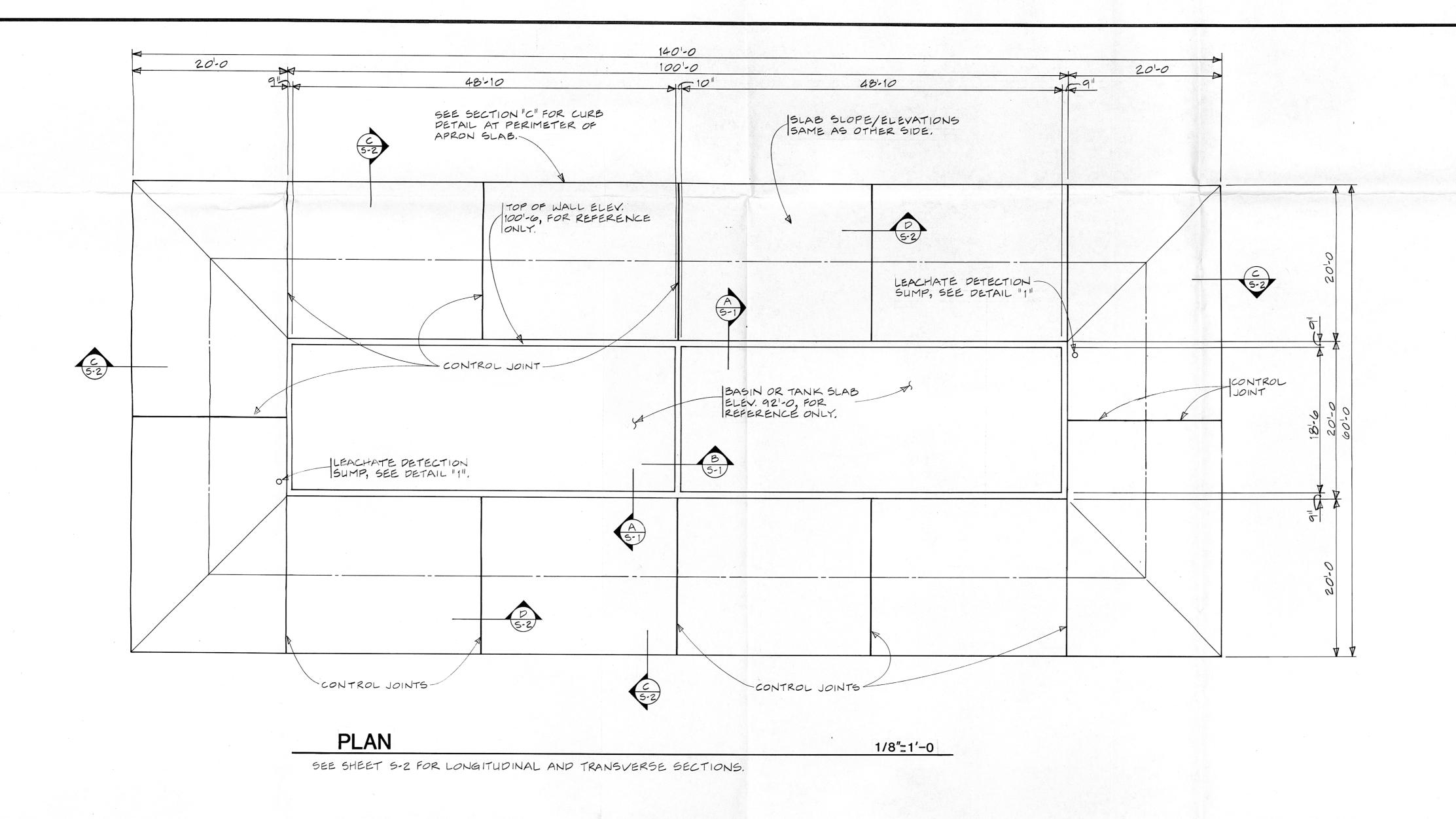


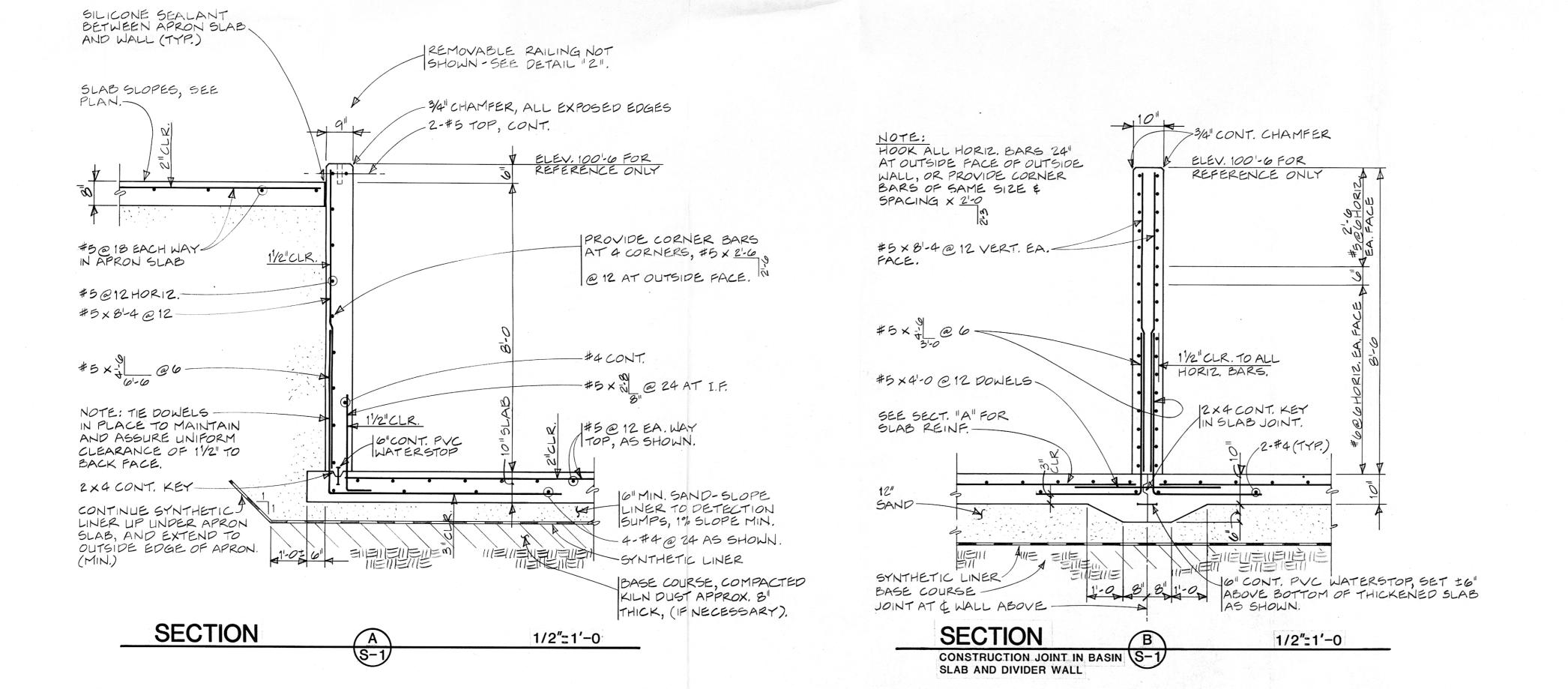
TYPICAL DISPOSAL CELL DESIGN NOT TO SCALE

ADCO CONSULTANTS, INC. 2090 EAST 104TH AVENUE, SUITE 305 THORNTON, COLORADO 80233 (303) 450-2204 R.W. BAYER & ASSOCIATES G.A.B.DRAWN BY: 1333 WEST 120TH AVENUE, SUITE 109 WESTMINSTER, COLORADO 80234 (303) 452-4433 R.N.F. CHECKED BY: RB-,91115 BY FILE NAME: PLATE7 NO. DATE DESCRIPTION

CSI EAST ASBESTOS CELL DESIGN & FILLING PLAN IN THE N.W. 1/4 OF THE N.E. 1/4 SECTION 25







GENERAL NOTES

1. LIVE LOADS USED IN DESIGN: A. EQUIVALENT FLUID PRESSURE ON OUTSIDE FACE OF WALLS ----- 70 PCF B. SURCHARGE (LATERAL LOAD) ----- 100 PSF

- A. ALL CONCRETE SHALL DEVELOP 4000 PSI COMPRESSIVE STRENGTH IN 28 DAYS. B. CONCRETE MIX SHALL CONTAIN A MINIMUM OF 6 SACKS OF CEMENT PER CUBIC YARD, EVEN THOUGH HIGHER STRENGTHS MAY BE ATTAINED WITH USE OF A WATER REDUCING AGENT. USE ASTM C 150 TYPE II CEMENT
- C. ALL CONCRETE SHALL BE MADE WITH 3/4 INCH MAXIMUM SIZE AGGREGATE. D. PLACE CONCRETE WITH A SLUMP OF 4 INCHES, WITH A MAXIMUM VARIATION OF PLUS OR MINUS 1 INCH. ALL CONCRETE IS TO BE CONSOLIDATED WITH MECHANICAL VIBRATORS.
- MAXIMUM WATER-CEMENT RATIO TO BE 0.48.
- F. PROVIDE 6% AIR ENTRAINMENT IN CONCRETE, PLUS OR MINUS 2% BY VOLUME. G. CONCRETE SHALL NOT BE PLACED IF MINIMUM AIR TEMPERATURE IS EXPECTED TO FALL BELOW 40 F IN THE 7 DAY PERIOD FOLLOWING PLACEMENT OF CONCRETE. PROVIDE EXTERNAL HEAT AND INSULATION BLANKETS TO PROTECT CONCRETE IF TEMPERATURES DROP BELOW 40°F.
- H. CALCIUM CHLORIDE ADDITIVE IN CONCRETE WILL NOT BE ALLOWED EXCEPT BY WRITTEN PERMISSION OF ENGINEER.
- I. THE OWNER WILL ENGAGE A CONCRETE TESTING COMPANY TO OBTAIN AND TEST CONCRETE SPECIMENS. THE CONTRACTOR MUST COORDINATE HIS SCHEDULES SO THE SPECIMENS MAY BE TAKEN DURING CONCRETE PLACEMENTS. FOUR (4) SPECIMENS OR CYLINDERS SHALL BE TAKEN FOR EVERY 30 CUBIC YARD POUR, AND FOR EACH AND EVERY POUR MADE. BREAK ONE CYLINDER AT 7 DAYS, AND TWO AT 28 DAYS. HOLD THE FOURTH CYLINDER IN CASE THERE IS A
- 2 DAYS AFTER BREAKS. J. DO NOT BACKFILL AGAINST WALLS UNTIL CONCRETE IN FIELD (NOT LABORATORY SPECIMENS) EXCEEDS 3000 PSI COMPRESSIVE STRENGTH, OR 7 DAYS, WHICHEVER

QUESTION ON 28 DAY STRENGTH. REPORT TEST RESULTS TO ENGINEER WITHIN

- K. CONCRETE JOINTS IN A HORIZONTAL PLANE WILL NOT BE ALLOWED, EXCEPT AS DETAILED. ANY STOP IN CONCRETE WORK MUST BE MADE WITH VERTICAL BULK-HEADS. ALL CONSTRUCTION JOINTS SHALL BE AS DETAILED OR APPROVED BY THE ENGINEER. BASIN WALLS MUST BE CAST IN ONE POUR. BASIN SLAB TO BE CAST
- L. APPLY WHITE PIGMENTED LIQUID CURING COMPOUND (CONFORMING TO ASTM C309) TO SLABS WITHIN 30 MINUTES OF FINISHING. APPLY SECOND COAT OF COMPOUND WITHIN 30 MINUTES OF FIRST COAT, APPLIED AT 90° TO THE FIRST. COVER
- SLABS WITH 6 MIL POLYETHYLENE FILM FOR 48 HOURS AFTER POURING. M. FORMWORK WILL CONFORM TO SHAPES, LINES, AND DIMENSIONS OF MEMBERS SHOWN ON THE DRAWING AND BE SUFFICIENTLY TIGHT TO PREVENT LEAKAGE OF MORTAR. COAT ALL FORMWORK PRIOR TO PLACING REINFORCEMENT. FORMWORK WILL NOT EMPLOY STAKES DRIVEN INTO THE SAND DRAIN MATERIAL, TO PREVENT PENETRATING SYNTHETIC LINER.
- N. COVER TOP OF WALLS WITH BURLAP, KEPT WET FOR 4 DAYS AFTER POURING. FORMWORK CAN BE REMOVED NO LESS THAN 4 DAYS AFTER POURING.

- A. ALL REINFORCING SHALL CONFORM TO ASTM 615(SI), GRADE 60. B. NO SPLICES OR REINFORCEMENT SHALL BE MADE EXCEPT AS DETAILED OR AUTHORIZED BY THE ENGINEER. LAP SPLICES, WHERE PERMITTED, SHALL BE A
- MINIMUM OF 36 BAR DIAMETERS. C. STAGGER SPLICES A MINIMUM OF 4'-0" FOR ADJACENT HORIZONTAL CONTINUOUS
- DETAIL BARS IN ACCORDANCE WITH A.C.I. DETAILING MANUAL AND A.C.I. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, LATEST EDITIONS.
- PROVIDE ALL ACCESSORIES NECESSARY TO SUPPORT REINFORCING AT POSITIONS SHOWN ON THE DRAWINGS. SHOP DRAWINGS FOR REINFORCING MUST BE REVIEWED BY THE ENGINEER PRIOR TO
- THE ENGINEER MUST OBSERVE THE PLACEMENT OF REINFORCING PRIOR TO PLACING CONCRETE. CALL A MINIMUM OF 24 HOURS BEFORE PLACING CONCRETE, AND REVERIFY IF REQUIRED.

FIGURE 6A

3

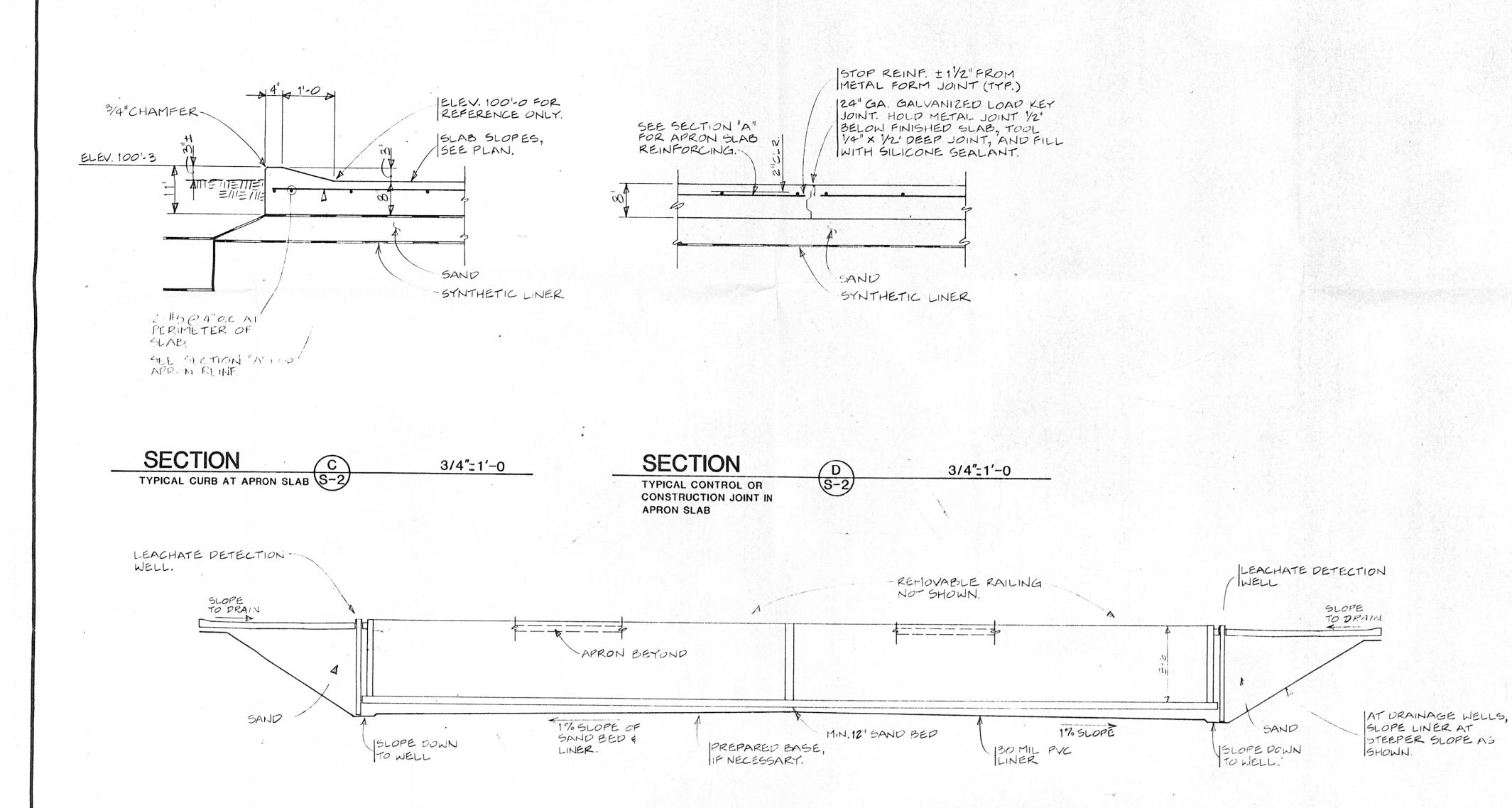
PLAN AND SECTIONS PLATE 9

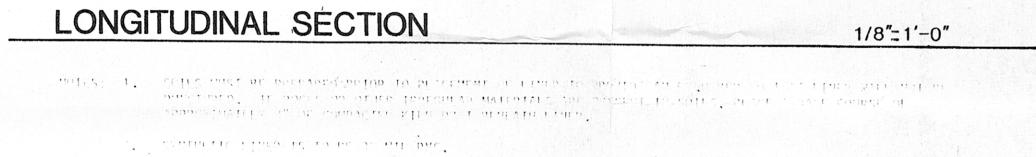
MIXING BASIN

CONSERVATION SERVICES INCORPORATED

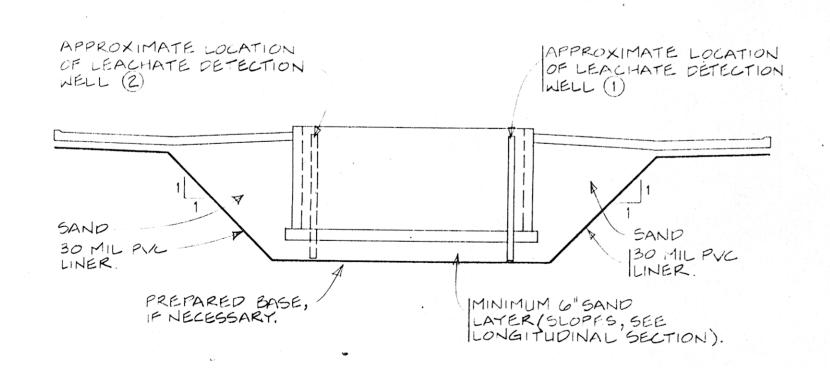
	INCORPORATED
DATE JULY 23, 1987	MICKEY AND SCHNEIDER, INC. CONSULTING ENGINEERS
BY FRM/LLK	
CHECKED FRM	7500 W. MISSISSIPPI AVE. SUITE 236

SUITE 236 (303) 922-1118 LAKEWOOD, COLORADO 80226





1. I THE THE CAME IN TO BE THEN FOR DRAWN IN THATCHES, THE HERE OF SAME PRIMARY BASH STAR. AT I THOSE WALL TO BE RE CHOSE TOWNER ! APPAIR WILLS TO I DEPORTE STORE AS SHOWING



1. The confement will not at all page of Thee of Charles

TRANSVERSE SECTION 1/8":1'-0"

ISEE SECTION "A" FOR WALL AND OTHER DETAILS NOT REPEATED HERE. 6" PERFORATED SCHEDULE 40 PVC PIPE. (SLOTTED PINAL 2.0 FEET) 1-0-EL. 92'-0 FOR REF. ONLY <−1% SLOPE -SYNTHETIC LINER DETAIL 3/8"=1'-0 LECHATE DETECTION SUMP 2" \$ STD. PIPE POSTS. AT 5'-0 O.C. SET INSERTS FROM SHOP PRAWINGS OF RAILING SYSTEM. FABRICATE REMOVABLE RAILING FROM STD. PIPE -11/2" \$ STD. PIPE RAILINGS AND GALVANIZE, MAX. 201 REMOVABLE SECTIONS. 12/2" \$ × 6" GALV. PIPE -INSER'S AT 5'-0 O.C.;
AT EACH RAILING POST.
PROVIDE DOUBLE INSERTS
WHERE RAILING SECTIONS ABUT ABOUT EVERY 20'-0. SILICONE SEALANT WELD 1/4" X 4" SQUARE PLATE TO BOTTOM OF PIPE SLEEVE; GALV. AFTER WELDING. 1/2" JOINT MATERIAL DETAIL 1"=1'-0 REMOVABLE PIPE RAILING

BUMPER POST

FIGURE 6B

RAILING NOT SHOWN.

1ADD 2-#4 BARS X 0

AROUND SLAB OPENING.

PLATE 10 SECTIONS AND DETAILS

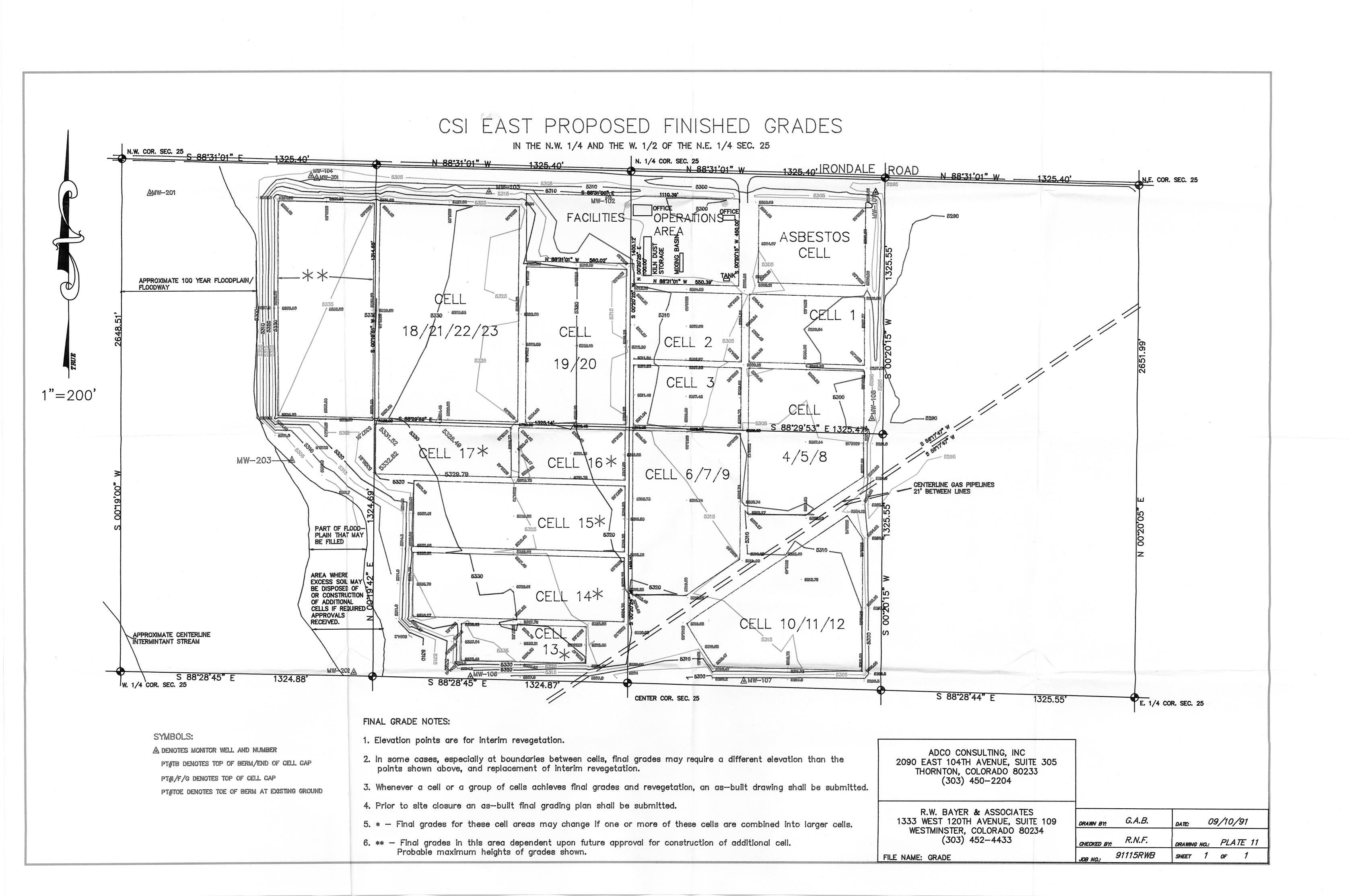
MIXING BASIN

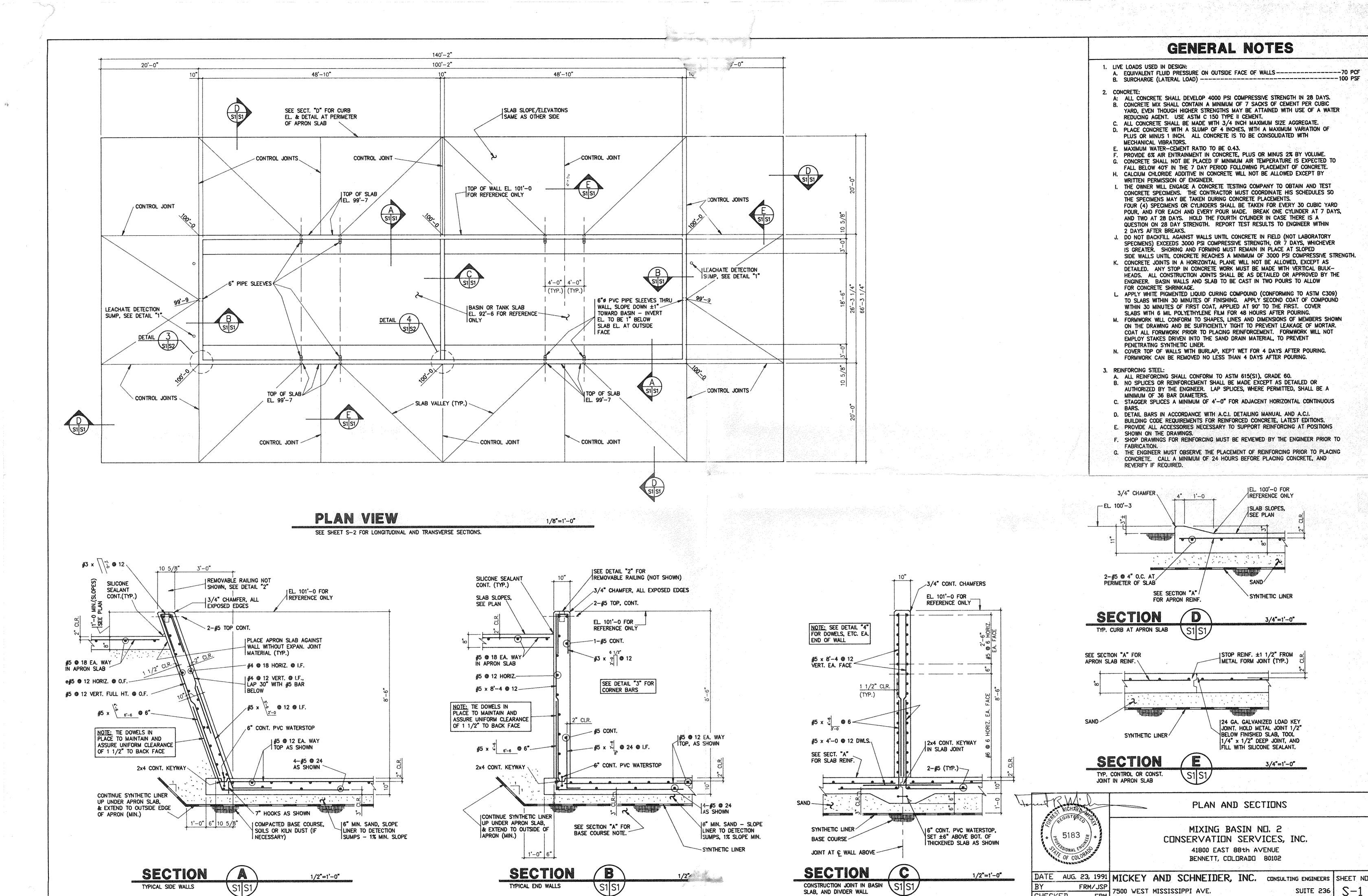
CONSERVATION SERVICES INCORPORATED

DATE JULY 23,1987 MICKEY AND SCHNEIDER, INC. CONSULTING ENGINEERS FRM/LLK

CHECKED FRIM JOB NO. 87269 7500 W. MISSISSIPPI AVE. SUITE 236 LAKEWOOD, COLORADO 80226

(303) 922-1118





SLAB, AND DIVIDER WALL

CHECKED

JOB NO.

FRM

87269

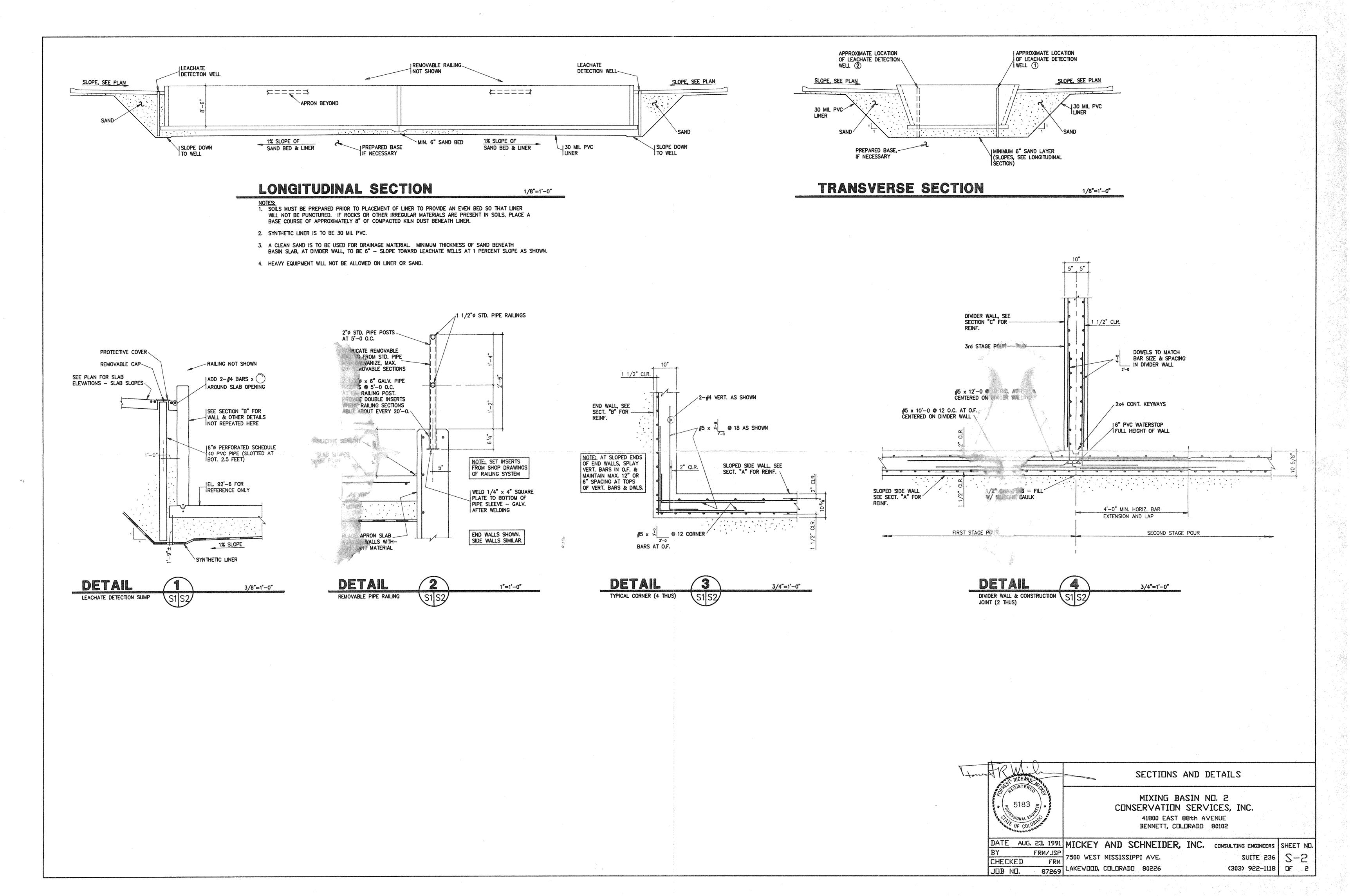
LAKEWOOD, COLORADO 80226

S-1

OF 2

SUITE 236

(303) 922-1118



APPENDIX A - TRAFFIC STUDY

Leigh, Scott & Cleary, Inc.

TRANSPORTATION PLANNING
& TRAFFIC ENGINEERING CONSULTANTS
Offices in Denver and Colorado Springs

1889 York Street
Denver, Colorado 80206
(303) 333-1105

October 11, 1988

Mr. Gerald W. Knudsen, P.E. Industrial Compliance, Inc. 511 Orchard St. Golden, CO 80401

> RE: 88th & Schumaker Disposal Facility (LSC #880740)

Dear Mr. Knudsen:

We have completed a revised traffic analysis of the proposed Conservation Services Non-Hazardous Solid Wastes Disposal Facility which is planned to be located along the south side of Irondale Road (88th Avenue) and west of Schumaker Road in Adams County, Colorado. The following comments summarize our findings.

Location and Project Description

The location of the proposed Conservation Services Facility is shown in Figure 1. The only direct access to and from the site is planned along Irondale Road. In the vicinity of the site, Irondale Road is a two-lane paved roadway approximately 24 feet wide. The remaining roads in the vicinity are two-lane gravel routes with widths which vary between 20 and 30 feet. Two types of wastes will be delivered to the site--solid and liquid. When the project is fully operational, approximately 20 workers will be employed at the site and future maximum site usage may increase the employee count to 30.

Traffic Generation - Initial Operations

Traffic entering and exiting the site can be divided into five categories; solid materials delivery trucks, liquid waste delivery trucks, asbestos waste delivery trucks, employee trips and equipment/supply delivery trucks. Based upon observed activity at the Conservation Services, Inc. landfill at 777 West 62nd Avenue in Denver, Colorado, and past experience of Conservation Services, Inc., it is conservatively estimated that the site will initially generate a maximum of 5 solid materials delivery trucks, 7 liquid waste delivery trucks and 2 supply trucks per day. In addition, approximately 3 asbestos delivery trucks are anticipated per week. An assumption of about two vehicle-trips per employee per day results in about 70 total vehicle-trips being generated by the site per day (30 heavy truck trips and 40 automobile/ pickup truck trips). This traffic activity will occur only on weekdays between 7:00 A.M. and 5:30 P.M.

Traffic Generation - Future Operations

The estimates cited above reflect total daily delivery of about 160 cubic yards of equivalent solid waste material to the proposed facility. This level of activity is expected to be reached within the first year of operations. In the future, however, the facility could accept a maximum of 385 cubic yards of equivalent solid waste materials per day which would require an additional ten employees (30 total employees). If this activity level is achieved, truck traffic generation would increase proportionately to about 70 vehicle-trips per day and employee traffic would increase to 60 trips per day (130 total daily vehicle-trips).

Distribution

Figure 1 also illustrates the estimated directional distribution of site-generated traffic. Three different distributions are anticipated. Initially, the largest concentration of disposal site vehicles will be along Irondale Road where 70 percent of the traffic is anticipated. With the closure of Irondale in conjunction with the construction of Denver's new regional airport, the distribution will be primarily oriented towards I-70 via Imboden and Manilla Roads. Similarly, the third distribution assumes the closure of Manilla Road south of Irondale in conjunction with the possible expansion of the Front Range Airport.

Traffic Impacts

Current traffic activity along Irondale Road near the site is estimated to be about 500 vehicle-trips per day. It is further estimated that both Imboden and Manilla Roads presently accommodate less than 200 vehicle-trips per day. Traffic generated by the Conservation Services Facility will add as much as 130 trips per day to Irondale and about 104 daily trips to Imboden. These increases can easily be accommodated considering current capacity of these routes—about 5,000 vehicles per day on Irondale and about 1,000 on Imboden. In addition, the amount of facility—generated traffic to be added to the surrounding road system should not create any significant reduction in traffic safety. The affected routes are provided with adequate traffic controls, right—of—way assignment at all affected intersections is controlled by two—way Stop signs, and relatively low speed limits are posted.

* * *

We trust that our findings will assist with further planning for the proposed Conservation Services facility. Please call if we can be of additional assistance.

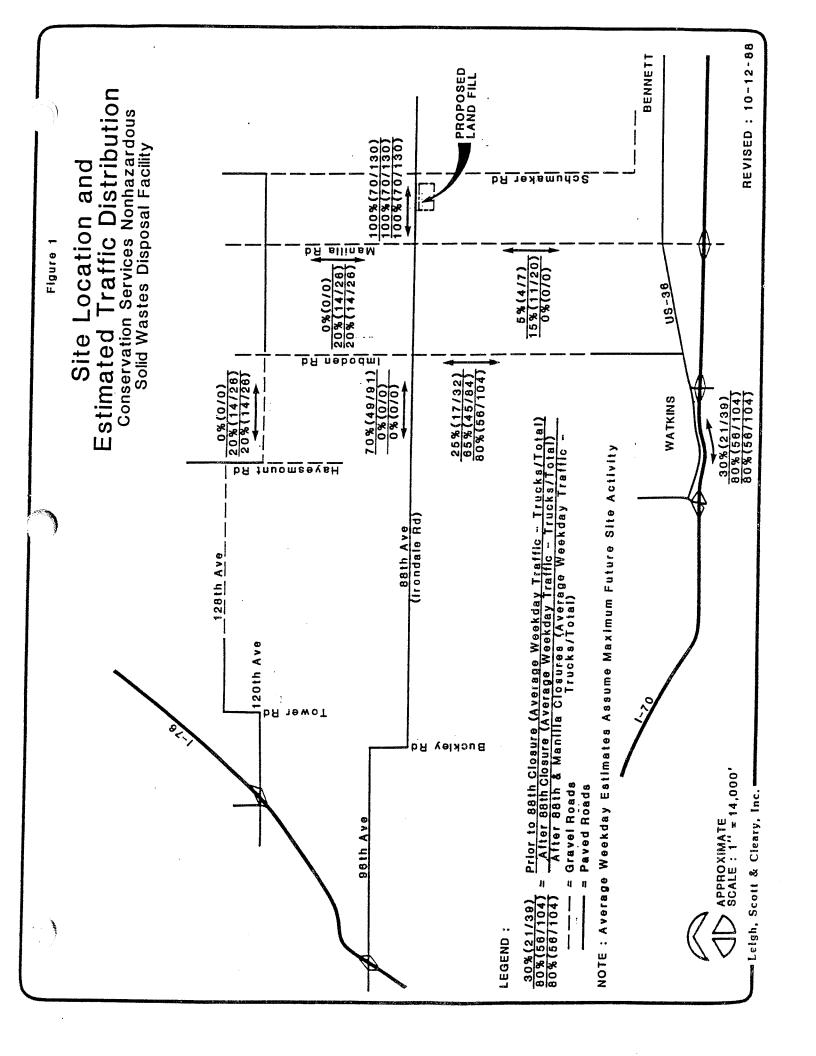
Respectfully submitted,

LEIGH, SCOTT & CLEARY, INC.

Philip N. Scott III. P.E.

PNS/mlc

Enclosure: Figure 1



APPENDIX B - DRAINAGE AND FLOOD PLAIN REPORT

CSI OFFSITE DRAINAGE PRELIMINARY ANALYSIS OCTOBER, 1988

Prepared by:

QCI Development Consultants 11990 Grant St. Suite 310 Northglenn, Co. 80233 450-0917



CSI OFFSITE DRAINAGE PRELIMINARY ANALYSIS

Introduction

Conservation Services, Inc., CSI, has requested a Certificate of Designation for: 1. Offices and Plant for solidification of non-hazardous liquid waste,
2. Landfill of solidified waste, contaminated soil, asbestos and other EPA designated non-hazardous materials.

The size of the site is approximately 240 acres located at approximately 1/4 mile west of the SW corner of 88th Ave. (Irondale Rd.) and Schumacher Rd. (30N). It is more particularly described as the NW 1/4 and the W 1/2, NE 1/4 of Section 25, T2S, R64W of the 6th PM, County of Adams, State of Colorado.

Present zoning on the property is A-3 and the property lies within the "Airport Area" Overlay Zone District of the County. The municipal growth area is projected to be Aurora. Existing land use is agricultural. No specific land use or zoning designations are yet available for future development patterns. As such this Report shall be deemed a Preliminary Analysis, approximating by use of the Colorado Urban Hydrograph Procedure those flow rates impacting the site under present basin characteristics.

The purpose of this Report is to quantify present storm water run off from upstream basins affecting the site. The Colorado Urban Hydrograph Procedure was utilized as the most expedient and appropriate method of calculation for this Preliminary Analysis. Full HEC-II analysis or SWMM analysis will probably someday be conducted by the UD&FCD for FHAD mapping but this level of detailed study is not warranted at this time for this single user or until land use patterns of the area become more defined. We therefore encourage the reader to utilize the data as preliminary for the planning and determination of site facilities with respect to the approximate flow rates shown herein.

Existing Conditions

The site is presently agricultural. It and the surrounding fields appear to be farmed in dry land grasses of varying types. Topography is very gentle from south to north. The property is divided by a ridge running from south to north through its approximate center. The east side of the site is intended to be the operational area. The west side of the site is intended to be an area for disposal and grading of surplus fill material.

Run off from the south enters the east portion at the site's south property line. No defined drainage way or swale exists. The offsite basin consists of approximately 288 acres of dry land farming and is defined as Basin A herein. Basin B of this Report is the east portion of the site itself. Basins A and B sheet flow to approximately the southwest corner of 88th Ave. and Schumacher Rd. No culvert of drainage structure exists.

Both basins A & B are poorly defined in the gentle rolling country of this area. Farming operations generally furrowing perpendicular to the topography inhibit run off and generally retain water. Soil Conservation Service mapping of these basins was examined and it was determined that the general Hydrologic Soil Group was "B". The maps in the Appendix of this Report describe the areas noted as Basins A and B.

The west portion of the site is impacted by Basins C & D as defined herein. Both of these basins are large, encompassing approximately seven square miles each. Their size makes them more defined with more obvious drainage paths at their low points. These are the basins that are likely for study by UD&FCD in the future. They are presently agricultural however, gently sloping to a common confluence at the extreme southwest corner of the site. A Hydrologic Soil Group "C" was arbitrarily chosen for these basins as the most conservative soil group.

Developed Conditions

This Report has assumed a run off coefficient of 0% impervious, for its analysis. This coefficient is intended to represent the present land use for the area. It is applied to all the offsite (Basins A,C,D) and onsite (Basin B) basins. No upstream detention is assumed. In reality, at time of development, detention policies are usually enforced for areas such as this where no defined waterway exists.

Hydrology and Methodology

The UD&FCD "Colorado Urban Hydrograph Procedure Computer Program - PC Version", January, 1985 was used in concert with the UD&FCD Manual for analysis. The following basin and storm parameters were applied:

Description				
-	A	Pasins A&B	С	D
Area (sq. mi.)	0.45	0.77	7.0	7.1
Sub-basin length (mi.)	1.4	2.2	8.3	8.1
Sub-basin centroid distance (mi.)	0.7	1.1	5.2	4.6

Percent Impervious	0	0	Ø	0
Slope (ft/ft)	0.010	0.0066	0.006	0.0068
Pervious detention (inches)	0.35	0.35	0.35	0.35
Impervious detention (inches)	0.1	0.1	0.1	0.1
Hydrologic soil group	В	В	С	. с
Rainfall (100yr1hr.)	2.75	2.75	2.75	2.75
Initial Infiltration Rate (in/hr)	4.5	4.5	3.0	3.0
Decay rate	0.0018	0.0018	0.0018	0.0018
Final Infiltration Rate (in/hr)	0.6	0.6	0.5	0.5

The data produced the following 100 year flow rates that the CSI site must provide for:

Entering the east portion of the site from the south, Basin A: Q = 255cfs

Exiting the east portion of the site at the NE corner, Basin A&B: Q = 295cfs

At the southwest corner of the site, Basins C&D combined at a common time to peak of 225 minutes: Q = 2261cfs 100

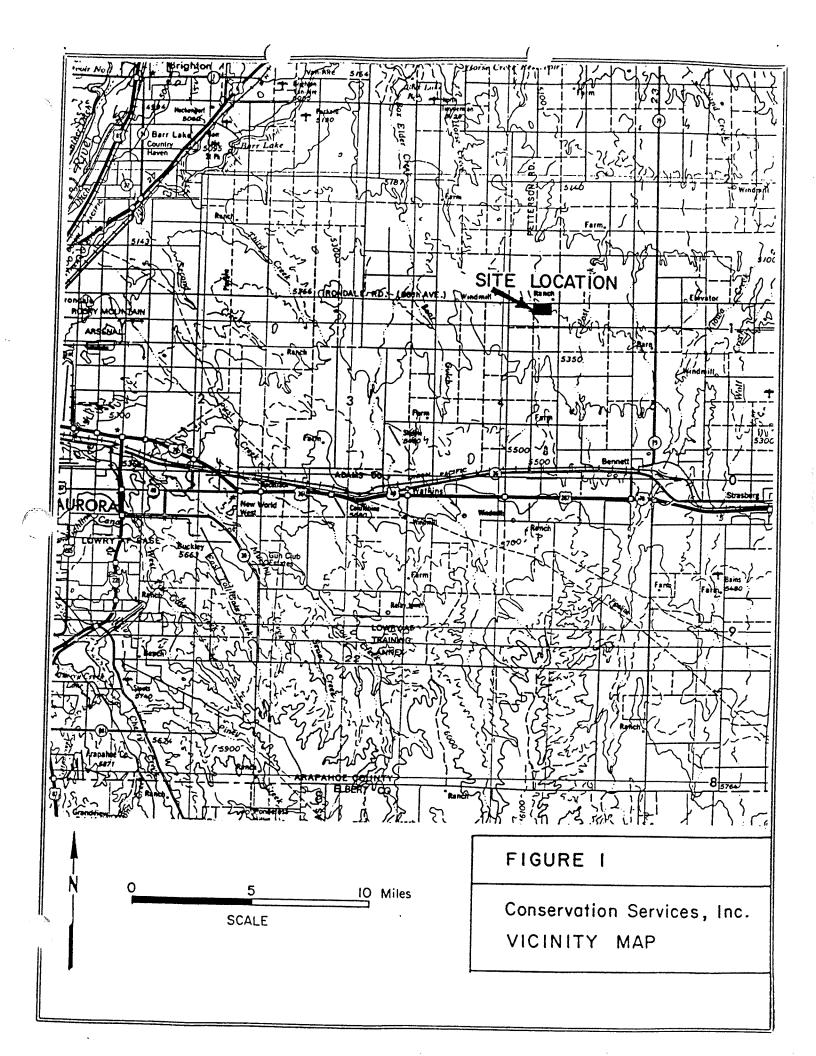
West Floodplain

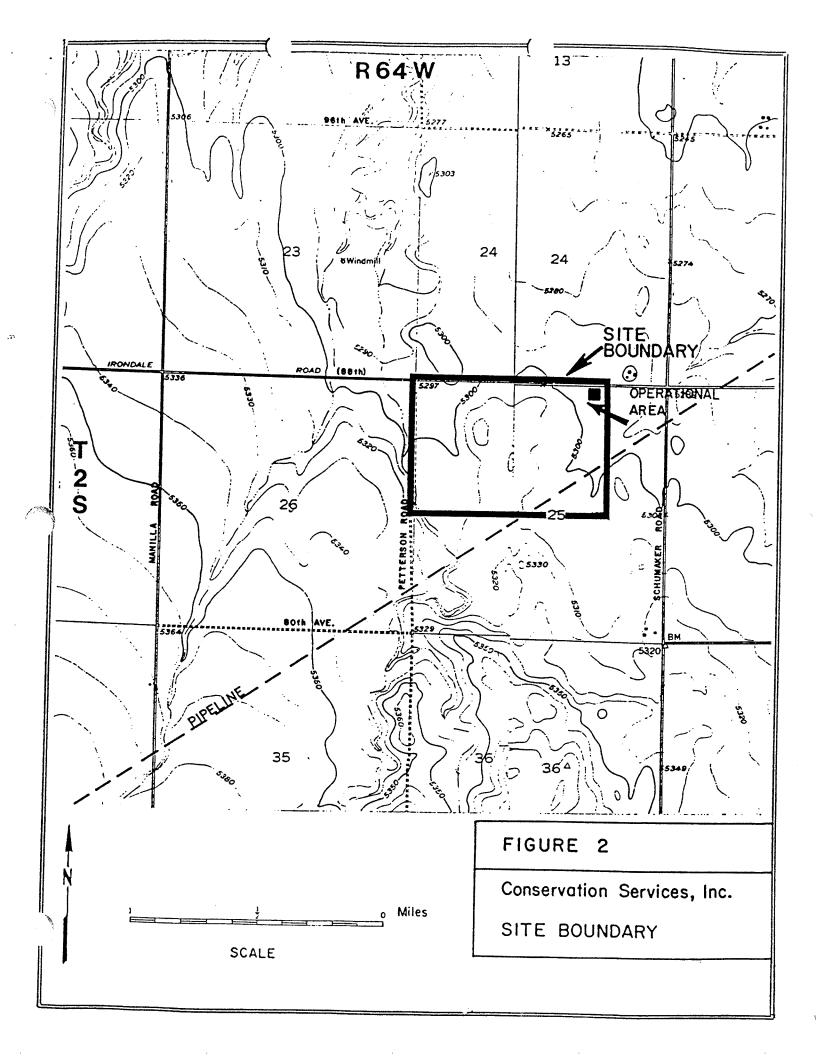
Based on the hydrology developed and the topography available the approximate limits of the 100 year floodplain affecting the west portion of the site has been plotted. The floodplain enters the site at the southwest corner, Section A-A, as a broad relatively shallow floodplain. A flow split ocurrs through Sections B-B, and C-C creating an area of shallow flooding east of the main channel. Sections B-B and C-C have been calculated assuming the full 2261 cfs remains in the main channel. This would necessitate filling the area of shallow flooding and would cause a rise of approximately 0.3' in water surface profile at Section A-A.

Conclusions

The flow rates are based on present developed conditions with no upstream detention. Future development changes to Schumacher Rd. or other land parcels may alter the upstream characteristics of the basin. These alterations will occur over time however and any resulting impact to the CSI site can be analyzed on a case by case basis. Provisions by the CSI site to accommodate the flow rates projected above will afford operational stability until such time as full hydraulic analysis based on projected land use patterns can be created.

APPENDIX B. Drainage C.
Abod Plan





Symbol	Name	Hyd. Soil Group
		
AsC	Ascalon	В
AsD	Ascalon	В
AcC	Adena-Colby	C – B
AsB	Ascalon	В
WmB	Weld	C -
VnD	Vona	В
Αt	Ascalon-Platner	B-C

ADAMS COUNTY, COLORADO - SHFFT NUMBER 55

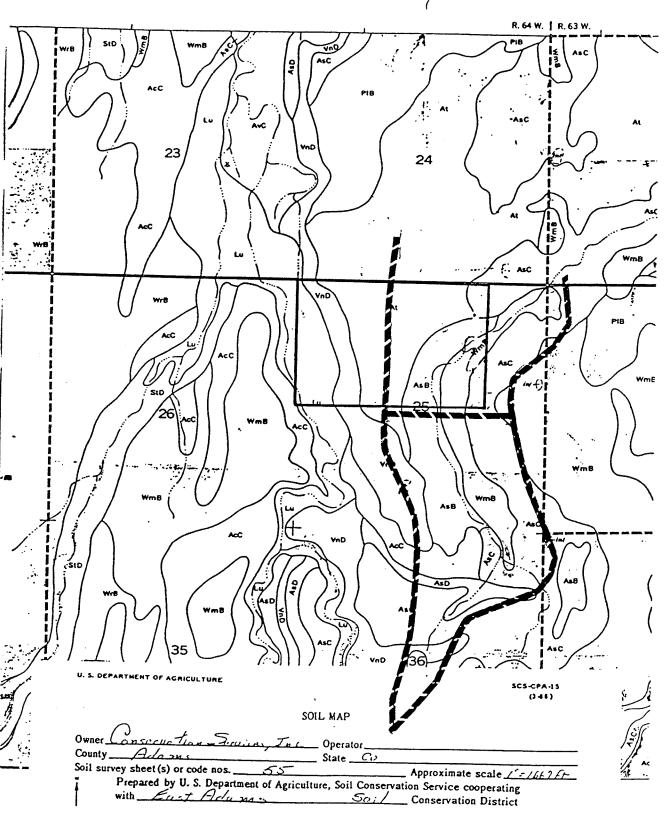


TABLE 3-1 (42)

RECOMMENDED RUNOFF COEFFICIENTS AND PERCENT IMPERVIOUS

LAND USE OR	PERCENT		FREQU	ENCY	
SURFACE CHARACTERISTICS	IMPERVIOUS	2	5	10	100
Business:					
Commercial Areas	95	.87	.87	.88	.89
Neighborhood Areas	70	.60	.65	.70	.80
Residential:					
Single-Family	*	.40	.45	.50	.60
Multi-Unit (detached)	50	.45	.50	.60	.70
Multi-Unit (attached)	70	.60	.65	.70	.80
1/2 Acre Lot or Larger	*	.30	.35	.40	.60
Apartments	70	.65	.70	.70	.80
Industrial:					
Light Areas	80	.71	.72	.76	.82
Heavy Acres	90	.80	.80	.85	.90
Parks, Cemetaries:	7	.10	.10	.35	.60
Playgrounds:	13	.15	.25	.35	.65
Schools:	50	.45	.50	.60	.70
Railroad Yard Areas	40	.40	.45	.50	.60
Undeveloped Areas:					
Historic Flow Analysis-	2	(See	"Lawns")	•	
Greenbelts, Agricultur	ral				
Offsite Flow Analysis (when land use not defin	45 ned)	.43	.47	.55	.65
Streets:					
Paved	100	.87	.88	.90	.93
Gravel	13	.15	.25	.35	.65
Drive and Walks:	96	.87	.87	.88	.89
Roofs:	90	.80	.85	.90	.90
Lawns, Sandy Soil	0	.00	.01	.05	.20
Lawns, Clayey Soil	0 .	.05	.10	.20	.40

NOTE: These Rational Formula coefficients may not be valid for large basins.

^{*}See Figure 2-1 for percent impervious.

TABLE 2-1

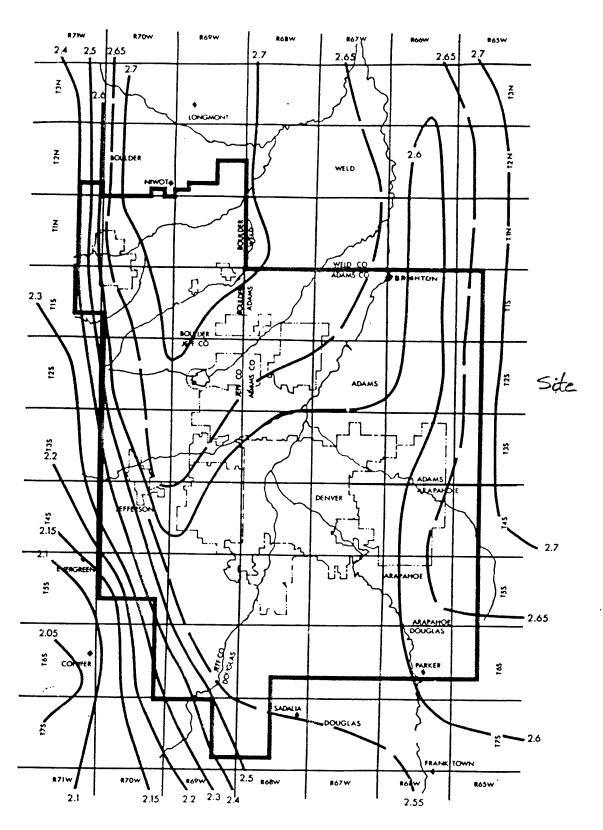
TYPICAL DEPRESSION RETENTION FOR VARIOUS LAND COVERS (all values in inches)

(For use with CUHP Method)

Depression &	
Detention	Recommended
0.05 - 0.15	0.1
	0.1
0.03 - 0.1	0.05
0.2 - 0.5	0.25
·	0.35
- U.U	0.4

TABLE 2-2
RECOMMENDED HORTON'S EQUATION PARAMETERS

SCS Hydrologic	Infiltration (in/hr)	Decay
Soil Group	Initial - fi Final-fo	Coefficient
Α	5.0 1.0	0.0007
В	4.5 0.6	0.0018
C	3.0 0.5	0.0018
D	3.0 0.5	0.0018



URBAN DRAINAGE & FLOOD CONTROL DISTRICT

RAINFALL

DEPTH - DURATION - FREQUENCY

Weighted Slope

Basin-H

$$S = \left(2500(.012)^{.24} + 800(.0375)^{.24} + 4100(.007)^{.24}\right)^{4.17}$$

$$= \left(\frac{865 + 364 + 1246}{7400}\right)^{4.17}$$

Basino A&B

$$S = \left(\frac{865 + 364 + 1246 + 4200(.0026)^{.24}}{11,600}\right)^{4.17}$$

Weighted Slope

Basin C

$$5 = (4900(.004)^{.24} + 3800(.005)^{.24} + 12,000(.0075)^{.24} + 23,000(.006)^{.24})$$

= 0.006 St/St

Basin D

$$5 = (4900(.004)^{.24} + 13,000(.005)^{.24} + 5500(.005)^{.24} + 19500(.010)^{.24})^{4.17}$$

$$= \left(\frac{1302 + 3645 + 1542 + 6457}{42,900}\right)^{4.17}$$

... = 0.0068 St/st

Time of Concentration

Basin A:

weighted slope = 1% grassed waterway Fig 3-2 V= 1.6 Sps L= 7400'
t = 77 mins.

Basin A&B:

weighted slope = 0.66%

grassed waterway Fig 3.2 V = 1.6 Sps

L= 11,600'

t = 121 mins.

U.D.F.C.D. CUHP RUNOFF ANALYSIS

EXECUTED ON DATE

AT TIME

CUHPE/PC VERSION MODIFIED IN JANUARY 1985

PRINT OPTION NUMBER SELECTED FOR THIS BASIN IS O

CSI EAST DRAINAGE BASIN A

BASIN ID: A -- BASIN COMMENT: 288 ACRES BASIN A

AREA OF BASIN LENGTH OF BASIN DIST TO CENTROID IMPERVIOUS AREA SLOPE UNIT DURATION (SOMI) (MI) (PCT) (FT/FT) (MIN)

.45 1.40 .70 .00 .0100 5.00

COEFFICIENT COEFFICIENT (REFLECTING TIME TO PEAK) (RELATED TO PEAK RATE OF RUNOFF)

. 163

.312

CALCULATED UNIT HYDROGRAPH

TIME TO PEAK TIME OF CONCENTRATION PEAK RATE OF RUNOFF UNIT HYDROGRAPH PEAK VOLUME OF RUNOFF (MIN) (CFS/SQMI) (CFS) (AF)

31.75

77.00

410.04

184.52

24.00

WIDTH AT 50 = 73. MIN. WIDTH AT 75 = 38. MIN. K50 = .26 K75 = .35

RAINFALL LOSSES INPUT W/ BASIN DATA

MAX. PERVIOUS RET. = .35 IN.

INFILTRATION = 4.50 IN./HR.

MAX. IMPERVIOUS RET. = .10 IN.

DECAY = .00180/SECOND FNINFL = .60 IN./HR.

TIME	UNIT	•	TIME	UNIT	•	TIME	UNIT	•
	HYDROGRAPH	*		HYDROGRAPH	*		HYDROGRAPH	*
		*			*		III DROOKAI II	-
		*			*			
0.	0.	*	100.	77.	*	200.	21.	*
5.	22.	*	105.	72.	*	205.	20.	*
10.	66.	*	110.	68.	*	210.	19.	*
15.	113.	*	115.	63.	*	215.	18.	*
20.	150.	*	120.	60.	*	220.	17.	*
25.	174.	*	125.	56.	*	225.	15.	
30.	184.	*	130.	52.	*	230.	15.	*
35.	183.	*	135.	49.	*	235.		*
40.	174.	*	140.	46.	*	240.	14.	*
45.	162.	*	145.	43.	*	245.	13. 12.	
50.	149.	*	150.	41.	*	250.	11.	-
55.	140.	*	155.	38.	*	255.	11.	*
60.	133.	*	160.	36.	*	260.	10.	*
65.	125.	*	165.	33	*	265.	9.	*
70.	117.	*	170.	31.	*	270.	9.	*
75.	109.	*	175.	29.	*	275.	8.	*
80.	101.	*	180.	28.	*	280.	8.	*
85.	94.	*	185.	26.	*	285.	0.	*
90.	87.	*	190.	24.	*	0.	0.	
95.	82.	*	195.	23.	*	0.	0.	*

•

BASIN ID: A -- BASIN COMMENT: 288 ACRES BASIN A

**** STORM NO. = 1 **** DATE OR RETURN PERIOD = 100 YR.

TIME (MIN.)	INCREMENT RAINFALL (IN)	TOTAL EXCESS	STORM HYDROGRAPH	*	TIME	INCREMENT RAINFALL	TOTAL EXCESS	STORM HYDROGRAPH	*
(1114.)	(IN)	PRECIP	(CFS)	# -	(MIN.)	(IN)	PRECIP	(CFS)	*
				*					*
0.	•00	•000	•	*					*
5.	•03		0.	*	165.	.00	•000	81.	*
10.	•08	.000	0.	*	170.	•00	.000	76.	*
15.		.000	0.	*	175.	•00	.000	71.	*
20.	• 13 • 22	•000	0.	*	180.	•00	.000	67.	*
25.	.38	.000	0.		185.	•00	•000	63.	*
30.	• 6 9	•077 •621	2.	*	190.	•00	•000	59.	*
35.	•38	•325	19.	*	195.	•00	.000	55.	*
40.	• 2 2		57.		200.	.00	.000	52.	*
45.	• 17	. 164	107.	*	205.	•00	.000	48.	*
50.	.14	.117	157.	*	210.	• 00	.000	45.	*
55.	• 14	.086	200.	*	215.	•00	•000	43.	*
60.	• 11	.059	230.	*	220.	.00	•000	40.	*
65.	• 11	.059	248.	*	225.	•00	.000	38.	*
70.	•05	•060	255.	ж	230.	•00	•000	35.	*
75.	• 05	.005	255.	*	235.	•00	.000	33.	*
80.	•03	•005	248.		240.	•00	•000	31.	*
85.	.03	•000 •000	239.	*	245.	•00	•000	29.	*
90.	.03	.000	229. 218.	*	250.	•00	•000	27.	*
95.	.03	•000	205.	*	255. 260.	•00	•000	26.	*
100.	د٥.	•000	192.	*		•00	•000	24.	*
105.	.03	•000		*	265.	•00	.000	22.	*
110.	•03	•000	179.	*	270.	•00	•000	21.	*
115.	.03		167.		275.	•00	•000	20.	*
120.	•03	•000	156.	*	280.	•00	•000	19.	*
125.		.000	146.	*	285.	• 00	.000	17.	*
	•00	•000	136.	*	290.	.00	.000	16.	*
130.	•00	•000	127.	*	295.	•00	.000	15.	*
135.	•00	•000	119.	*	300.	•00	.000	14.	*
140.	•00	•000	112.	*	305.	•00	.000	13.	*
145.	•00	. •000	105.	*	310.	•00	.000	8.	*
150.	•00	.000	98.	*	315.	•00	.000	5.	*
155.	• 0 0	•000	92.	*	320.	.00	.000	3.	*
160.	•00	•000	86.	*	325.	•00	.000	2.	*

TOTAL PRECIP. = 3.18 (1-HOUR RAIN = 2.75) EXECESS PRECIP. = 1.578 INCHES VOLUME OF EXCESS PRECIP = 38. ACRE-FEET
PEAK Q = 255. CFS TIME OF PEAK = 65. MIN.
INFILT. = 4.50 IN/HR DECAY = .00180 FNINF = .60 IN/HR
MAX.PERV.RET. = .35 IN. MAX.IMP.RET. = .10 IN.

RATIONAL FORMULA C = .50 I = 2.3 INCHES/HOUR A = 288.0 ACRES 0 = 334. CFS

*** WARNING : THE AREA IS TOO LARGE (GREATER THAN 160 ACRES) FOR AN APPLICATION OF THE RATION, AND THE ABOVE ESTIMATED PEAK MAY NOT BE RIGHT.

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U.D.F.C.D. CUHP RUNOFF ANALYSIS EXECUTED ON DATE

AT TIME

.0060

5.00

CUHPE/PC VERSION MODIFIED IN JANUARY 1985

PRINT OPTION NUMBER SELECTED FOR THIS BASIN IS O

CSI EAST DRAINAGE BASIN A&B

BASIN ID: A&B -- BASIN COMMENT: 493 ACRES BASIN A&B

AREA OF BASIN LENGTH OF BASIN DIST TO CENTROID IMPERVIOUS AREA SLOPE UNIT DURATION (SOMI) (MI) (HI) (PCT) (FT/FT) (MIN) .77 2.20

.00

COEFFICIENT COEFFICIENT

(REFLECTING TIME TO PEAK) (RELATED TO PEAK RATE OF RUNOFF)

1.10

.163 .339

CALCULATED UNIT HYDROGRAPH

TIME TO PEAK (MIN) PEAK RATE OF RUNOFF UNIT HYDROGRAP PEAK VOLUME OF RUNOFF (CFS/SQMI) (AF)

53.53 254.76 196.17 41.07

WIDTH AT 50 = 118. MIN. WIDTH AT 75 = 61. MIN. K50 = .27 K75 = .37

RAINFALL LOSSES INPUT W/ BASIN DATA

MAX. IMPERVIOUS RET. = .10 IN. MAX. PERVIOUS RET. = .35 IN. INFILTRATION = 4.50 IN./HR. DECAY - .00180/SECOND FNINFL = .60 IN./HR.

TIME	UNIT	•	TIME	UNIT	•	TIME	UNIT	•
	HYDROGRAPH	*		HYDROGRAPH	*		HYDROGRAPH	*
		*			*			
		*			*			
0.	0.	*	155.	86.	*	310.	25.	*
5.	10.	*	160.	83.	*	315.	24.	*
10.	31.	*	165.	80.	*	320.	23.	*
15.	59.	*	170.	76.	*	325.	22.	*
20.	89.	*	175.	73.	*	330.	21.	*
25.	119.	*	180.	70.	*	335.	20.	*
30.	144.	*	185.	68.	*	340.	19.	*
35.	165.	*	190.	65.	*	345.	18.	*
40.	180.	*	195.	62.	*	350.	18.	*
45.	190.	*	200.	60.	*	355.	17.	*
50.	195.	*	205.	58.	*	360.	16.	*
55.	196.	*	210.	55.	*	365.	16.	*
60.	193.	*	215.	53.	*	370.	15.	*
65.	188.	*	220.	51.	*	375.	14.	*
70.	181.	*	225.	49.	*	380.	14.	*
75.	173.	*	230.	47.	*	385.	13.	*
80.	164.	*	235.	45.	*	390.	13.	*
85.	156.	*	240.	43.	*	395.	12.	*
90.	149.	*	245.	42.	*	400.	12.	*
95.	144.	*	250.	40.	*	405.	11.	*
100.	139.	*	255.	38.	*	410.	11.	*
105.	134.	*	260.	37.	*	415.	10.	*
110.	128.	*	265.	35.	*	420.	10.	*
115.	123.	*	270.	34.	*	425.	10.	*
120.	118.	*	275.	33.	*	430.	9.	*
125.	113.	*	280.	31.	*	435.	9.	*
130.	108.	*	285.	30.	*	440.	9.	*
135.	102.	*	290.	29.	*	445.	8.	*
140.	97.	*	295.	28.	*	450.	8.	
145.	94.	*	300.	27.	*	455.	8.	*
150.	90.	*	305.	26.	*	460.		*
. • - •	, , ,		307.	2 17 ·	•-	400	0.	_

**** STORM NO. = 1 **** DATE OR RETURN PERIOD = 100 YR

_										
		INCREMENT	TOTAL	STORM	*		INCREMENT	TOTAL	STORM	*
	TIME	RAINFALL	EXCESS	HYDROGRAPH	*	TIME	RAINFALL	EXCESS	HYDROGRAPH	*
_	(MIN.)	(IN)	PRECIP	(CFS)	*	(MIN.)	(IN)	PRECIP	(CFS)	*
					*	,	,		(0.5)	*
					*					
_	0.	.00	.000	0.	*	255.	• 0 0	.000	79.	
	5.	• 0 3	.000	0.	*	260.	•00	.000		*
	10.	.08	•000	0.	*	265.	•00		76.	*
_	15.	• 13	•000	0.		270.	-	.000	73.	
	20.	. 2 2	.000	0.			•00	•000	70.	*
	25.	• 38	•077	-		275.	•00	.000	67.	*
	30.	•69		1.	-	280.	•00	•000	65.	*
•			.621	9.	*	285.	•00	.000	62.	*
	35.	• 38	• 325	27.	*	290.	•00	•000	60.	*
	40.	• 2 2	. 164	55.	*	295.	•00	.000	57.	*
-	45.	• 17	• 117	90.	*	300.	•00	•000	55.	*
	50.	. 14	•086	128.	*	305.	•00	.000	53.	*
	55.	• 11	.059	166.	*	310.	•00	.000	51.	*
_	60.	•11	.059	201.	*	315.	•00	.000	49.	*
	65.	• 11	•060	232.	*	320.	.00	.000	47.	*
	70.	• 0.5	.005	257.	*	325.	•00	.000	45.	*
_	75.	• 0.5	• 00 5	276.	*	330.	•00	.000	43.	*
	80.	•03	•000	288.	*	335.	•00	.000	41.	*
	85.	•03	•000	294.	*	340.	•00	.000	40.	*
	90.	•03	•000	295.	*	345.	• 0 0	.000	38.	*
	95.	.03	•000	292.	*	. 350.	.00	.000	37.	*
	100.	•03	.000	284.	*	355.	•00	.000	35.	*
	105.	•03	•000	275.	*	360.	•00	.000	34.	*
∠	110.	•03	.000	264.	*	365.	•00	.000	32.	*
Car A	115.	• 03	•000	254.	*	370.	•00	.000	31.	*
	120.	•03	.000	244.	*	375.	•00	.000	30.	*
_	125.	.00	•000	234.	*	380.	• 0 0	.000	29.	*
	130.	•00	.000	225.	*	385.	.00	.000	28.	*
	135.	• 00	•000	216.	*	390.	•00	.000	26.	*
_	140.	.00	•000	208.	*	395.	•00	.000	25.	*
	145.	•00	•000	199.	*	400.	•00	.000	24.	*
	150.	•00	.000	191.	*	405.	•00	•000	23.	*
_	155.	•00	.000	182.	*	410.	•00	•000	23.	*
	160.	.00	.000	174.	*	415.	•00	•000	22.	*
	165.	.00	.000	166.	*	420.	•00	•000	21.	*
_	170.	•00	.000	159.	*	425.	•00	•000	20.	*
	175.	.00	.000	152.	*	430.	•00	.000	_	*
	180.	•00	•000	146.	*	435.	•00		19.	*
_	185.	•00	•000	140.	*	440.		•000	18.	
	190.	•00	•000	134.	*		•00	.000	18.	*
	195.	.00	•000	129.	*	445.	•00	•000	17.	*
_	200.	•00	.000	124.	*	450.	•00	•000	16.	*
-	205.	•00	.000	119.	_	455.	•00	•000	16.	*
	210.	•00	.000		_	460.	•00	.000	15.	*
	215.	•00	.000	114.	*	465.	•00	.000	14.	*
_	220.	•00	•000	109.	*	470.	•00	•000	14.	*
	225.	•00	•000	105. 101.	*	475.	•00	•000	13.	*
	230.	•00	•000		*	480.	•00	.000	12.	*
-	235.	•00	•000	97.	*	485.	•00	•000	7.	*
	240.	•00	•000	93.	*	490.	•00	•000	5.	*
	245.	•00		89.	*	495.	•00	•000	3.	*
- '	250.		•000	86.		500.	•00	•000	2.	*
	230.	•00	.000	82.	*	505.	•00	•000	1.	*

TOTAL PRECIP. = 3.18 (1-HOUR RAIN = 2.75) EXECESS PRECIP. = 1.578 INCHES VOLUME OF EXCESS PRECIP = 65. ACRE-FEET
PEAK O = 295. CFS TIME OF PEAK = 90. MIN.
INFILT. = 4.50 IN/HR DECAY = .00180 · FNINF = .60 IN/HR
MAX.PERV.RET. = .35 IN. MAX.IMP.RET. = .10 IN.

U.D.F.C.D. CUHP RUNOFF ANALYSIS EXECUTED ON DATE

AT TIME

CUHPE/PC VERSION MODIFIED IN JANUARY 1985

PRINT OPTION NUMBER SELECTED FOR THIS BASIN IS O

CSI WEST DRAINAGE BASIN C

BASIN ID: C -- BASIN COMMENT: 4480 ACRES BASIN C

AREA OF BASIN LENGTH OF BASIN DIST TO CENTROID IMPERVIOUS AREA SLOPE UNIT DURATION (SOMI) (MI) (MI) (PCT) (FT/FT) (MIN)

7.00 8.30 5.20 •00 .0060 5.00

COEFFICIENT COEFFICIENT (REFLECTING TIME TO PEAK) (RELATED TO PEAK RATE OF RUNOFF)

. 163

CALCULATED UNIT HYDROGRAPH

TIME TO PEAK PEAK RATE OF RUNOFF UNIT HYDROGRAP PEAK VOLUME OF RUNOFF (MIN) (CFS/SOMI) (CFS) (AF)

205.93 88.98 622.89 373.33

WIDTH AT 50 = 337. MIN. WIDTH AT 75 = 175. MIN. K50 = .35 K75 = .45

RAINFALL LOSSES INPUT W/ BASIN DATA

MAX. PERVIOUS RET. = .35 IN. MAX. IMPERVIOUS RET. = .10 IN. INFILTRATION = 3.00 IN./HR. DECAY = .00180/SECOND FNINFL = .50 IN./HR.

		TIME	UNIT HYDROGRAPH	1 *	TIME	UNIT HYDROGRAPH		TIME	UNIT HYDROGRAPH	1	
		0.	0.		420.	216	*	0.40	0.5		
1		5.	5.	*	425.	318. 312.		840. 845.	85. 83.		
*		10.	12.	*	430.	307.	*	850.	82.	•	
		15.	22.	*	435.	302.	*	855.	81.		
		20. 25.	34. 47.	*	440. 445.	297. 293.	*	860.	79.	*	
		30.	63.	•	450.	288.	*	865. 870.	78. 77.		
		35.	80.	*	455.	283.	*	875.	76.	*	
	_	40.	98.	*	460.	279.	*	880.	75.	*	
		45. 50.	118.	*	465.	275.	*	885.	73.	*	
		55.	138. 160.	*	470. 475.	270. 266.	*	890. 895.	72. 71.		
	•	60.	182.	*	480.	262.	*	900.	70.	*	
		65.	205.	*	485.	258.		905.	69.	*	
		70.	228.	*	490.	254.	*	910.	68.	*	
		75. 80.	251. 275.	*	495. 500.	250. 246.	*	915.	67.	*	
		85.	298.	*	505.	242.	*	920. 925.	66. 65.		
		90.	321.	*	510.	238.	*	930.	64.	*	
		95. 100.	344.	*	515.	235.	*	935.	63.	*	
		105.	366. 387.	*	520. 525.	231. 227.	*	940. 945.	62.	•	
		110.	407.		530.	224.		950.	61. 60.		
		115.	426.	*	535.	220.	*	955.	59.	*	
		120.	444.	•	540.	217.	*	960.	58.	•	
		125. 130.	461. 479.	*	545. 550	214.	*	965.	57•	*	
		135.	499.	*	550. 555.	210. 207.		970. 975.	56. 55.	-	
	1	140.	517.	*	560.	204.	*	980.	54.		
		145.	533.	*	565.	201.	*	985.	54.	*	
		150.	54R.	*	570.	197.	*	990.	53.	*	
		155. 160.	561. 573.	*	575. 580.	194.	*	995.	52.	*	
		165.	584.	*	585.	191. 188.		1000. 1005.	51. 50.	-	
		170.	593.	*	590.	185.	*	1010.	50.	*	
		175.	601.	*	595.	183.	*	1015.	49.	•	
		180. 185.	608.	*	600.	180.	*	1020.	48.	*	
		190.	613. 617.	*	605. 610.	177.	*	1025.	47.	*	
		95.	620.	*	615.	174. 171.	-	1030. 1035.	47. 46.	-	
		00.	622.	*	620.	163.	*	1040.	45.		
in.		05.	623.	*	625.	166.	*	1045.	44.	*	
		10.	623.	*	630.	164.	*	1050.	44.	*	
•		20.	621. 619.	*	635.	161.	*	1055.	43.	*	
		25.	615.	*	640. 645.	158. 156.		1060. 1065.	42.	*	
		30.	611.	*	650.	154.	*	1070.	42. 41.	*	
		35.	606.	*	655.	151.	*	1075.	40.	*	•
		40. 45.	600.	*	660.	149.	*	1080.	40.	*	
		50.	593. 586.	*	665. 670.	146. 144.	*	1085. 1090.	39.	*	
		55.	577.	*	675.	142.	*	1095.	39. 38.	*	
		60.	568.	*	680.	140.	*	1100.	37.	*	
		65. 70.	558. 548	*	685.	138.	*	1105.	37.	*	
	_	75.	548. 537.	*	690. 695.	135. 133.	*	1110. 1115.	36.	*	
		80.	525.	*	700.	131.	*	1120.	36. 35.	*	
		85.	513.	*	705.	129.	*	1125.	34.	*	
		90. 95.	500. 487.	*	710.	127.	*	1130.	34.	*	
		00.	474.	*	715. 720.	125. 123.		1135. 1140.	33. 33.	*	
		05.	464.	*	725.	121.	*	1145.	32.		
		10.	457.	*	730.	110.	~ #	TI50.	32.	*	* 1 delicate or ***********************************
		15.	451.	*	735.	118.	*	1155.	31.	*	
		20. 25.	445. 438.	*	740.	116.	*	1160.	31.	*	
		30.	432.	*	745. 750.	114. 112.	*	1165. 1170.	30.	*	
		35.	426.	*	755.	110.	*	1175.	30. 29.		
	34		419.	*	760.	109.	*	1180.	29.	*	
		45.	413.	*	765.	107.	•	1185.	29.	*	
		50. 55.	407. 400.	*	770. 775.	105.	*	1190.	28.	*	
		50.	394.	*	780.	104. 102.	*	1195. 1200.	28.	*	
	3 6	55.	388.	*	785.	100.	*	1205.	27. 27.	*	
		70.	381.	*	790.	99.	*	1210.	26.	*	
		75. 30.	375.		795.	97.	*	1215.	26.	*	
		35.	369. 362.	*	800. 805.	96. 94.	*	1220. 1225.	26.	*	
		90.	356.	*	810.	93.	*	1230.	25. 25.	*	
	3 9	95.	350.	*	815.	91.	*	1235.	24.	*	
		00.	343.	*	820.	90.	*	1240.	24.	*	
)5. 10.	337. 331.	*	825.	89.	*	1245.	24.	*	
1		15.	324.	*	830. 835.	87. 86.	*	0. 5.	0. 0.	*	
4			•			00.		٠.	٠.	-	

BASIN ID: C -- BASIN COMMENT: 4480 ACRES BASIN C

**** STORM NO. = 1 **** DATE OR RETURN PERIOD = 100 YR

TIME	INCREMENT RAINFALL	TOTAL EXCESS	STORM HYDROGRAPH	*	TIME	INCREMENT	TOTAL	STORM	*
(MIN.)	(IN)	PRECIP	(CFS)	*	(MIN.)	RAINFALL (IN)	EXCESS PRECIP	HYDROGRAPH	*
				*	(112114)	(117)	PRECIP	(CFS)	*
0.	0.0		_	*					*
5.	.00 .03	.000 .000	0.	*	625.	•00	.000	323.	*
10.	-08	.000	0. 0.	*	630. 635.	•00	.000	318.	*
15.	. 13	.000	0.	*	640.	•00 •00	•000 •000	313.	*
20.	• 2 2	.000	0.	*	645.	•00	•000	308. 303.	*
25.	.38	• 153	1.	*	650.	•00	.000	298.	*
30. 35.	• 69	.635	5.	*	655.	•00	.000	294.	*
40.	•38 •22	.337	13.	*	660.	• 00	.000	289.	*
45.	• 17	• 175 • 127	24. 39.	*	665.	•00	•000	285.	*
50.	. 14	.095	57 .	*	670. 675.	•00	.000	280.	*
55.	• 11	.068	78.	*	680.	.00 .00	.000 .000	276.	*
60.	-11	.068	103.	*	685.	•00	•000	271. 267.	*
65.	• 11	.068	130.	*	690.	•00	•000	263.	*
70.	• 05	.013	160.	*	695.	•00	•000	259.	*
75. 80.	•05	.013	192.	*	700.	• 00	•000	255.	*
85.	.03 .03	•000 •000	226.	*	705.	• 00	•000	251.	*
90.	•03	•000	262. 300.	*	710.	•00	•000	247.	*
95.	•03	•000	338.	*	715. 720.	•00	•000	243.	*
100.	•03	•000	378.	*	725.	•00 •00	•000 •000	239.	*
105.	.03	•000	418.	*	730.	•00	•000	236. 232.	*
110.	•03	•000	458.	*	735.	•00	.000	228.	*
115.	• 03	•000	498.	*	740.	•00	•000	225.	*
120. 125.	•03 •00	•000	538.	*	745.	•00	.000	221.	*
130.	•00	.000 .000	578.	*	750.	•00	.000	218.	*
135.	•00	•000	616. 653.	*	755. 760.	•00	•000	214.	*
140.	•00	.000	689.	*	765.	•00 •00	•000	211.	*
145.	•00	.000	723.	*	770.	•00	•000 •000	208.	*
150.	•00	.000	756.	*	775.	•00	•000	205. 201.	*
155. 160.	•00	.000	789.	*	780.	•00	.000	198.	*
165.	•00 •00	•000	822.	*	785.	•00	.000	195.	*
170.	•00	•000 •000	854. 885.	*	790.	•00	.000	192.	*
175.	•00	•000	913.	*	795. 800.	•00	•000	189.	*
180.	•00	.000	940.	*	805.	•00 •00	•000	186.	*
185.	•00	•000	964.	*	810.	•00	•000 •000	183. 180.	*
190.	•00	.000	986.	*	815.	•00	•000	178.	*
195.	•00	•000	1006.	*	820.	•00	•000	175.	*
200. 205.	•00 •00	•000	1024.	*	825.	.00	•000	172.	*
210.	•00	•000 •000	1039.	*	830.	•00	•000	169.	*
215.	•00	•000	1052. 1063.	*	835.	•00	•000	167.	*
220.	•00	•000	1072.	*	840. 845.	.00 .00	•000	164.	*
225.	•00	•000	1078.	*	850.	•00	.000 .000	162.	*
230.	•00	•000	1083.	*	855.	•00	•000	159. 157.	*
235. 240.	•00	•000	1085.	*	860.	•00	•000	154.	*
240.	•00 •00	•000	1086.	*	865.	•00	.000	152.	*
250.	•00	•000 •000	1085.	*	870.	•00	.000	149.	*
255.	•00	•000	1082. 1077.	*	875.	•00	•000	147.	*
260.	•00	.000	1070.	*	880. 885.	•00 •00	•000	145.	*
265.	•00	.000	1062.	*	890.	•00	•000 •000	142.	*
270.	•00	.000	1053.	*	895.	•00	•000	140. 138.	*
275.	•00	•000	1042.	*	900.	•00	•000	136.	*
280. 285.	•00 •00	•000	1029.	*	905.	•00	.000	134.	*
290.	•00	.000 .000	1016. 1001.	*	910.	.00	•000	132.	*
295.	•00	•000	984.	*	915.	•00	•000	130.	*
300.	•00	.000	967.	*	920. 925.	•00 •00	•000	128.	*
305.	•00	.000	948.	*	930.	•00	•000 •000	126. 124.	*
310.	•00	.000	929.	*	935.	•00	•000	122.	*
315. 320.	•00	•000	908.	*	940.	•00	.000		*
325.	•00 •00	•000	-	*	945.	•00	.000	118.	*
-	• • • •	.000	865.	*	950.	• 00	.000	116.	*

۹											
	330.	•00	.000	845.	*	955.	.00	.000	114.		
	335.	•00	•000	829.	*					*	
	340.	•00				960.	•00	.000	1,13.	*	
\ \	345.		•000	815.	*	965.	•00	.000	111.	*	
)		• 00	•000	801.	*	970.	•00	•000	109.	*	
	350.	• 0 0	•000	788.	*	975.	.00	.000	107.	*	
	355.	• 0 0	•000	775.	*	980.	•00	.000	106.	*	
~	360.	•00	.000	763.	*	985.	•00	.000	104.		
	365.	• 00	.000	752.	, *	990.	•00				
	370.	•00	•000	741.	*			.000	102.	*	
~	375.	•00	.000		*	995.	•00	•000	101.	*	
	380.			730.		1000.	•00	•000	99.	*	
		•00	.000	718.	*	1005.	•00	•000	98.	*	
_	385.	•00	.000	707.	*	1010.	•00	.000	96.	*	
_	390.	•00	.000	696.	*	1015.	•00	.000	95.	*	
	395.	•00	•000	685.	*	1020.	.00	.000	93.		
	400.	•00	.000	674.	*	1025.	•00	.000	92.		
_	405.	• 00	.000	663.	*	1030.	.00	•000		*	
	410.	•00	.000	652.	*	1035.			90.		
	415.	•00	•000	641.		1040.	•00	•000	89.	*	
_	420.	•00	•000	630.	*		•00	•000	88.	*	
	425.	•00	.000	618.	*	1045.	•00	•000	86.	*	
	430.	•00	•000	607.	*	1050.	•00	•000	85.	*	
_	435.	•00	•000		*	1055.	•00	•000	84.	*	
	440.	•00	.000	596.	*	1060.	•00	•000	82.	*	
	445.	•00		585.		1065.	•00	•000	81.	*	
	450.		•000	574.	*	1070.	•00	•000	80.	*	
~	455.	•00	•000	563.	*	1075.	•00	•000	78.	*	
		• 00	•000	553.	*	1080.	•00	.000	77.	*	
	460.	•00	•000	544.	*	1085.	•00	.000	76.	*	
<u> </u>	465.	•00	•000	535.	*	1090.	•00	.000	75.	*	
	470.	•00	•000	526.	*	1095.	.00	.000	74	*	
	475.	•00	•000	518.	*	1100.	•00	•000	72.	*	
$\overline{}$	480.	•00	.000	509.	*	1105.	•00				
	485.	•00	.000	501.	*	1110.		.000	71.	*	
	490.	•00	•000	493.	*	1110.	•00	•000	70.	*	
L	495.	•00	•000		*	1115.	• 0 0	.000	69.	*	
•	500.	•00		486.		1120.	•00	•000	68.	*	
	505.		•000	478.	*	1125.	•00	•000	67.	*	
	510.	•00	•000	471.	*	1130.	•00	.000	66.	*	
		•00	•000	463.	*	1135.	•00	•000	65.	*	
. 8	515.	•00	•000	456.	*	1140.	•00	•000	64.	*	
J	520.	• 0 0	•000	449.	*	1145.	.00	.000	63.	*	
	525.	•00	•000	442.	*	1150.	•00	•000	62.		
	530.	•00	.000	435.	*	1155.	•00	.000	61.		
	535.	• 0 0	•000	428.	*	1160.	•00	•000			
<u> </u>	540.	•00	.000	422.	*	1165.	•00		60.	*	
	545.	•00	.000	415.	*	1170.	•00	•000	59.	*	
	550.	•00	.000	409.	*	1175.		•000	58.	*	
_	555.	• 00	.000	402.	*	1180.	•00	•000	57.	*	
-	560.	•00	•000	396.			•00	.000	56.	*	
	565.	•00	•000		*	1185.	•00	•000	55.	*	
	570.	•00	•000	390.	*	1190.	•00	•000	55.	*	
_	575.			384.		1195.	•00	•000	54.	*	
	580.	•00	•000	378.	*	1200.	•00	•000	53.	*	
		•00	•000	372.	*	1205.	•00	•000	52.	*	
_	585.	•00	•000	366.	*	1210.	•00	•000	51.	*	
	590.	•00	•000	360.	*	1215.	•00	•000	50.	*	
	595.	• 0 0	.000	355.	*	1220.	•00	•000		*	
$\overline{}$	600.	•00	.000	349.	*	1225.	•00		50.		
	605.	•00	.000	344.	*	1230.		.000	49.	*	
	610.	•00	•000	338.	*		•00	•000	48.	*	
	615.	•00	•000	333.	*	1235.	•00	•000	47.	*	
•	620.	•00	•000			1240.	•00	•000	47.	*	
			• • • •	328.	*	1245.	•00	.000	46.	*	

TOTAL PRECIP. = 3.18 (1-HOUR RAIN = 2.75) EXECESS PRECIP. = 1.751 INCHES VOLUME OF EXCESS PRECIP = 654. ACRE-FEET PEAK O = 1086. CFS TIME OF PEAK = 240. MIN. INFILT. = 3.00 IN/HR DECAY = .00180 FNINF = .50 IN/HR MAX.PERV.RET. = .35 IN. MAX.IMP.RET. = .10 IN.

U.D.F.C.D. CUHP RUNOFF ANALYSIS EXECUTED ON DATE

AT TIME

CUHPE/PC VERSION MODIFIED IN JANUARY 1985

PRINT OPTION NUMBER SELECTED FOR THIS BASIN IS 0

CSI WEST DRAINAGE BASIN D

BASIN ID: D -- BASIN COMMENT: 4544 ACRES BASIN D

AREA OF BASIN LENGTH OF BASIN DIST TO CENTROID IMPERVIOUS AREA SLOPE UNIT DURATION (SOMI) (MI) (PCT) (FT/FT) (MIN)

7.10

8.10

4.60

.00

.0060

5.00

COEFFICIENT COEFFICIENT (REFLECTING TIME TO PEAK) (RELATED TO PEAK RATE OF RUNOFF)

. 163

.472

CALCULATED UNIT HYDROGRAPH

TIME TO PEAK PEAK RATE OF RUNOFF UNIT HYDROGRAP PEAK VOLUME OF RUNOFF (MIN) (CFS/SOMI) (CFS) (AF)

192.07

95.69

679.42

378.67

WIDTH AT 50 = 314. MIN. WIDTH AT 75 = 163. MIN. K50 = .35 K75 = .45

RAINFALL LOSSES INPUT W/ BASIN DATA

MAX. PERVIOUS RET. = .35 IN. MAX. IMPERVIOUS RET. = .10 IN. INFILTRATION = 3.00 IN./HR. DECAY = .00180/SECOND FNINFL = .50 IN./HR.

>

TIME	UNIT		TIME	UNIT		T 1 MP	****			
	HYDROGRAPH		11112	HYDROGRAPH	•	TIME	UNIT HYDROGRAPH	*		
		ě			•			••		
0.	0.	•	420.	313.	*	840.	75.	*		
5. 10.	6. 15.	*	425. 430.	308. 303.	*	845.	74.	*		
15.	27.	*	435.	298.	*	850. 855.	73. 72.	*		
20. 25.	41. 57.	*	440.	293.	*	860.	71.	*		
30.	76.	*	445. 450.	288. 283.	*	865. 870.	69. 68.	*		
35.	96.	*	455.	278.	*	875.	67.	*		
40. 45.	118. 142.	*	460. 465.	273. 269.	*	880. 885.	66. 65.	*		
50.	166.	*	470.	264.		890.	64.			
55. 60.	192.	*	475.	260.	*	895.	63.	*		
65.	218. 245.		480. 485.	255. 251.	*	900. 905.	62.	*		
70.	272.	*	490.	247.	*	910.	61. 60.	*		
75. 80.	300. 327.	*	495. 500.	243.	*	915.	59.	*		
85.	354.	*	505.	239. 235.	*	920. 925.	58. 57.	*		
90.	380.	*	510.	231.	*	930.	56.	*		
95. 100.	406. 430.	*	515. 520.	227. 223.	*	935. 940.	55.	*		
105.	454.	*	525.	219.	*	945.	54. 53.	*		
110.	476.	*	530.	216.	*	950.	52.	*		
115. 120.	496. 516.		535. 540.	212. 208.		955. 960.	51.	*		
125.	539.	*	545.	205.	*	965.	50. 49.	*	•	
130. 135.	561. 580.	*	550.	202.	*	970.	49.	•		
140.	598.	•	555. 560.	198. 195.	*	975. 980.	48. 47.	*		
145.	613.	*	565.	192.	*	985.	46.	÷		
150. 155.	627. 639.	*	570. 575.	188.	*	990.	45.	*		
160.	650.	*	580.	185. 182.	*	995. 1000.	45. 44.	*		
165. 170.	658.	*	585.	179.	*	1005.	43.	*		
175.	666. 671.	*	590. 595.	176. 173.	*	1010. 1015.	42. 42.	*		
180.	675.		600.	170.	*	1020.	41.	*		
185. 190.	678. 679.	*	605. 610.	167.	*	1025.	40.	*		
195.	679.	*	615.	164. 162.		1030. 1035.	40. 39.	*		
200. 205.	678.	*	620.	159.	*	1040.	38.	*		
210.	675. 671.	*	625. 630.	156. 154.	*	1045.	38.	*		
215.	666.	*	635.	151.	*	1050. 1055.	37. 36.	*		
220. 225.	660.	*	640.	149.	*	1060.	36.	*		
230.	653. 644.		645. 650.	146. 144.	*	1065. 1070.	35.	*		
235.	635.	*	655.	141.	*	1075.	35. 34.	*		
240. 245.	625. 614.	*	660.	139.	*	1080.	33.	*		
250.	602.	*	665. 670.	137. 134.	*	1085. 1090.	33. 32.	*		
255.	589.	*	675.	132.	*	1095.	32.	*		
260. 265.	576. 561.	*	680. 685.	130.	*	1100.	31.	*		
270.	547.	*	690.	128. 125.	*	1105.	31. 30.	*		
275. 280.	531.	*	695.	123.	*	1115.	30.	*		
285.	515. 505.	*	700. 705.	121. 119.	*	1120. 1125.	29. 29.	*		
290. 295.	497.	*	710.	117.	*	1130.	28.	*		
300.	490. 482.	*	715. 720.	115. 113.	*	1135. 1140.	28. 27.	*		
305.	475.	*	725.	111.	*	1145.	27.	*		
310. 315.	467. 460.	*	730. 735.		*	1150. 1155.		* *		
320.	453.	*	740.	106.	Ŕ	1160.		*		
325. 330.	445 . 438.	*	745.		*	7765.	, J.			
335.	430.	*	755.		*	1170. 1175.		* *		
340. 345.	423.	*	760.	99.	*	1180.	24.	*		
350.	415. 408.	*	765. 770.	• • •	*	1185. 1190.		# #		
355.	401.	*	775.	94.	*	1195.	23.	k		
360. 365.	393. 386.	*	780. 785.	•	*	1200.	22.	h		
370.	378.	*	790.		*	1205.		# #		
375.	371.	*	795.	88.	*	1215.		t .		
380. 385.	363. 356.	*	800. 805.		*	1220.		.		
390.	348.	*	810.	•	*	1225. 1230.		k k		
395.	341.	*	815.	82.	*	1235.	20.	•		
400. 405.	335. 329.	*	820. 825.		*	1240. 1245.	• • •	t		
410.	324.	*	830.		- ★	0.				
415.	318.	*	835.		*	6.		ŧ		

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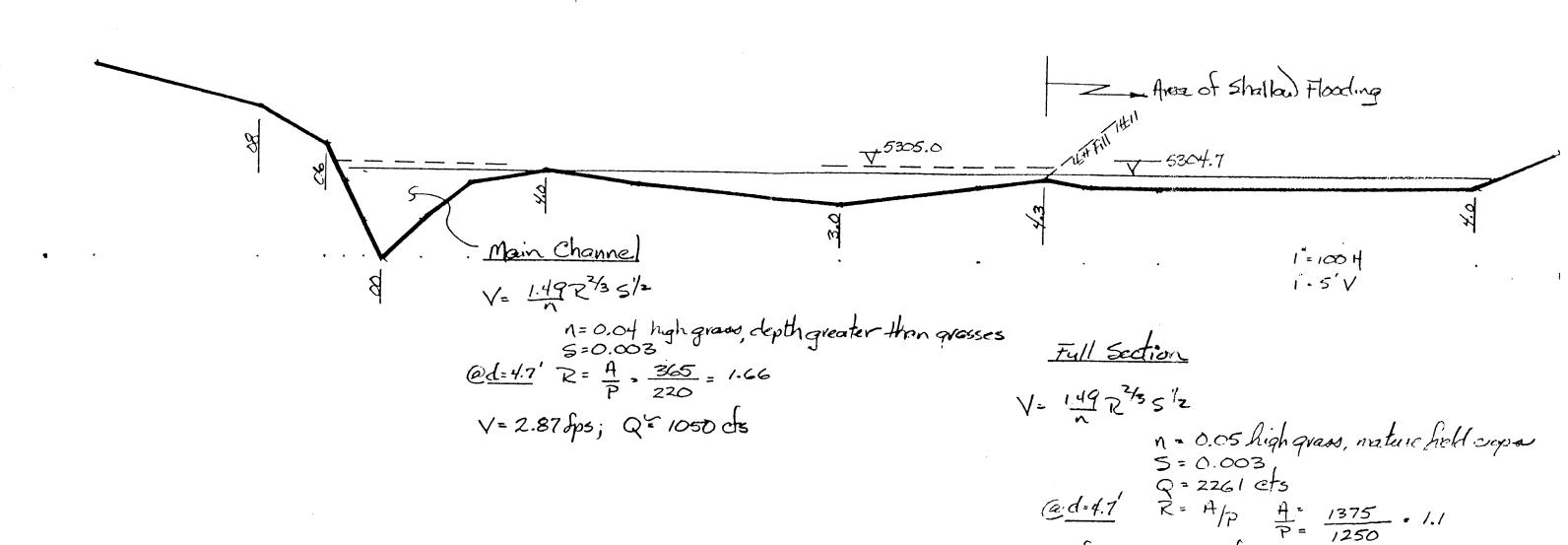
BASIN ID: D -- BASIN COMMENT: 4544 ACRES BASIN D

**** STORM NO. = 1 **** DATE OR RETURN PERIOD = 100 YR

TIME (MIN.)	INCREMENT RAINFALL (IN)	TOTAL EXCESS PRECIP	STORM HYDROGRAPH (CFS)	*	TIME	INCREMENT RAINFALL	TOTAL EXCESS	STORM HYDROGRAPH	*
(018+)	(18)	LKECIF	(CFS)	-	(MIN.)	(IN)	PRECIP	(CFS)	*
				*					*
0. 5.	.00 .03	.000	0.	*	625.	•00	.000	306.	*
10.	.08	.000 .000	0. 0.	*	630. 635.	•00 •00	.000 .000	301. 296.	*
15.	. 13	.000	0.	*	640.	•00	.000	291.	
20.	.22	.000	0.	*	645.	.00	.000	286.	*
25.	- 38	. 153	1.	*	650.	•00	.000	281.	*
30. 35.	.69 .38	.635 .337	6.	*	655.	•00	.000	277.	*
40.	.22	.175	16. 29.		660. 665.	•00 •00	.000	272. 267.	*
45.	. 17	. 127	47.	*	670.	.00	.000	263.	*
50.	- 14	.095	69.	*	675.	.00	.000	258.	*
55. 60.	.11	.068	94.	*	680.	•00	.000	254.	*
65.	• 11	.068 .068	124. 157.	*	685. 690.	.00 .00	.000	250.	*
70.	.05	.013	193.	*	695.	•00	.000	246. 242.	*
75.	.05	.013	231.	•	700.	.00	.000	237.	*
80.	•03	.000	273.	*	705.	•00	.000	233.	*
85. 90.	.03 .03	.000	315. 360.	*	710.	•00	.000	230.	*
95.	.03	•000	406.		715. 720.	.00 .00	.000	226. 222.	*
100.	•03	.000	452.	*	725.	.00	.000	218.	*
105.	.03	•000	499.	*	730.	.00	.000	215.	*
110.	.03 .03	.000	546.	*	735.	•00	•000	211.	*
120.	.03	.000	592. 638.	*	740. 745.	.00 .00	.000	207. 204.	*
125.	.00	.000	682.	•	750.	•00	.000	200.	*
130.	.00	.000	725.	*	755.	•00	•000	197.	*
135.	•00	.000	767.	*	760.	•00	.000	194.	*
140. 145.	•00 •00	.000 .000	806. 844.	*	765. 770.	•00	.000	191.	*
150.	•00	•000	883.	*	775.	•00 •00	.000 .000	187. 184.	*
155.	•00	.000	922.	*	780.	•00	•000	181.	*
160.	•00	•000	958.	*	785.	.00	.000	178.	*
165. 170.	•00	.000	991.	*	790.	•00	.000	175.	*
175.	.00 .00	.000 .000	1023. 1051.	*	795. 800.	•00	.000	172.	*
180.	•00	•000	1077.	*	805.	•00 •00	.000	169. 166.	*
185.	•00	.000	1100.	*	810.	.00	.000	164.	*
190. 195.	•00 •00	.000	1120. 1137.	*	815.	•00	.000	161.	*
200.	.00	.000	1152.	*	820. 825.	.00 .00	.000 .000	158. 155.	*
205.	•00	.000	1163.	*	830.	.00	.000	153.	*
210. 215.	.00 .00	.000	1172.	* .	835.	•00	•000	150.	*
220.	•00	.000	1178. 1182.	*	840. 845.	•00 •00	.000 .000	148. 145.	*
225.	.00	.000	1183.	*	850.	•00	.000	143.	
230.	•00	.000	1182.	*	855.	.00	.000	140.	*
235. 240.	.00 .00	.000	1179.	*	860.	•00	.000	138.	*
245.	•00	.000	1174. 1166.	*	865. 870.	.00 .00	.000 .000	136. 133.	*
250.	•00	.000	1157.	*	875.	.00	•000	131.	*
255.	•00	.000	1145.	*	880.	• 00	.000	129.	*
260. 265.	.00 .00	.000 .000	1132. 1117.	*	885.	•00	.000	127.	*
270.	.00	.000	1100.	*	890. 895.	.00 .00	.000 .000	125. 123.	*
275.	.00	.000	1082.	*	900.	•00	.000	121.	*
280. 285.	00 .00	.000 .000	1062. 1040.	*	905.	•00	.000	119.	*
290.	•00	•000	10 18.	*	910. 915.	.00 .00	.000	117. 115.	*
295.	•00	.000	994.	*	920.	•00	.000	113.	*
300.	•00	-000	969.	*	925.	•00	.000	111.	•
305. 310.	•00 •00	•000	944.	*	930.	.00	.000	109.	*
315.	•00	.000 .000	922. 903.	*	935. 940.	•00 •00	.000 .000	107. 105.	*
320.	•00	.000	886.	*	945.	•00	.000	104.	*
33§:	:88	:888	839:	*	358 :	:88	:888	188	*
335. 340.	•00	.000	841: 827.	*		.00			*
345.	.00	.000	813.	÷	965. 970.	•00	.000 .000	97. 95.	*
350.	•00	.000	800.	*	975.	•00	.000	94.	*
355.	•00	•000	787.	*	980.	.00	.000	92.	*

		-								
360.	•00	.000	774.	*	985.	.00	.000	90.	*	
365.	.00	•000	761.	*	990.	.00	.000	89.	•	
370.	.00	.000	748.	*	995.	•00	.000	87.		
375.	.00	.000	735.	*	1000.	.00	.000	86.	*	
380.	.00	.000	722.	*	1005.	.00	.000	85.	*	
385.	•00	.000	709.	*	1010.	.00	.000	83.	*	
390.	.00	.000	696.	*	1015.	.00	.000	82.	*	
395.	•00	•000	683.	*	1020.	.00.	.000	80.	*	
400.	• 00	.000	670.	*	1025.	•00	.000	79.	*	
405.	• 00	• 000	657.	*	1030.	.00	.000	78.	*	
410.	•00	.000	644.	*	1035.	•00	.000	76.	•	
415.	• 00	• 000	630.	*	1040.	.00	.000	75.	*	
420.	•00	.000	618.	*	1045.	•00	.000	74.	*	
425.	• 00	•000	606.	*	1050.	•00	.000	73.	*	
430.	•00	.000	595.	*	1055.	•00	.000	71.	*	
435.	• 00	•000	584.	*	1060.	.00	.000	70.	*	
440.	•00	.000	574.	*	1065.	.00	.000	69.	*	
445.	• 00	.000	564.	*	1070.	.00	•000	68.	*	
450.	.00	.000	554.	*	1075.	•00	•000	67.	*	
455.	•00	.000	545.	*	1080.	•00	.000	66.		
460.	.00	.000	535.	*	1085.	.00	•000	64.	*	
465.	.00	.000	526.	*	1090.	.00	•000	63.	*	
470.	•00	.000	518.	*	1095.	.00	.000	62.	*	
475.	•00	.000	509.	*	1100.	.00	.000	61.	*	
480.	•00	.000	500.	*	1105.	.00	.000	60.	*	
485.	•00	•000	492.	*	1110.	.00	.000	59.	*	
490.	•00	•000	484.	*	1115.	•00	.000	58.	*	
495. 500.	•00	.000	475.	*	1120.	•00	•000	57.	*	
505.	•00	•000	467-	*	1125.		*********	··· 5ክኔ	*	
510.	•00 •00	•000	460.	*	1130.	•00	.000	55.	*	
515.	•00	•000	452.		1135.	•00	•000	54.	*	
520.	•00	•000	444.	*	1140.	•00	.000	53.	*	
525.	•00	•000	437.	*	1145.	•00	•000	53.	*	
530.	•00	•000	430.	*	1150.	•00	.000	52.	*	
535.	•00	.000	422.		1155.	•00	•000	51.	*	
540.	•00	•000	415.	*	1160.	.00	.000	50.	*	
545.		.000	408.	*	1165.	•00	•000	49.	*	
550.	-00	•000	401.	*	1170.	•00	•000	48.	*	
555.	•00	•000	395.	*	1175.	•00	.000	48.	*	
	•00	.000	388.	*	1180.	.00	.000	47.	*	
560.	•00	•000	382.	*	1185.	•00	.000	46.	*	
565.	•00	•000	375.	*	1190.	.00	.000	45.	*	
570.	•00	.000	369.	*	1195.	•00	.000	44.	*	
575.	•00	•000	363.	*	1200.	•00	.000	44.	*	
580.	•00	•000	357.	*	1205.	• 00	.000	43.	*	
585.	• 00	•000	351.	*	1210.	•00	.000	42.	*	
590.	•00	•000	345.	*	1215.	•00	.000	41.	*	
595.	•00	•000	339.	*	1220.	•00	.000	41.	*	
600.	•00	•000	333.	*	1225.	•00	.000	40.	*	
605.	•00	.000	328.	*	1230.	•00	.000	39.	*	
610.	•00	.000	322.	*	1235.	•00	.000	39.	*	
615.	•00	•000	317.	Ŕ	1240.	.00	.000	38.	*	
620.	•00	•000	311.	*	1245.	•00	.000	37.	*	

TOTAL PRECIP. = 3.18 (1-HOUR RAIN = 2.75) EXECESS PRECIP. = 1.751 INCHES VOLUME OF EXCESS PRECIP = 663. ACRE-FEET PEAK Q = 1183. CFS TIME OF PEAK = 225. MIN. INFILT. = 3.00 IN/HR DECAY = .00180 FNINF = .50 IN/HR MAX.PERV.RET. = .35 IN. MAX.IMP.RET. = .10 IN.



Full Section % Area of Shallow Floating

Y= 1.75 fps; Q= 2400 cfs > 2261 cfs ox

$$V = \frac{1.49}{\eta} \frac{7^{2/3}}{3} \frac{5/2}{5}$$

$$n = 0.05$$

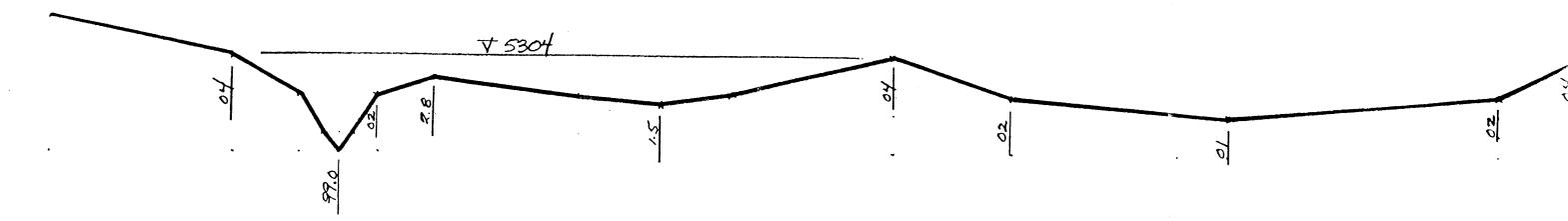
$$5 = 0.003$$

$$Q = 2261 \text{ cfs.}$$

$$Q = \frac{1.40}{770} = 1.48$$

$$V = 2.13 \text{ fps;} Q = \frac{2430 \text{ cfs.}}{2430 \text{ cfs.}} > 2261 \text{ cfs.}$$

SEC A-A



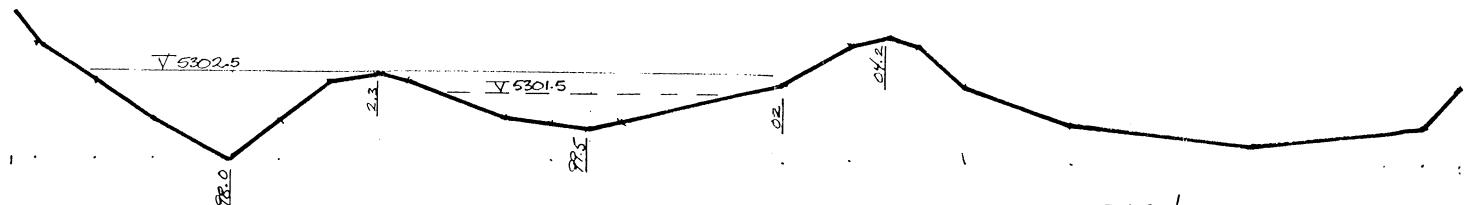
$$V = 1.49 R^{3/3} 5^{1/2}$$

$$1 = 0.05$$

$$5 = 0.003$$

$$6 d = 5' R = \frac{A}{P} = \frac{1/50}{710} \cdot 1.62$$

$$V = 2.26 fps; Q = 2000 cfs > 2261 ds OX$$



11/2in Channel

V= 1.49 R2/3 5/2

 $0.05 \\ 5 = 0.003 \\ P = \frac{4}{315} \cdot \frac{625}{315} \cdot \frac{1.98}{1.98}$

V=2.59 fps; Q=1600 cfs

2261 (1605) 660 spillover Full Section $V = 1.49 R^{2/3} 5^{1/2}$ 1 = 0.05 5:0.003 $ed: 4.5' R = \frac{A}{P} = \frac{1400 R^2}{760} = 1.84$ $V = 2.46 cfs; Q = 3450 \neq 2261 cfs$

Spilouer Section

V= 1.49 R²/3 5 1/2

N=0.05

S=0.003

Q=4.2'

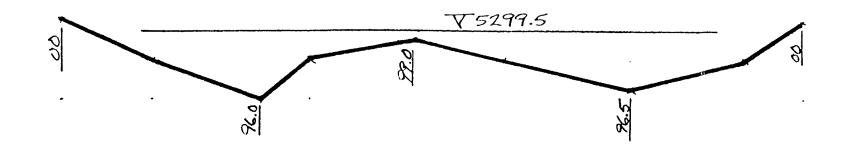
R=4

P=345

1.08

Y=1.72 Sps; Q=600 cfs

SEC C-C



SEC D-D

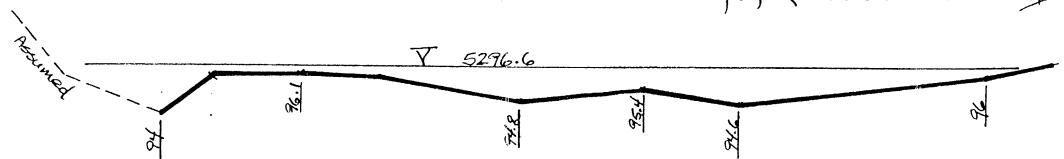
$$V = \frac{1.49}{9} R^{2/3} 5^{1/2}$$

$$1.0.05$$

$$5.0.003$$

$$R \cdot \frac{A}{P} \cdot \frac{1140}{730} = 1.56$$

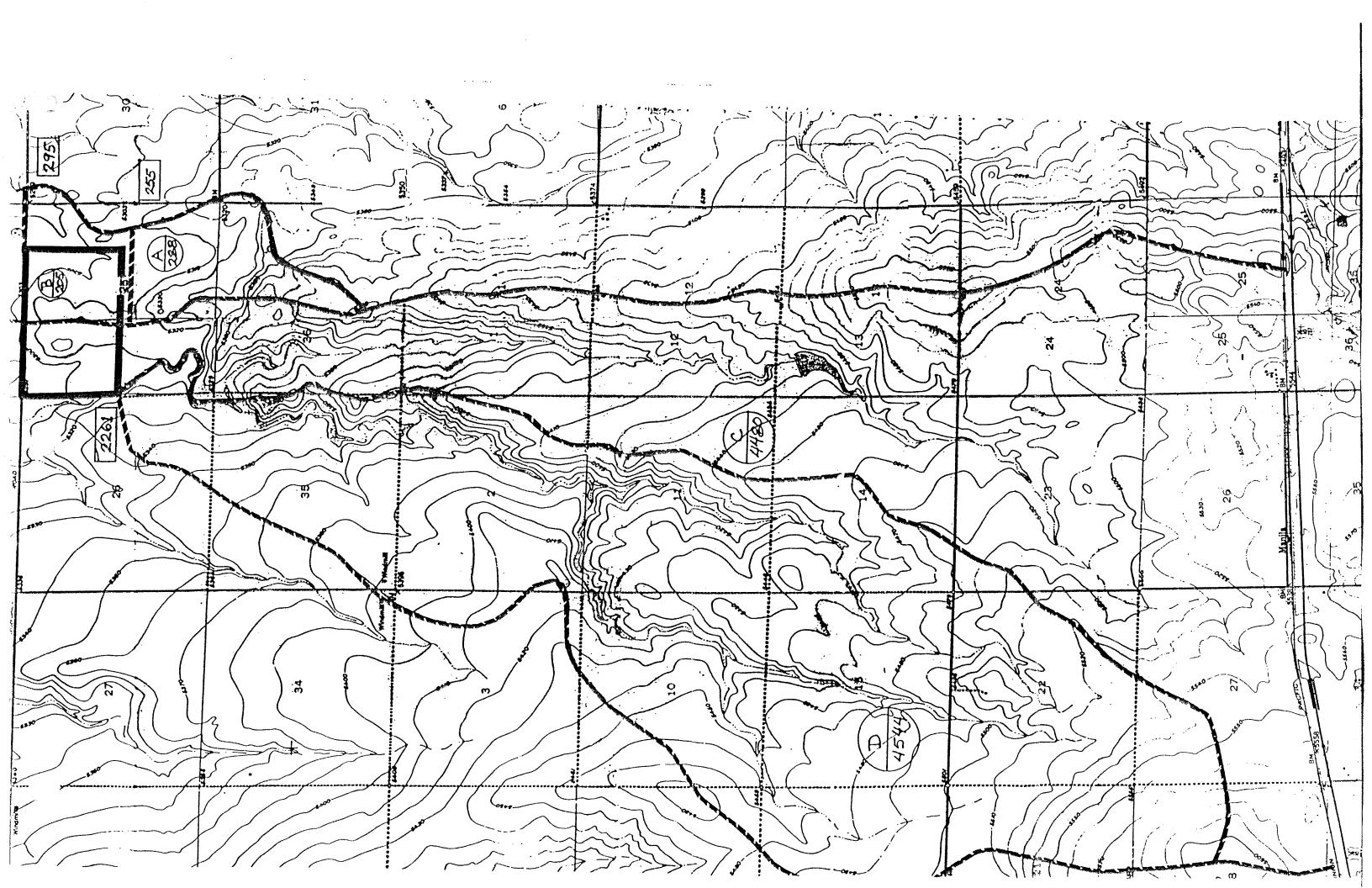
V= 2.20 fpo; Q= 2500 cfs > 2261 OX



SEC E-E

$$N = 0.05$$

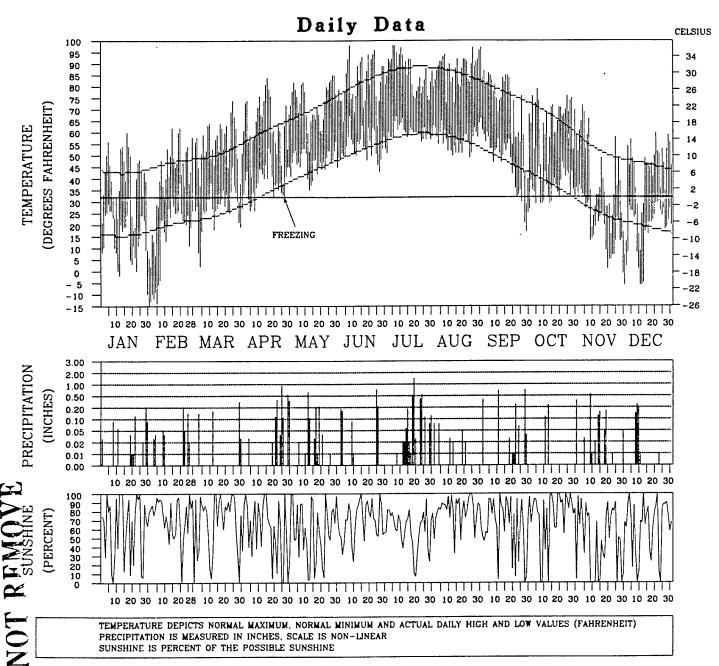
 $S = 0.003$
 $R = \frac{A}{P} = \frac{1250 R^2}{1040} = 1.20$



APPENDIX C - CLIMATOLOGY

1985 LOCAL CLIMATOLOGICAL DATA DENVER,

COLORADO



I CERTIFY THAT THIS IS AN OFFICIAL PUBLICATION OF THE NATIONAL OCEANIC AND ATMOSPHERIC ADMINISTRATION, AND IS COMPILED FROM RECORDS ON FILE AT THE NATIONAL CLIMATIC DATA CENTER, ASHEVILLE, NORTH CAROLINA, 28801

noaa oceanic and oceanic and atmospheric administration

NATIONAL ENVIRONMENTAL SATELLITE, DATA AND INFORMATION SERVICE

NAT LONAL CLIMATIC DATA CENTER ASHEVILLE NORTH CAROLINA NATIONAL CLIMATIC DATA CENTER

METEOROLOGICAL DATA FOR 1985

DENVER, COLORADO

				יאוטע	LIK, CO	DOILAD	,						
LATITUDE: 39°45'N L	ONGITUDE JAN	: 104 °: FEB	MAR	ELEVAT APR	ION: FT MAY	. GRND JUNE	5282 B JULY	ARO 5	287 TI SEP	ME ZONE OCT	: MOUNT	DEC	AN: 23062 YEAR
TEMPERATURE OF: Averages -Daily Maximum -Daily Minimum -Monthly -Monthly Dewpt.	37.5 13.6 25.6 13.3	40.4 14.9 27.7 13.8	54.5 27.1 40.8 19.7	63.8 38.1 51.0 28.4	73.0 46.9 60.0 36.5	82.0 53.9 68.0 40.5	86.3 59.6 73.0 47.2	87.7 57.0 72.4 44.7	71.5 46.1 58.8 38.5	65.1 36.3 50.7 28.1	40.4 19.1 29.8 17.4	41.6 17.2 29.4 12.8	62.0 35.8 48.9 28.4
Extremes -Highest -Date -Lowest -Date	60 17 -15 31	62 20 -14 5	74 25 2 4	84 16 26 8	85 29 35 13	98 8 43 27	98 7 51 2	97 31 49 24	87 2 17 29	79 6 27 1	72 4 -1 30	60 21 -6 12	98 JUL 7 -15 JAN 31
DEGREE DAYS BASE 65 °F: Heating	1215	1041	742	412	167	42	0	1	241	435	1051	1094	6441
Cooling	0	0	0	1	19	137	256	238	63	0	0	0	714
% OF POSSIBLE SUNSHINE	62	73	68	59	59	69	60	80	63	73	48	63	65
AVG. SKY COVER (tenths) Sunrise - Sunset Midnight - Midnight NUMBER OF DAYS: Sunrise to Sunset	5.5 5.3	5.5 4.8	5.1 4.8	6.0 5.7	5.5 5.3	5.3 5.1	6.3 5.8	4.2 4.0	5.4 5.5	3.9 3.8	7.0 6.6	5.5 4.9	5.4 5.1
-Clear -Partly Cloudy -Cloudy	11 9 11	7 13 8	12 7 12	8 8 14	11 10 10	12 9	3 19 9	11 14 6	11 7 12	16 8 7	5 9 16	9 11 11	113 127 125
Precipitation .01 inches or more	10	8	4	9	10	6	15	6	9	3	10	5	95
Snow,Ice pellets 1.0 inches or more	4	3	3	0	0	0	0	0	1	1	5	3	20
Thunderstorms	0	0	0	2	7	8	13	9	4	0	0	0	43
Heavy Fog, visibility /4 mile or less	3	1	0	0	0	0	0	0	0	0	7	0	11
Temperature of -Maximum 90° and above 32° and below	0 10	0 7	0	0	0	8	12 0	15 0	0	. 0	0 10	0 7	35 36
-Minimum 32° and below 0° and below	31 5	27 7	24 0	6	0	0	0	0	5 0	10 0	27 1	31 4	161 17
AVG. STATION PRESS. (mb)	837.5	834.4	833.1	834.4	835.1	837.5	840.2	838.5	837.5	836.8	832.4	837.8	836.1
RELATIVE HUMIDITY (%) Hour 05 Hour 11 Hour 17 Hour 23	73 54 59 75	70 49 52 71	65 38 33 54	64 40 35 55	65 34 35 56	57 31 29 49	62 35 35 55	63 29 26 52	69 42 40 60	63 36 33 59	75 57 60 70	58 46 51 59	65 41 41 60
PRECIPITATION (inches):													
Water Equivalent -Total -Greatest (24 hrs) -Date	0.68 0.29 29-30	0.59 0.26 22-23	0.69 0.34 29	2.61 0.98 24-25	1.33 0.77 12-13	1.46 0.93 25-26	3.71 1.57 18-19	0.28 0.08 4	2.33 0.80 28-29	0.77 0.38 31	1.20 0.57 9	0.66 0.40 8- 9	16.31 1.57 JUL 18-19
Snow,Ice pellets -Total -Greatest (24 hrs) -Date	12.5 5.4 29-30	8.7 3.3 22-23	7.6 3.4 29	0.8 0.7 26	T 13	0.0 0.0	0.0	0.0	8.7 8.7 28-29	1.9 1.9 13	17.0 7.1 9	10.3 6.9 8- 9	67.5 8.7 SEP 28-29
WIND: Resultant -Direction (!!) -Speed (mph)	136 0.4	121 0.4	187 1.8	295 0.7	185 1.7	126 2.0	176 1.6	168 3.1	142 2.0	151 1.3 7.5	036 0.8	160 1.3 7.5	160 1.1 7.9
Average Speed (mph) Fastest Obs. 1 MinDirection (!!) -Speed (mph)	6.6 31 29	7.3 04 25 25	10.3 28 35	8.9 33 41	8.6 30 31 30	8.3 27 31 25	7.9 30 31 4	7.6 35 26 18	7.9 04 30 22	30 32 22	6.7 06 22 5	30 29 17	33 41 APR 3
-Date Peak Gust -Direction (!!) -Speed (mph) -Date	17 NH 41 15	NE 39 25	26 W 52 26	N 53 4	N 45 13	и 46	NW 44	NH 48	N 44	N₩ 41	N 37	H 37 23	N 53 APR 4
							Da	(D					

(!!) See Reference Notes on Page 68 Page 2

NORMALS, MEANS, AND EXTREMES

					DENVE	R, COL	ORADO							
LATITUDE: 39 045 N L	DNG I,	TUDE: 1			_		RND 52		5287		ZONE: M	1		AN: 23062
	(a)	JAN	FEB	MAR	APR	MAY	JUNE	JULY	AUG	SEP	OCT	NOV	DEC	YEAR
TEMPERATURE OF: Normals -Daily Maximum -Daily Minimum -Monthly		43.1 15.9 29.5	46.9 20.2 33.5	51.2 24.7 38.0	61.0 33.7 47.4	70.7 43.6 57.2	81.6 52.4 67.0	88.0 58.7 73.3	85.8 57.0 71.4	77.5 47.7 62.6	66.8 36.9 51.9	52.4 25.1 38.8	46.1 18.9 32.5	64.3 36.2 50.2
Extremes -Record Highest -Year -Record Lowest -Year	51 51	73 1982 -25 1963	76 1963 -30 1936	84 1971 -11 1943	85 1960 -2 1975	96 1942 22 1954	104 1936 30 1951	104 1939 43 1972	101 1938 41 1964	97 1960 17 1985	88 1947 3 1969	79 1941 -8 1950	75 1980 -21 1983	104 JUL 1939 -30 FEB 1936
NORMAL DEGREE DAYS: Heating (base 65°F)		1101	879	837	528	253	74	0	0	135	414	789	1004	6014
Cooling (base 65°F)		0	0	0	0	11	134	261	203	63	В	0	0	680
% OF POSSIBLE SUNSHINE	36	71	71	70	67	64	71	71	73	74	72	64	67	70
MEAN SKY COVER (tenths) Sunrise - Sunset MEAN NUMBER OF DAYS: Sunrise to Sunset	37	5.6	5.8	6.1	6.0	6.2	5.0	4.9	4.9	4.4	4.5	5.4	5.4	5.4
-Clear -Partly Cloudy -Cloudy	51 51 51	10.0 9.2 11.7	8.1 8.8 11.4	7.8 10.3 12.8	6.9 10.7 12.5	6.1 12.1 12.8	9.6 12.3 8.1	9.1 15.9 6.1	10.0 13.8 7.3	13.5 8.9 7.6	13.5 9.0 8.5	10.3 9.4 10.2	10.6 9.8 10.6	115.5 130.2 119.5
Precipitation .01 inches or more	51	5.8	5.7	8.6	8.8	10.6	8.8	9.2	8.6	6.2	5.3	5.3	5.2	88.3
Snow, Ice pellets 1.0 inches or more	51	2.3	2.4	3.7	2.5	0.5	0.0	0.0	0.0	0.3	1.2	2.5	2.3	17.7
Thunderstorms	51	0.0	0.*	0.2	1.5	6.3	9.8	11.1	8.2	3.4	0.9	0.1	0.0	41.4
Heavy Fog Visibility 1/4 mile or less Temperature F	45	1.2	1.6	1.0	0.8	0.5	0.4	0.4	0.6	0.6	0.7	1.2	1.0	10.1
-Maximum 90° and above 32° and below	25 25	0.0 6.9	0.0 4.2	0.0 3.0	0.0 0.5	0.3	5.9 0.0	15.2 0.0	9.3	2.2 0.*	0.0	0.0 2.5	0.0 5.1	32.9 22.6
-Minimum 32° and below 0° and below	25 25	30.0 4.3	26.4 1.7	25.4 0.5	12.0 0.*	1.7 0.0	0.0	0.0	0.0 0.0	1.0 0.0	8.7 0.0	24.7 0.2	29.1 2.7	158.9 9.5
AVG. STATION PRESS.(mb)														
RELATIVE HUMIDITY (%) Hour 05 Hour 11 Hour 11 (Local Time) Hour 23	25 25 25 25 25	64 46 49 63	66 43 43 64	67 42 41 62	67 38 35 58	70 39 38 61	69 37 35 59	68 35 34 • 56	69 36 35 58	68 38 34 59	64 36 35 59	69 44 49 65	64 45 51 63	67 40 40 61
PRECIPITATION (inches): Water Equivalent -Normal -Maximum Monthly -Year -Minimum Monthly -Year -Maximum in 24 hrs -Year	51 51 51	0.51 1.44 1948 0.01 1952 1.02	0.69 1.66 1960 0.01 1970 1.01 1953	1.21 4.56 1983 0.13 1945 2.79 1983	1.81 4.17 1942 0.03 1963 3.25 1967	2.47 7.31 1957 0.06 1974 3.55 1973	1.58 4.69 1967 0.09 1980 3.16 1970	1.93 6.41 1965 0.17 1939 2.42 1965	1.53 5.85 1979 0.06 1960 3.43 1951	1.23 4.67 1961 1 1944 2.44 1936	0.98 4.17 1969 0.05 1962 1.71 1947	0.82 2.97 1946 0.01 1949 1.29 1975	0.55 2.84 1973 0.03 1977 2.00 1982	15.31 7.31 MAY 1957 T SEP 1944 3.55 MAY 1973
Snow,Ice pellets -Maximum Monthly -Year -Maximum in 24 hrs -Year	51 51	23.7 1948 12.4 1962	18.3 1960 9.5 1953	30.5 1983 18.0 1983	28.3 1935 17.3 1957	13.6 1950 10.7 1950	0.3			21.3 1936 19.4 1936	31.2 1969 12.4 1969	39.1 1946 15.9 1983	30.8 1973 23.6 1982	39.1 NOV 1946 23.6 DEC 1982
WIND: Mean Speed (mph)	37	8.7	9.0	9.8	10.2	9.4	8.9	8.4	8.1	8.0	7.9	8.4	8.7	8.8
Prevailing Direction through 1963		s	S	s	S	S	s	s	S	S	S	s	S	s
Fastest Mile -Direction (!!) -Speed (MPH) -Year	31 31	N 53 1976	NH 49 1953	NH 53 1952	NW 56 1960	SE 54 1978		SH 56 1965	N 42 1978	NH 47 1955	NW 45 1958	# 48 1962	NE 51 1953	SW 56 JUL 1965
Peak Gust -Direction (!!) -Speed (mph) -Date	2 2	NH 41 1985	N 51 1984	52 1985	N 53 1985	SE 48 1984		NE 45 1984	NH 48 1985	# 56 1984	NW 41 1985	NW 44 1984	NW 47 1984	56 SEP 1984

PRECIPITATION (inches)

DENVER, COLORADO

EAR	JAN	FEB	MAR	APR	MAY	JUNE	JULY	AUG	SEP	OCT	NOV	DEC	ANNUAL
1956	0.39	0.77	0.89	0.72	2.36	0.44	4.17	1.83	0.01	0.27	1.25	0.62	13.72
1957	0.32	0.73	1.09	4.13	7.31	1.09	1.29	2.03	0.42	2.62	0.49	0.06	21.58
1958	0.73	1.00	1.48	1.73	4.46	1.47	3.50	1.17	1.51	0.37	0.74	0.64	18.80
1959	1.24	1.31	2.85	1.35	3.33	0.44	0.83	0.25	1.82	2.46	0.40	0.26	16.54
1960	0.77	1.66	0.89	2.56	2.27	0.63	1.31	0.06	0.38	2.46	0.49	1.50	14.98
1961	0.07	0.66	2.51	1.06	4.12	1.11	1.60	1.21	4.67	0.77	0.93	0.30	19.01
1962	1.33	1.05	0.52	1.10	0.84	1.52	0.54	0.46	0.19	0.05	0.68	0.17	8.45
1963	0.71	0.21	1.42	0.03	0.68	3.59	0.55	2.52	1.25	0.31	0.45	0.51	12.23
1964	0.26	1.04	1.38	1.25	2.53	0.82	0.72	0.27	0.41	0.18	0.88	0.40	10.14
1965	1.00	1.27	1.20	1.05	1.82	4.14	6.41	1.06	2.58	0.45	0.36	0.53	21.87
1966	0.30	1.28	0.32	1.46	0.34	1.41	1.04	2.06	1.15	0.96	0.32	0.17	10.81
1967	0.84	0.39	0.79	3.95	4.77	4.69	3.25	0.83	0.60	1.13	1.01	1.06	23.31
1968	0.51	0.74	0.85	2.39	0.71	0.50	1.34	2.53	0.59	0.75	0.71	0.51	12.13
1969	0.17	0.43	1.10	1.33	6.12	2.99	1.81	0.79	1.67	4.17	0.62	0.32	21.52
1970	0.10	0.01	1.34	0.97	0.64	3.83	1.67	0.54	2.47	0.88	1.19	0.09	13.73
1971	0.35	0.78	0.53	1.98	1.34	0.23	1.20	0.85	2.85	0.44	0.16	0.25	10.96
1972	0.36	0.44	0.50	3.52	0.49	2.94	0.63	2.71	2.07	0.82	1.69	0.70	16.87
1973	1.31	0.16	1.76	3.73	5.06	0.20	2.47	1.28	2.85	0.47	0.83	2.84	22.96
1974	1.03	0.82	1.32	2.28	0.06	2.01	2.34	0.16	0.98	1.68	1.06	0.29	14.03
1975	0.23	0.37	1.19	1.14	2.80	2.11	2.78	2.00	0.24	0.30	1.88	0.47	15.51
1976	0.19	0.54	1.34	1.27	1.34	0.63	2.31	2.50	1.88	0.93	0.32	0.16	13.41
1977	0.16	0.27	1.24	2.13	0.34	1.02	2.98	1.00	0.10	0.48	0.59	0.03	10.34
1978	0.27	0.27	1.07	1.82	3.46	1.17	0.54	0.26	0.07	1.45	0.50	0.82	11.70
1979	0.34	0.42	1.25	1.41	3.53	2.39	0.81	5.85	0.36	1.28	1.66	1.06	20.36
1980	0.64	0.45	1.15	2.54	2.73	0.09	2.93	1.65	0.63	0.10	0.66	0.10	13.67
1981	0.29	0.35	2.27	1.01	3.76	0.63	0.90	1.16	0.35	0.79	0.42	0.66	12.59
1982	0.32	0.09	0.18	0.34	3.48	2.26	0.92	1.16	1.38	1.51	0.47	2.34	14.45
1983	0.15	0.07	4.56	2.10	3.62	2.65	1.75	1.51	0.13	0.39	2.63	0.63	20.19
1984	0.18	0.81	1.19	2.42	0.65	1.26	2.11	3.20	0.47	3.47	0.27	0.46	16.49
1985	0.68	0.59	0.69	2.61	1.33	1.46	3.71	0.28	2.33	0.77	1.20	0.66	16.31
Record Mean	0.47	0.57	1.13	1.99 Se	2.40 e Refer	1.49 ence No Page	1.71 tes on 4A	1.43 Page 6B	1.10	1.02	0.68	0.63	14.62

AVERAGE TEMPERATURE (deg. F)

DENVER, COLORADO

- EAR	JAN	FEB	MAR	APR	MAY	JUNE	JULY	AUG	SEP	OCT	NOV	DEC	ANNUAL
1956	34.0	27.7	40.1	45.5	60.9	73.4	72.2	69.7	65.5	55.9	37.2	35.7	51.5
1957	25.8	40.7	39.1	41.4	53.9	65.9	73.5	72.6	61.4	51.4	36.8	39.6	50.2
1958	32.9	37.4	32.8	44.6	61.7	68.1	70.3	73.6	64.4	53.9	40.6	35.8	51.4
1959	30.0	30.2	37.6	45.6	56.2	70.9	72.6	73.0	61.1	48.1	37.6	36.5	50.0
#1960	27.6	24.8	38.1	50.5	57.2	68.3	73.2	73.4	65.0	52.0	39.5	26.5	49.7
1961	31.7	35.2	38.9	46.0	55.7	66.1	71.5	72.2	56.3	50.0	34.7	27.7	48.9
1962	19.5	29.9	34.6	50.3	59.8	65.5	72.9	72.5	62.4	53.4	41.3	33.8	49.7
1963	19.1	37.3	37.3	50.0	60.9	66.7	74.8	68.7	65.5	57.9	41.7	28.5	50.8
1964	30.6	27.4	33.0	46.6	58.8	65.0	75.8	70.4	62.5	52.7	40.0	33.2	49.7
1965	35.0	27.4	29.0	51.2	57.1	63.9	72.7	70.2	55.7	55.1	43.3	35.0	49.6
1966	28.6	28.4	42.5	44.6	58.7	64.6	76.9	70.8	65.0	52.2	41.5	31.9	50.5
1967	34.0	35.1	42.9	48.2	52.6	60.6	69.1	68.2	62.1	52.5	40.5	26.5	49.4
1968	29.7	34.2	40.6	43.0	53.9	67.8	71.7	68.1	60.9	51.9	35.7	28.9	48.9
1969	35.0	35.4	32.2	52.2	59.3	61.5	74.7	73.9	64.5	39.0	39.1	32.5	49.9
1970	30.6	38.6	33.5	43.7	58.8	65.2	72.0	73.9	59.5	45.9	39.1	33.3	49.5
1971	32.1	30.6	38.5	47.8	54.2	69.0	70.6	72.8	57.5	49.4	39.1	31.9	49.5
1972	30.5	36.2	44.8	48.5	57.0	68.3	70.2	71.0	62.1	52.1	32.9	24.9	49.9
1973	27.3	35.5	39.9	43.2	55.6	67.5	71.0	73.5	59.9	54.5	39.5	31.6	49.9
1974	23.7	35.2	43.2	47.9	61.6	68.4	74.7	69.5	59.4	52.4	38.0	31.2	50.5
1975	31.7	30.6	37.3	44.1	54.3	64.3	72.7	70.8	59.5	53.2	36.8	37.5	49.4
1976	32.3	39.3	37.1	49.2	56.7	66.3	75.3	70.2	61.8	48.4	39.5	35.5	51.0
1977	29.2	38.0	39.9	51.1	60.7	71.9	74.3	70.2	66.6	53.3	40.3	35.1	52.5
1978	25.8	31.4	43.3	50.3	54.4	66.9	74.7	69.6	65.0	53.1	37.8	24.6	49.7
1979	18.0	34.2	40.5	49.1	54.8	65.8	73.7	69.5	66.3	53.8	33.3	34.5	49.5
1980	26.0	34.5	38.0	47.7	57.1	71.9	76.4	73.2	65.8	52.4	41.9	41.2	52.2
1981 1982 1983 1984 1985	37.3 30.3 31.9 27.3 25.6	36.2 32.0 36.6 34.1 27.7	41.2 41.1 36.2 37.2 40.8	56.4 47.4 41.0 42.3 51.0	57.1 55.1 51.4 60.0	70.4 63.1 62.8 66.5 68.0	75.9 72.7 73.3 74.9 73.0	72.0 73.1 74.4 71.8 72.4	68.2 61.7 64.9 60.7 58.8	52.6 49.0 52.7 44.8 50.7	45.9 35.7 37.0 39.7 29.8	35.8 30.9 17.5 32.8 29.4	54.1 49.3 48.3 49.3 48.9
Record Mean Max Min	30.0 42.7 17.3	32.9 45.4 20.4	38.7 51.3 26.1	47.5 60.2 34.8 Se	56.8 69.5 44.0 e Refer	66.7 80.6 52.8 ence No	72.8 86.6 58.9 tes on	71.3 85.0 57.7 Page 6B	62.7 76.9 48.5	51.5 65.4 37.6	39.5 52.5 26.5	32.3 44.9 19.6	50.2 63.4 37.0
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DENVER, COLORADO

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14 0 6 0 6	0 19 7 16 7	273 112 29 123 296	9 5 5 5 2 7 5 2 7 0 3 3 3 3 3	903 703 690 743 645	1150 961 1125 981 924	1411 1417 1059 921 1122	976 768 1082 1044 1017	934 848 982 1108 691	437 442 545 411 604	175 156 235 204	72 50 72 63 82	6803 5828 6056 6030 5900
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See Reference Notes on Page 6B. Page 5A

COOLING DEGREE DAYS Base 65 deg. F

DENVER, COLORADO

See Reference Notes on Page 6B. Page 5B

SNOWFALL (inches)

DENVER, COLORADO

SEASON	JULY	AUG	SEP	OCT	NOV	DEC	JAN	FEB	MAD	400			
1956-57	0.0	0.0	0.0	0.6	21.3	6.3	5.3		MAR	APR	MAY	JUNE	TOTAL
1957-58 1958-59	0.0	0.0	Ţ T	3.9 2.6	3.0	0.8	8.9	1.6	8.9 14.4	25.5 14.1	8.8 0.0	0.0	78.3 57.1
1959-60	0.0	0.0	12.9	11.8	9.7 5.3	7.7 2.7	17.4 10.7	17.5 18.3	26.8 9.0	17.6 9.3	Ť Ť	0.0	99.3
1960-61 1961-62	0.0	0.0	0.0	4.6	5.1	17.8	1.0	7.9	29.2	8.6	6.4	0.0	80.0 80.6
1962-63	0.0	0.0	5.8 0.7	6.2 0.0	11.4 5.0	3.8 1.2	17.2	11.3	6.8	10.0	0.0	0.0	72.5
1963-64 1964-65	0.0	0.0	0.0	1.1	3.5	5.9	9.1 2.6	2.1 12.7	18.0 18.4	0.2 12.1	0.0	0.0	36.3
1965-66	0.0	0.0	0.0	0.0	6.0 5.5	4.4 5.6	13.2 3.6	17.1	14.9	0.3	т (0.0	57.3 55.9
1966-67	0.0	0.0	т	8.3	3.0	1.9	9.9	14.6 4.4	2.8 6.6	6.4	2.9	0.0	46.9
1967-68 1968-69	0.0	0.0	0.0	1.7	9.4	13.1	3.0	7.3	9.2	3.6 15.1	3.0	0.0	40.7 58.8
1969-70 1970-71	0.0	0.0	0.0	31.2	5.8 5.1	6.9 3.1	2.8 0.9	4.2 0.3	13.2	4.7	0.0	0.0	33.3
1971-72	0.0	0.0	4.6	5.9	9.2	0.9	8.6	. 11.9	9.6	6.0	T	0.0	65.8 56.7
1972-73	0.0	0.0	17.2	3.1 9.7	1.4	8.4	10.9	9.1	7.1	17.2	0.0	0.0	74.4
1973-74 1974-75	0.0	0.0	0.0	2.3	9.3	30.8	12.1	3.0	15.1 12.8	24.8	1.0	0,0	94.9 91.5
1975-76	0.0	0.0	1.8	1.0	11.9	2.1	3.6	4.0	14.3	10.9	6.1	0.0	55.7
1976-77 1977-78	0.0	0.0	0.0	7.2	4.5	3.1	2.4	3.1	18.7 9.6	1.2	0.0	0.0	54.7
1978-79	0.0	0.0	0,0	3.3	4.1	0.7	5.5	6.2	8.6	4.6	0.0	0.0	34.6 46.5
1979-80 1980-81	0.0	0.0	0.0	2.7	6.9	14.2 16.5	9.1	5.8 9.6	18.2	8.1	8.2	0.0	73.2
1981-82	0.0	0.0	0.0	1.5	7.1	1.2	4.1	4.3	24.0	2.9	†	0.0	85.5 45.1
1982-83	0.0	0.0	0.0	2.8	3.3	9.9	4.8	1.8	2.1	2.0	Т	0.0	26.7
1983-84 1984-85	0.0	0.0	T	T	29.3	11.5	3.4	0.8	30.5	11.3	7.6 T	0.0	81.6
1985-86	0.0	0.0	5.2 8.7	13.1	2.3	5.0	12.5	8.7	7.6	0.8	Ť	0.0	80.9 55.2
Record Mean													
Hean	0.0	0.0	1.7	3.7	8,3	7.1	7.8	7.4	13.0	9.1	1.8	т	60.0
				see	Referen	ce Note Page (s on Pa	ge 6B.		·		′ '	55.0
						_							

REFERENCE NOTES

DENVER, COLORADO

GENERAL
T - TRACE AMOUNT.
BLANK ENTRIES DENOTE MISSING/UNREPORTED DATA.
* INDICATES A STATION OR INSTRUMENT RELOCATION.
SEE STATION LOCATION TABLE ON PAGE 8.

SPECIFIC
PAGE 2
PM - INCLUDES LAST DAY OF PREVIOUS MONTH

PAGE 3

(a) - LENGTH OF RECORD IN YEARS, ALTHOUGH
INDIVIDUAL MONTHS HAY BE MISSING.

* LESS THAN .05
NORMALS - BASED ON THE 1951-1980 RECORD PERIOD.
EXTREMES - DATES ARE THE MOST RECENT OCCURRENCE
HIND DIR. NUMERALS SHOM TENS OF DEGRESS CLOCKHISE
FROM TRUE NORTH. "OO" INDICATES CALM.
RESULTANT DIRECTIONS ARE GIVEN TO HHOLE DEGREES.

EXCEPTIONS

PAGE 3
1. FASTEST MILE WINDS ARE THROUGH AUGUST 1981.
PAGES 4A, 4B, 6A
RECORD HEANS ARE THROUGH THE CURRENT YEAR,
BEGINNING IN 1872 FOR TEMPERATURE
1872 FOR PRECIPITATION
1935 FOR SNOWFALL

DENVER, COLORADO

Denver enjoys the invigorating climate that prevails over much of the central Rocky Mountain region, without the extremely cold mornings of the high elevations during winter, or the hot afternoons of summer at lower altitudes. Extremely warm or cold weather in Denver is usually of short duration.

Situated a long distance from any moisture source, and separated from the Pacific Ocean by several high mountain barriers, Denver enjoys low relative humidity, light precipitation, and abundant sunshine.

Air masses from four different sources influence Denver weather. These include arctic air from Canada and Alaska, warm, moist air from the Gulf of Mexico, warm, dry air from Mexico and the southwestern deserts, and Pacific air modified by its passage over mountains to the west.

In winter, the high altitude and mountains to the west combine to moderate temperatures in Denver. Invasions of cold air from the north, intensified by the high altitude, can be abrupt and severe. However, many of the cold air masses that spread southward out of Canada never reach the altitude of Denver, but move off over the lower plains to the east. Surges of air from the west are moderated in their descent down the east face of the Rockies, and reach Denver in the form of chinook winds that often raise temperatures into the 60s, even in midwinter.

In spring, polar air often collides with warm, moist air from the Gulf of Mexico and these collisions result in frequent, rapid and drastic weather changes. Spring is the cloudiest, windiest, and wettest season in the city. Much of the precipitation falls as snow, especially in March and early April. Stormy periods are interspersed with stretches of mild, sunny weather that quickly melt previous snow cover.

Summer precipitation falls mainly from scattered thunderstorms during the afternoon and evening. Mornings are usually clear and sunny, with clouds forming during early afternoon to cut off the sunshine at what would otherwise be the hottest part of the day. Severe thunderstorms, with large hail and heavy rain occasionally occur in the city, but these conditions are more common on the plains to the east.

Autumn is the most pleasant season. Few thunderstorms occur and invasions of cold air are infrequent. As a result, there is more sunshine and less severe weather than at any other time of the year.

Based on the 1951-1980 period, the average first occurrence of 32 degrees Fahrenheit in the fall is October 8 and the average last occurrence in the spring is May 3.

)								1	levation	on abo						• Туре
7						See			<u>.</u>	G	round				-	M = AMOS T = AUTOB
Location	Occupied from	Occupied to	Atriline distance and direction from previoue location	Latitude North	Longitude West	Ground at tem- perature site	Wind instruments	Extreme thermometers	Psychrometer	Sunshine Switch	Tipping bucket rain gage	Weighing rain gage	8" rain gege	Hygrothermometer	Automatic Observir	Remarks
COOPERATIVE													İ			
One or more locations	11/1859	12/1873	NA													Voluntary observers, broken record.
CITY																# - Estimated.
16th (formerly G Street) & Larimer Streets	11/10/71	3/15/73	NA		105° 00'	5177	51	#20	#20				48			- Estimateo.
Woodward Building on Market (formerly Holiday Street) between 15th & 16th Streets	3/15/73	11/30/75	400' WNW	39° 45'	105° 00'	5212	71	37	36				52			
McClintock Block 16th Street	11/30/75	7/1/77	350' ESE	39° 45'	105° 00'	5214	#70	#32	#32				# 50			# - Estimated.
Broadwell Block on Larimer Street	7/1/77	6/13/81	200' ENE	39° 45'	105° 00'	5214	80	45	44				60			
Tabor Block, 16th & Larimer Streets	6/13/81	12/1/87	200' WSW	39° 45'	105° 00'	5204	109	73	72				86			
Patterson & Thomas Block 17th & Curtis Streets	12/1/87	5/1/91	1100' ESE	39° 45'	105° 00'	5218	103	86	86				79			
Club Building 1700 Block on Arapahoe Street	5/1/91	10/1/95	375' N	39° 45'	105° 00'	5229	121	108	107		97		97			
U. S. Post Office 16th & Arapahoe Streets	10/1/95	12/8/04	600' SW	39° 45	105° 00'	5214	151	83 a79	83 479		74		74			a - Effective 6/13/96.
Boston Building 17th & Champa Streets	12/8/04	1/29/16	800' E	39° 45	105° 00'	5219	136 Ъ172	129	128		119		119			b - Effective 3/1/10.
New Post Office Building 19th & Stout Streets	1/29/16	Present	1000' ENE	39° 45'	105° 00'	5221	113 c	106	108	NA	98 c	NA d98	98 c	NA	NA	c - Removed 4/1/50. d - Installed 4/1/50.
Alrport								1								
Administration Building Stapleton Airfield	9/15/31	6/25/47	NA NA		104° 53'	5292	59	46	46			5	42			e - Installed 3/31/50.
WB-FAA Building Stapleton Airfield †	6/25/47	5/7/69	0.3 mi.NW	39° 46	104° 53'	5292 15283	72 g20	6	6	NA el9 f40	4	6	5	h5	N.A	f - Effective 2/1/57. g - Effective 7/8/60. Fastest mile data from 40' prior to 7/12/60.
† Stapleton Int'l AP (Effective 10/1/64)																h - Commissioned 5200' ESE of therm- mometer site 8/1/60. i - Effective 8/1/60.
W. B. Forecast Office †† Stapleton International Airport †† Weather Service Fost Office (Effective 1970)	5/7/69	1/20/82	1.7 mi.ESE	39° 45	104° 52'	5283	j20	5	5	22	4	4	4	35	NA	j - Not moved 5/7/69.
Weather Service Fcst Office Stapleton International Airport	1/20/82	Present	1 mi. N	39° 46	104° 52'	5282	33	5	5	25	4	4	4	5	NA 	;

SUBSCRIPTION:
Price and ordering information available through: National Climatic Data Center, Federal Building, Asheville, North Carolina 28801

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CLIMATOGRAPHY OF THE UNITED STATES NO. 82 - 5

DECENNIAL CENSUS OF UNITED STATES CLIMATE—

SUMMARY OF HOURLY OBSERVATIONS

105th Meridian Time Zone

DENVER, COLORADO

Stapleton Airfield

1951 - 1960 - 1960 - 1960 - 1960 - 1960 Washington, D.C.: 1963 Reprinted February 1974

PREFACE AND EXPLANATION OF TABLES

This summary of surface weather data is one of a series prepared under the Decennial Census of United States Climate, 1960 program. Similar summaries are being published for most Weather Bureau stations. All data are based on monthly data published in Local Climatological Data Supplements for all or part of the period 1951 - 1960. Where the full 10 year period is not covered by the monthly data, summaries are based on the period 1956 - 1960.

This series supersedes the series entitled "Climatography of the United States No. 30-Summary of Hourly Observations", which was published in 1956. It differs from this earlier information in that a longer, more representative period is used for summarization

wherever possible. Comparability between stations is improved in the Decennial Census series by the use of identical 10-year and 5-year periods.

The tables in this pamphlet are similar to Tables A through E in the monthly Local Climatological Data Supplements except that Tables B, D, and E give percentage frequencies instead of total occurrences. In these tables all hourly observations are represented unless otherwise specified. The total number of observations used is indicated on each page under the month name. In the percentage tables "+" indicates more than 0 but less than 0, 5 in Table E and 0, 05 in Tables B and D. Values are not adjusted to make their sums exactly equal to column or row totals.

STATION LOCATION

DENVER, COLORADO STAPLETON AIRFIELD

		Remarks	(a) Mean Bourly Speed and Prevailing Direction from 72' above ground	atop the FAA lower prior to 7/8/60. Fastest Mile from 40' above ground at listed location prior to 7/12/60.	Hygrothermometer installed at remote site 5200' ESE of listed	location 8/1/60. (b) Tempera-	shown in this publication are	from 6' above ground at listed location prior to 8/1/60.		
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ı,		epsp nist pnidpieW	n	<u></u>						
•(fee	Ground	Tipping bucket rain gage	4	4						
Elevation above (feet)	ট	Telepeychrometer		ĸ						
Slevatic		Psychrometer	9	6 b						
		resemonment emeriza	9	6 9						-
		stasmunteni baiW	40	72 20a						
	Sea level	Actual barometer elevation (H _a)	5298	5298						
	S	Ground	5292	5292						
		Longitude	104°53'₩	104°53°W						
		Latinde	39°46'N	39°46'N						
		Airline distance and direction from previous location	6/25/47 8/1/60 1/4 MI NW	PRESENT NO CHANGE						
		Occupied to	8/1/60	PRESENT						
		mon'i beiqu∞oO	6/25/47	8/1/60						
		Location	WB-FAA BLDG MODIFICA- TION CTR STAPLETON AIR- FIELD							

LOCATION AND TOPOGRAPHY:

The airport is located at the northeast edge of Denver on slightly higher ground than most of the city. Denver is on the Great plains about 40 miles east of the main range of the Rocky Mountains (10,000 to 14,000 feet above sea level) and 18 miles east of the foothills (6000 to 8000 feet elevation). These ranges run almost due north and south. About 40 miles south of Denver is the Palmer Lake Divide, a connecting spur that extends due east of the main range. Its alltitude varies from 8600 feet where it joins the Rockies to about 4000 feet the Ransas line. The South Platte river flows from the southwest to the northeast about 5 miles west and 2-1/2 miles north of the airport.

SMOKE SOURCES:

Smoke is important only during the colder half of the year. At this time some smoke from the city is carried over the alroport on a light west wind (Southwest to northwest) but rarely limits visibility to less than three

DENTER, COLORADO Stapleton Airfield

2

7440 Obs.

PERCENTAGE FRECITENCES

west and 2-1/2 miles north of the airport.

DEN. CA. COLORADO Stapleton Airfield

TEMPERATURE AND WIND SPEED-RELATIVE HUMIDITY OCCURRENCES.

TXNUART 7440 Obs.

PERCENTAGE FREQUENCIES OF WIND DIRECTION AND SPEED:

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PERCENTAGE FREQUENCIES OF CEILING-VISIBILITY:

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PERCENTAGE FREQUENCIES OF SKY COVER, WIND, AND RELATIVE HUMIDITY:

**[43**]

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6792 Obs.

PERCENTAGE FREQUENCIES OF WIND DIRECTION AND SPEED:

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HOURLY OBSERVATIONS OF WIND SPEED IN MALES FOR HOUSE.

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DEN CLORADO Sta Airfield

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PERCENTAGE FREQUENCIES OF CEILING-VISIBILITY:

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PERCENTAGE FREQUENCIES OF WIND DIRECTION AND SPEED:

HOURLY ORSERVATIONS OF WIND SPEED IN MEET PRI HOUR!

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DENVER, COLORADO Stapleton Airfield

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TEMPERATURE AND WIND SPEED-RELATIVE HUMIDITY OCCURRENCES.

APRIL 7200 Obs.

PERCENTAGE FREQUENCIES OF WIND DIRECTION AND SPEED:

HOURTY OBSERVATIONS OF WIND SPEED of winds are soon.

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PERCENTAGE FREQUENCIES OF CEILING-VISIBILITY:

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MAT 7440 Obs.

PERCENTAGE FREQUENCIES OF WIND DIRECTION AND SPEED:

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HOURLY DESERVATIONS OF WIND SPEED R R 16 - 56 - 51 - 51 - 51

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DENVER, COLORADO Stapleton Airfield

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PERCENTAGE FREQUENCIES OF CEILING-VISIBILITY:

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JUNE 7200 Obs.

PERCENTAGE FREQUENCIES OF WIND DIRECTION AND SPEED:

HOURLY DESERVATIONS OF WIND SPEED

TEMPERATURE AND WIND SPEED-RELATIVE HUMIDITY OCCURRENCES.

DENVE ORADO Stapletes Airfield

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DENVER, COLORADO Stapleton Airfleld

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PERCENTAGE FREQUENCIES OF SKY COVER, WIND, AND RELATIVE HUMIDITY: M

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7400 Obs.

PERCENTAGE FREQUENCIES OF WIND DIRECTION AND SPEED;

HOURLY OBSERVATIONS OF WIND SPEED IN MIND SPEED

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DENVER, COLORADO Stapleton Airfield

AUGUST 7440 Obs.

PERCENTAGE FREQUENCIES OF WIND DIRECTION AND SPEED:

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PERCENTAGE FREQUENCIES OF SKY COVER, WIND, AND RELATIVE HUMIDITY:

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DENVER, COLORADO Stapleton Airfield

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SEPTEMBER 7200 Obs.

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HOURLY OSCIEVATIONS OF WIND SPEED

PERCENTAGE FREQUENCIES OF WIND DIRECTION AND SPEED:

DENVER, COLORADO Stapleton Airfield

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PERCENTAGE FREQUENCIES OF CEILING-VISIBILITY:

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PERCENTAGE FREQUENCIES OF SKY COVER, WIND, AND RELATIVE HUMIDITY:

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NOVEMBER 7200 Obs.

DENVER, COLORADO Stapleton Airfield TEMPERATURE AND WIND SPEED-RELATIVE HUMIDITY OCCURRENCES.

OCTOBER 7440 Obs.

PERCENTAGE FREQUENCIES OF WIND DIRECTION AND SPEED:

HOURY OBSERVATIONS OF WIND SPEED

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PERCENTAGE FREQUENCIES OF CEILING-VISIBILITY:

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NOVEMBER 7200 Obs.

PERCENTAGE FREQUENCIES OF WIND DIRECTION AND SPEED:

HOURLY OBSERVATIONS OF WIND SPEED IN MEET THE HOUSE

DENVER, COLORADO Stapleton Airfield

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PERCENTAGE FREQUENCIES OF CEILING-VISIBILITY:

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PERCENTAGE FREQUENCIES OF SKY COVER, WIND, AND RELATIVE HUMIDITY:

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PERCENTAGE FREQUENCIES OF WIND DIRECTION AND SPEED.

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In Tables A and C, occurrences are for the average year (10-year total divided by 10). Values are rounded to the nearest whole number, but not adjusted to make their sums exactly equal to column or row totals. "+" indicates more than 0 but less than 0.5.

PERCENTAGE FREQUENCIES OF CEILING-VISIBILITY:

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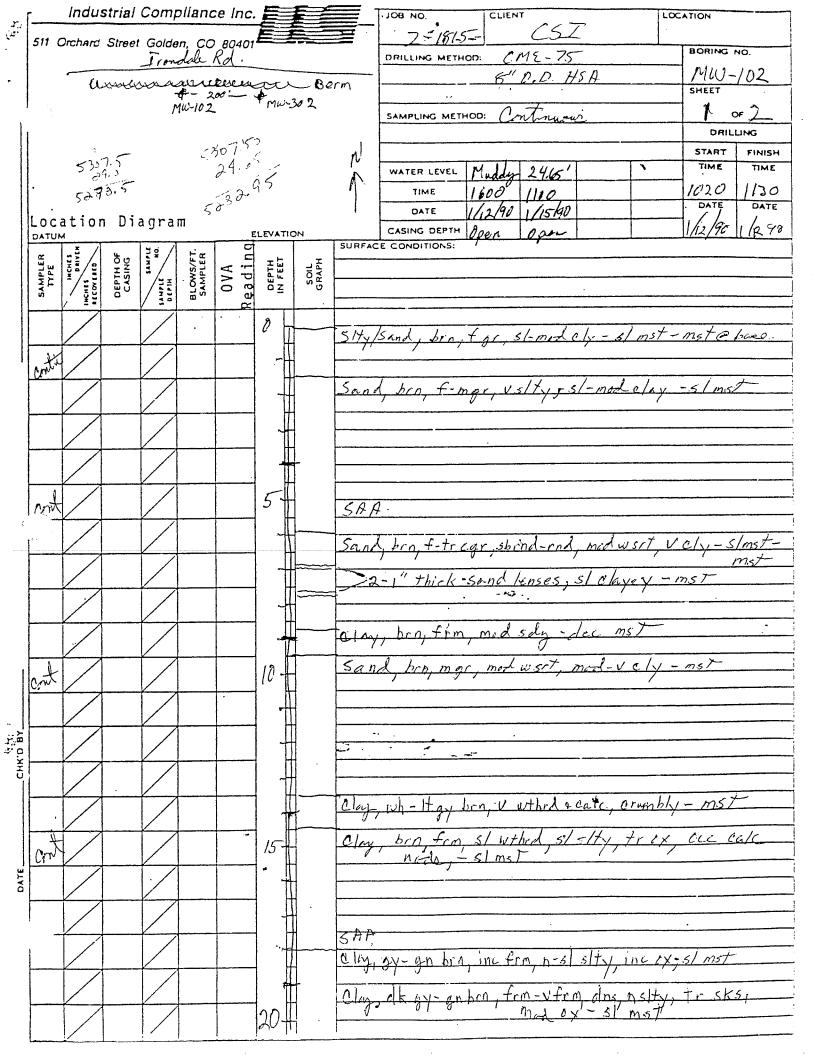
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APPENDIX D - ORIGINAL FIELD DATA AND BORING LOGS

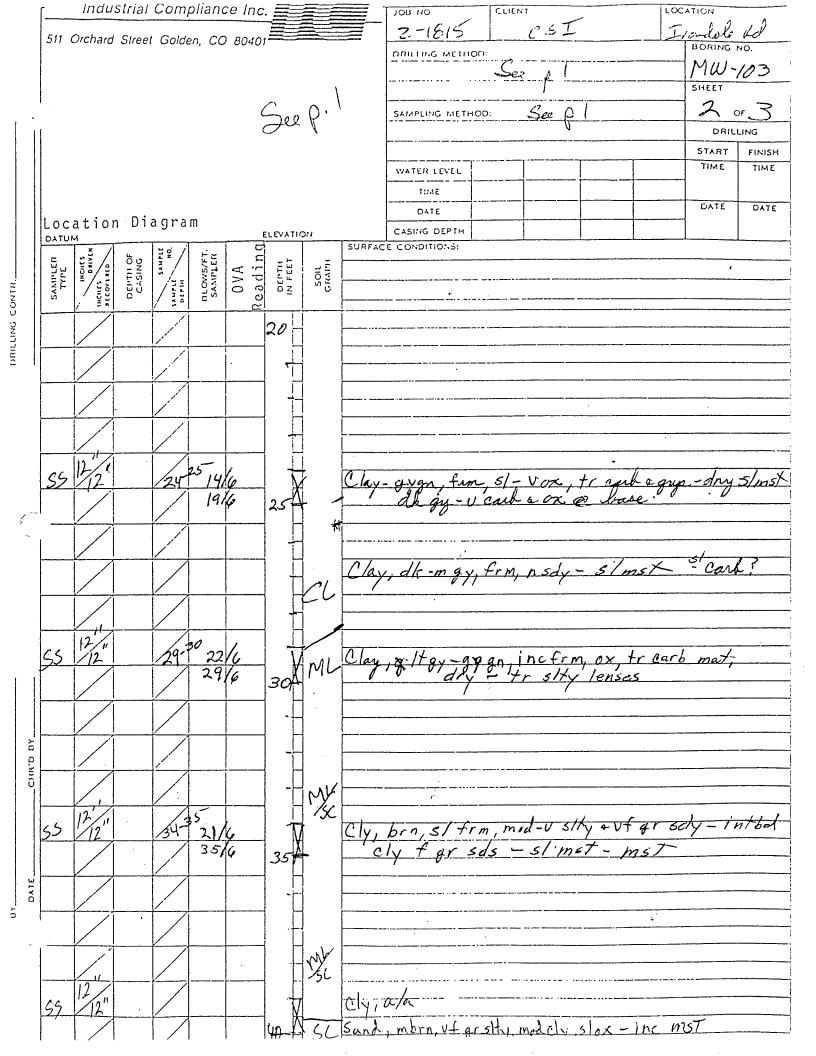
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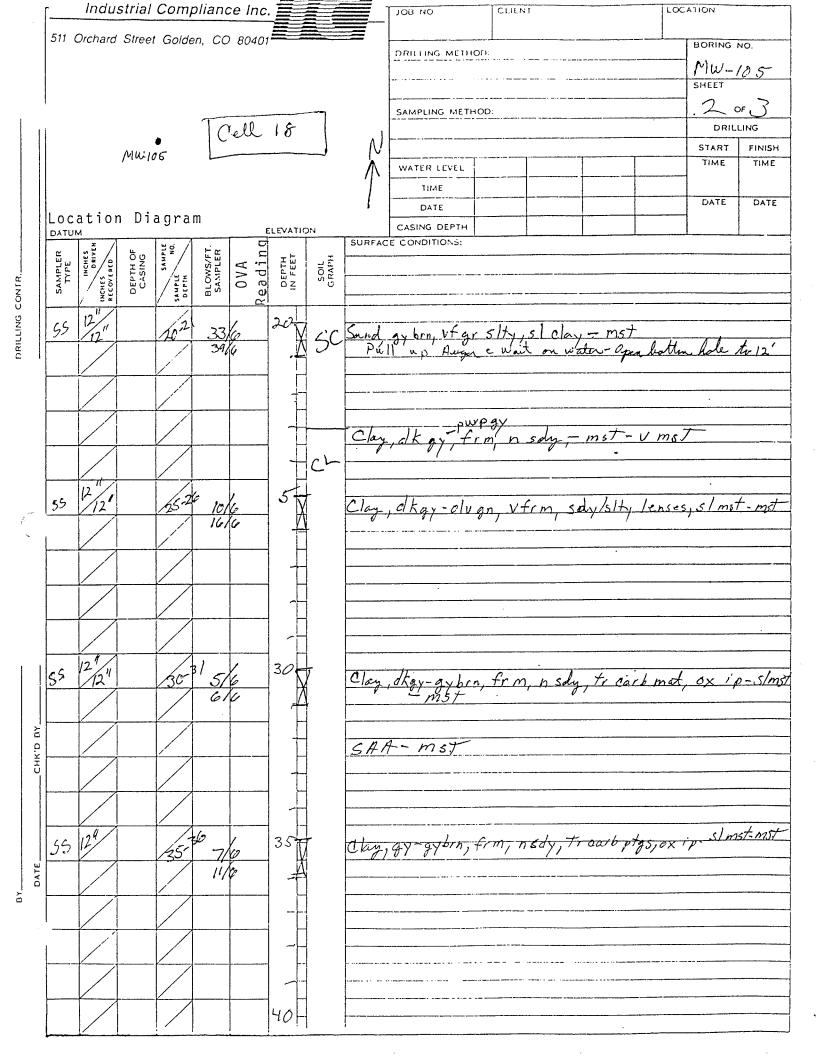
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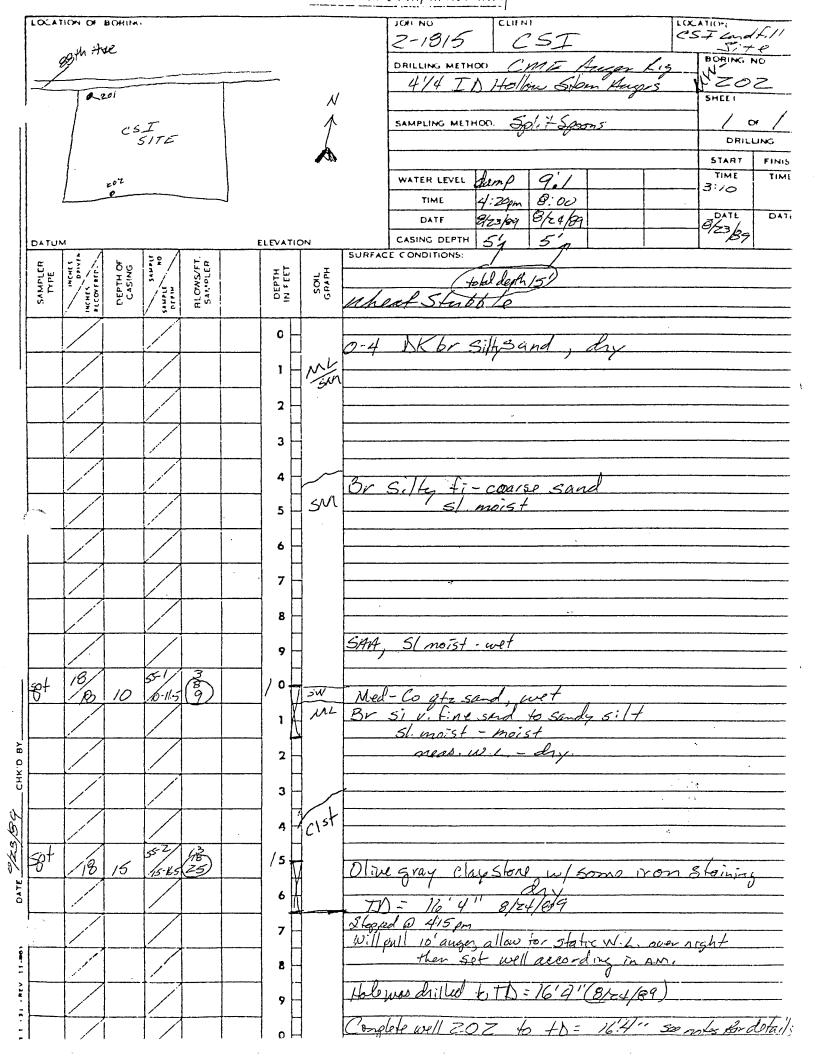
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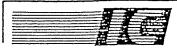
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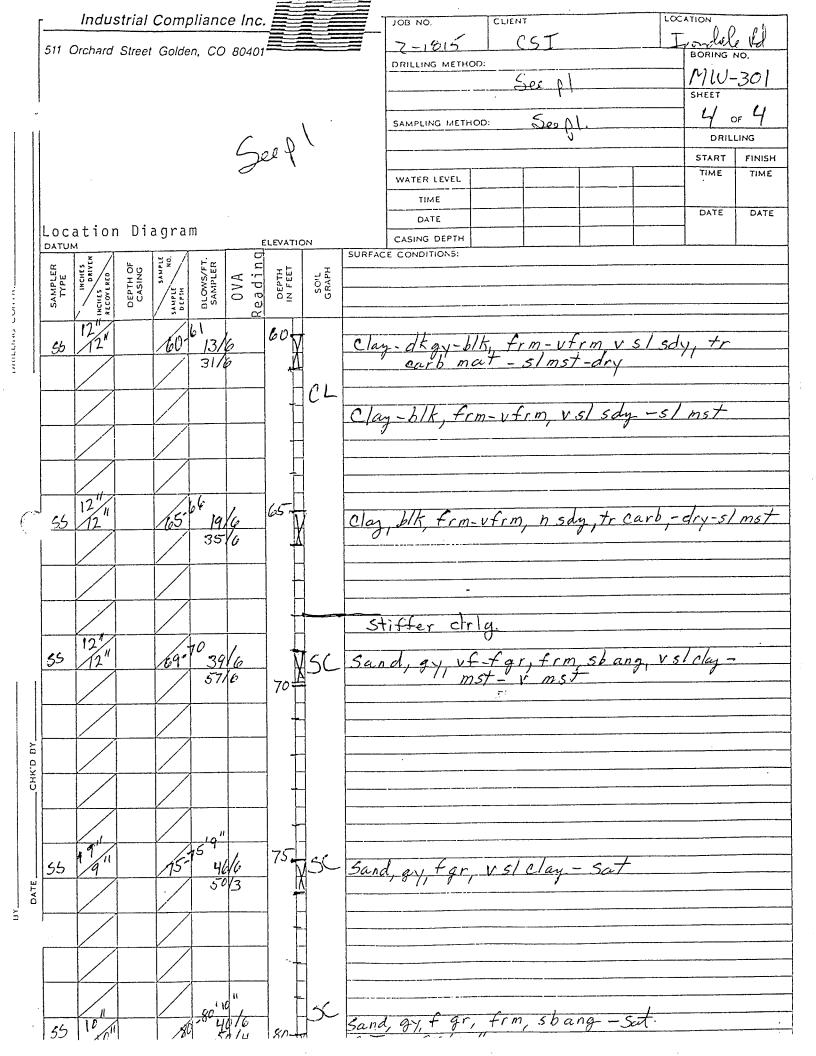
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PROJECT NAME: CSI
PROJECT NUMBER: 2-2502
PROJECT LOCATION: BENNETH CO

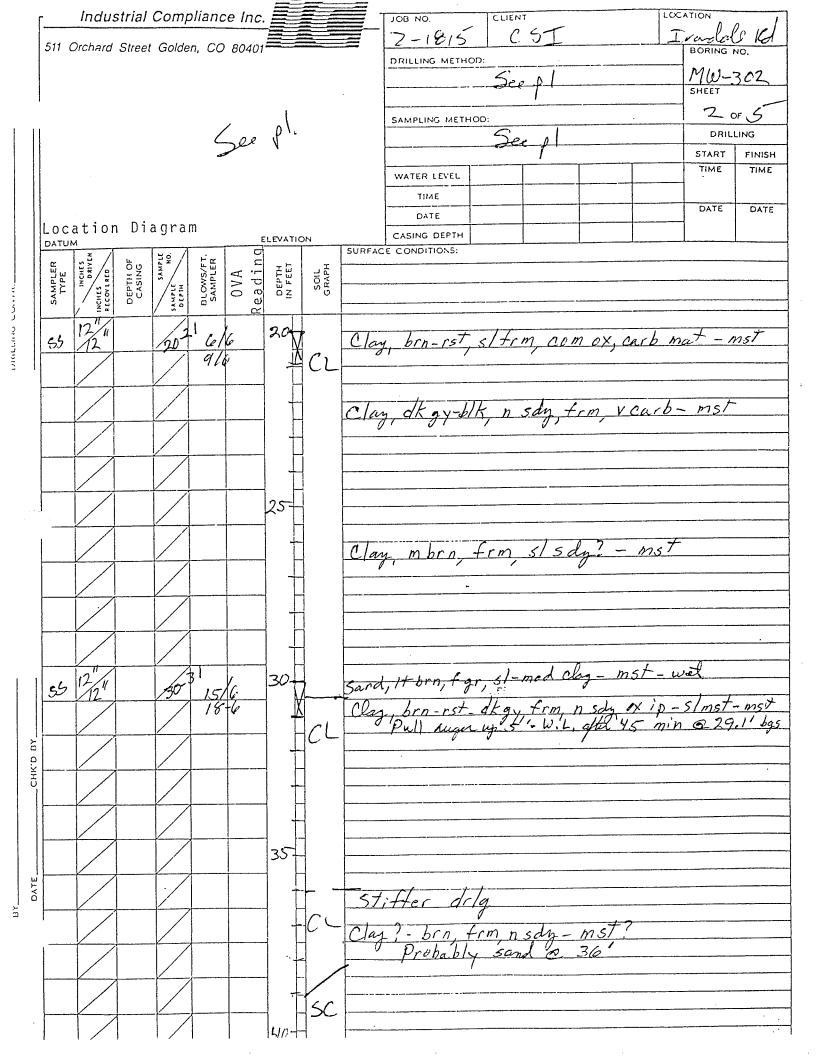
INDUSTRIAL COMPLIANCE

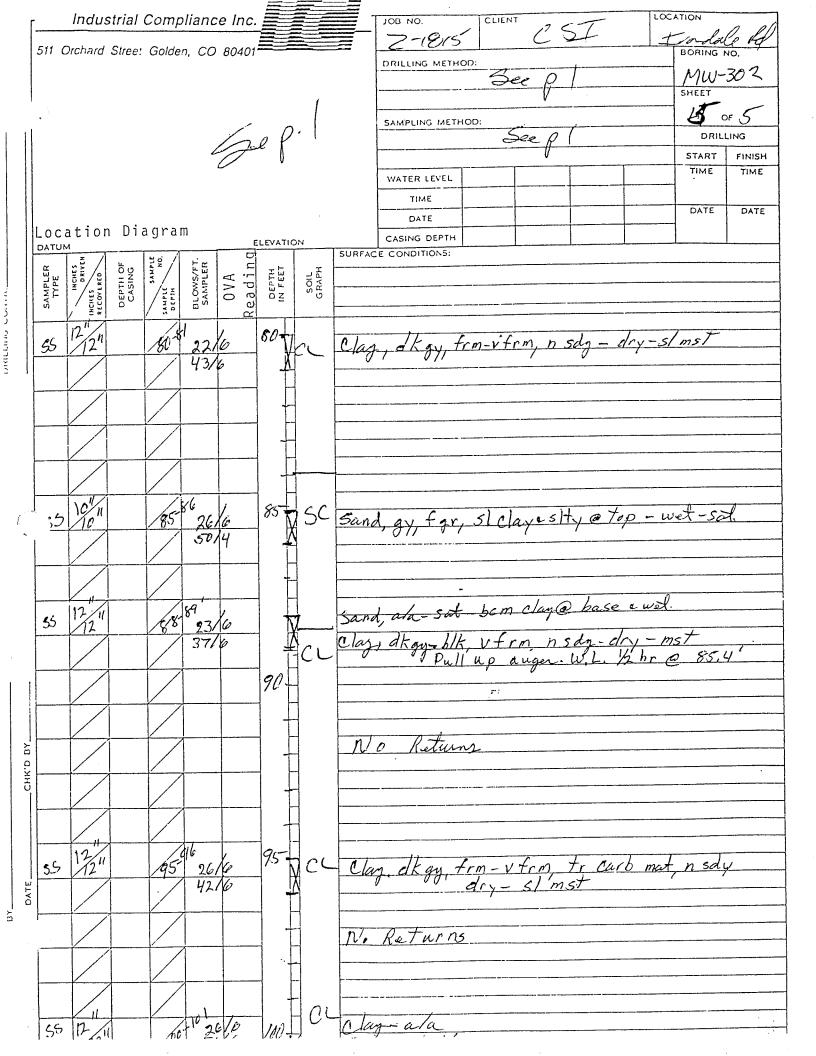
LOG OF BORING NUMBER: NW- 203 SHEET NUMER: / OF 1 INDUSTRIAL COMPLIANCE
AN SP ENVIRONMENTAL SYSTEMS COMPANY
1746 COLE BLVD.
BLDG. 21 SUITE 300
COLDEN, COLORADO 80401
(303)277-1400
FAX (303)277-1405 DRILLING METHOD: HSST-CMC 75 1 Ø, SAMPLING METHOD: CONTINUOUS

LOCATION DIAGRAM: pisposis Ave Aren MW-203

START	DATE: 2 1	41-92 F	105 A	MW-I ATE:	; 4 2.23	SURFACE ELEVATION: WATER LEVEL: TIME: DATE:
	7	OVA READING	SPT	DEPTH IN	SOIL GRAPH	SOIL DESCRIPTION AND DRILLING CONDITIONS
Court-	1/0-X'	시		2	ML	Moist Ton silty clay short material (silty work) Aroust Durk brown wilty clay, miner time grained send
cu.L	2/5-10:	N/M		1		Alb Retzern on 5-10 internal- outling it Bon silty elong, wise, Simil I fine grained)
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Compliance 511 Orchard Street Incorporatori Golden, Colorado 80401

FIELD DRILLING LOG

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FIELD DRILLING LOG

Date: 7-11-88 Sheet 1 017 Job No. _ 2-1815 (nd Elev. ______Boring No. 5B-6 Dis. 4 T.D. 25 Approval Date ____ _ Driller Boxles Bez Rig blaco TRACK X Field Engr./Geol.___ Water Table After Drilling. Water Table At Drilling: Dry Water: Depth 1st Encountered: □ Won't Date ______By ____ Water - After Ø_____ ☐ Might Caved - After Ø Dry at depth ____ □ Should Covered D Couldn't Find D Will PID (ppm) Color Field Soil Description Bir, fine ara, solty 50 1/215 Bon. Drill Break 018-20 Hzo 251 extremely EASY On Iling 8/1. W DL 2025 GIAJ-DAIR Brow, For Stair. 51 CL SLIFT 25 after Hole open to 16' raisel 9' Standard Samples-Comments: Fleid Laboratory Sample Description

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Date: 7-11-88 Sheet 1 of 1 Project _____ (SI TE Job No. 2-1815 ___Boring No. <u>5</u>B-7 4______T.D. 47' and Elev. ms Approval Date __ Project Manager ____ Driller Byles Rig Sino Topic Field Engr./Geol. Water Table At Drilling: Water Table After Drilling: Dry Water: Depth 1st Encountered: ______ Water at depth ____ □ Won't Water - After Ø_____ Date ______By ____ ☐ Might Caved - After Ø _____ Dry at depth ____ ☐ Should □ Covered Couldn't Find □ will PID Field Soil Description (ppm) 564151 med. Brn. Frynod gen. 17-18 and Break , BR-gray, port. Fr stignod-your plastic. 51. 51.51 HV + FRANCY L+- med Brc \$1.4 m 28- Harder - whentiered BR JIA 2-5-30 CL HAND So Stringers god much returns. 20-35 (L < A A 2.1 360 ensier Drilliac SAA TID med BEN SILY, STEARS Samples-Spoon Comments: 27-10 Cuffing withouring up Standard Penetrat. Field pryco 841 Laboratory Sample Description

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Compliance 511 Orchard Street Incorporated Golden, Colorado 80401

FIELD DRILLING LOG

Date: 7-14-88 Sheet 1 of 1 Project _____(ST-=: _ Job No. _ 2-/8/ ~ and Elev. Boring No. 5B-9 Dia. 4 T.D. 201 Approval Date _____ Kt. Driller Beyles Bror. Rig Linco TRAIL Field Engr./Geol.____ Water Table At Drilling: Water Table After Drilling: Water: Depth 1st Encountered: _____ Dry Won't Water at depth _____ Water - After Ø_____ Date ______By ____ ☐ Might Caved - After Ø _____ Dry at depth ____ ☐ Should Couldn't Find Covered □ will Depth SOIL CLASS Syller Moles Color PID Field Soil Description (ppm) Sett Sin Med Rock 8', Festain . 0-5 5-10/1 13 rediageral 0 STIPE STIM 10-15 CLS TD med Bin, s). sAnd. 4,CC 20 CLS Samples-Spoon Standard Comments: Depth Fleld · Laboratory Sample Description

FIELD DRILLING LOG

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CAR D							3	H	Fin	L. #20 6	Soull Co	Ves Karl	1/3	<u>- ₹</u> ~	,
							A	H	1.00	or and					
	Sep.	18	15		18 18 20	الممتين	/5	H	dive	gy clay w/	fisund lea	NOS - 2-	3 ′′	thet	
UNIE	٠,٠	10	1)		×0 <u> </u>	, 0,	6	\forall	ivan	5/21, mai 31 - 5/21/2, n Cl	+ sa	EI- day			
-							7			V					
-							8	H		(3)					
							9	H	Olive	9y- 94 clu	labum ir stain	ry and	des	60 p	27
		18		K-	٠,٠	PGND	20	H	to	sk clim abm	carbon di	rda 57ain	'')	-dhy	<u> </u>

	ION OF BORI	NG					JOB NO CLIENT L	OCATION	
							DRILLING METHOD	BORING	NC:
							STREET TO THE TOP	55-	78
								SHEET	.1
							SAMPLING METHOD:	2	of 4
									LING
								START	FINISH
							WATER LEVEL 39.5 39.2		
							TIME 1/20 1/50	DATE	DATE
							CASING DEPTH 3.7 37		
DATUN	z /	50 /		!	ELEVA	TION	URFACE CONDITIONS # 12-14 (10)		
SAMPLER TYPE	INCHES DENVEN CCOVERED DEPTH OF	SAMPLE	BLOWS/FT SAMPLER		H	PH H		· · · · · · · · · · · · · · · · · · ·	
SAM	INCHE INCHES PECOVERED	SAMPLE	3LOM SAM		DEPTH IN PEET	SOIL			
	7 2 2	/ 50				, 			
					20	122			
				grab le	,	C'Alken 2 me	6-b		
		1/			2				
					3	H			
		1/			4	H			
alif	18	55-2	26		-	-	34ft- 140, 5150 C/ 41/ 5000 0	⊋-⊋″	46.
<i>a</i> "	18 25	9/	36 47 55/53	<i>(t</i>	25		Buff- It gysiga cl w/ some =	ann	
					6.	VCL	dity		
		+	-		-				
					7	H			
		1/			8				
					-			s	
					9	Н	1100		
212	18	55-3	24 67		30	G C15			
Calif	18 30	///	38/61	<u> </u>	-		by C1. stone, ilon stand in the	2,	
					1		No Append Sand Lenses		
		17	1		1		mple = 157)		
		$-\!$			2	П	N .//		
				1	3	1	Dulling Soften up		· · · · · · · · · · · · · · · · · · ·
	1 / 1	1/	1	l	_	무밀	ruga Ca Hongs of a SS 61 m	DOT -	
		1/	1						
		/,			Ą	سلام ا			
21.F	18,0 26	المنزود ا	39 75/44	4 110	1	سلام ا	3 No. F. gtz Sand moist-wet		
?1.F	18/18/34	1 35/4	.59 75/43	4 310	1	سلام ا	3 rent: gtz Sand moist-wet		
21.4	18 35	5 35-4	.59 75/45	4 310	1	5W	3 Minf: 9tz Sand, moist-wet		
21.4	18/18/34	35-1	39 75/43	4 310	35	\$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$	3 rent: gtz Sand moist-wet		
Col. F	18 34	354	.59 75/43	4 310	35	\$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$	3 M.F. 9tz Sand moist-wet calif + gab sample		
<u> </u>	18/18/34	(35° N	39 75/45	4 310	35	54 54 51 71	3 No. F. gtz Sard, moist-wet		
[2]. {	18 35	, 35 th	39 75/45	4 310	35	54 54 112	3. No. F. gtz Sand moist-wet Calif + gab-sangel		

DRILLING CONTR.

LOCAT	TION OF BOR	IING					ON BOL	CLIENT		LOCATION	
							DRILLING MET	400·		BORING I	
							DIGELING MET			33	-28
										SHEET	
							SAMPLING MET	HOD.		- 3 ,	x 4
										DRIL	LING
										START	FINISI
							WATER LEVEL			TIME	TIME
							TIME				
							DATE			DATE	DATE
DATUN	ч			E	ELEVAT	NON	CASING DEPTH				
œ	~ × ×	# Q /	<u>- n</u>			_	SURFACE CONDITIONS:				
SAMPLER TYPE	INCHES DRIVE INCHES RECOVERED DEPTH OF	CASING SAMPLE NO.	BLOWS/FT SAMPLEP		DEPTH IN FEET	SOIL					
SAR	OE GO	CA	BLO		ÖΖ	8.5					
	10/			,	.1.	575	Bright sond	mo(3+	. <u>W</u>		
al.t	18 4	0 3/	38 50	Grob	/0	TSW	100 grana				
					,	I ML					
					' '	9	DK gy day se	chydone			
					2		0 /				
	$\overline{}$	-					Della	to be har	& From a	41.51	
					3		Drilling cant	. To be rias	4 From	41.3	
					1						***
		17				115					
					4	C15		1 meat	,		
	19	_55	27	S /]	_c'/s	Gy Cl. Store	Any want	/ junst	to shory	
Cal.f	19/18 4	5 55 1	27 32 45	EZ _{Ja} b]	<u>C</u> 15	Gy Chston	dry man	iven st	dining	
Cal. F	18 4	5 55 5	27 32 45	ED jab]	278	Gy Cl.Slow	dry smal	/ jron st	any	
Cal.f	18 4	5 55 1	32 45	E Zab	y 5 -	<i>C13</i>	Gy Chston	dry smal	irm s+	king	
Cal.f	18 4	5 55 7	27 32 45	D _{ola} b	<i>y</i> 5 -	2/3	Gy cliston	dry smal	iven St	dining.	
Cal.f	18 4	5 55 14	37 32 45	P. Jab	// 5 - 6 - 7	275	Gy Cliston	Jug smal	irm st	lanny	
Cal.f	19 4	5 55 7	27 32 45	P. Jab	y 5 -	273	Gy Cl.Story	dry in	irm St	duny.	
al.f	18 4	5 5 5	32 (45	D _{ja} b	//5 - 6 - 7	C/15		Jug smal		kanny.	
Cal. f		5 55 7			45- 6- 7 8			dry in		dining.	
Cal. f	18	5/			45- 6- 7 8			Jug J. mark Jug J. mark J. fr. 15.		kanny.	
Gl.f	18	554	27 32 45 45		45 7 8 9			B, dry		Ling	
Calf Sp 35p	18	5/			45- 6- 7 8			B, dry		kanny.	
Calf Sp 35p	18	5/			45 7 8 9		Lt br figtz 3	B, dry		Ling	
Calf Sp Sp	18	5/			45 7 8 9		Lt br figtz 3	B, dry			
Calf Sp 35p	18	5/			45 7 8 9		Lt br figtz 3	B, dry		taining	
Gl.f	18	5/			45. 6. 7 8 9 60. 1 2		Lt br figtz 3	B, dry			
Calf Sp	18	5/			150 7 8 9 50 1		Slivegy Clsilo	18, day ne 51.1	- 57.5		
3p	18 8 5	5 /	40 m/sg	. Grab	150 7 8 9 10 1 2 3		Slivegy Clsilo	18, day ne 51.1	- 57.5		
Gl.f	18/8/5	5/	40 m/sg		150 7 8 9 10 1 2 3		Lt br figtz 3	18, day ne 51.1	- 57.5		
3p	18 8 5	5 /	40 m/sg	. Grab	150 7 8 9 10 1 2 3		Slivegy Clsilo	18, day ne 51.1	- 57.5		
3p	18 8 5	5 /	40 m/sg	. Grab	45. 6. 7 8 9 50. 1 2 3 4. 55.		Slivegy Clsilo	18, day ne 51.1	- 57.5		
3p	18 8 5	5 /	40 m/sg	. Grab	45. 6. 7 8 9 50. 1 2 3 4. 55.		Slivegy Clsilo	18, day ne 51.1	- 57.5		
3p	18 8 5	5 /	40 m/sg	. Grab	15 6 7 8 9 50 1 2 3 A 5 5 6 7		Slivegy Clsilo	18, day ne 51.1	- 57.5		
3p	18 8 5	5 /	40 m/sg	. Grab	45.6.7 8 9 50.1 2 3 4 55.6		Slivegy Clsilo	18, day ne 51.1	- 57.5		
3p	18 8 5	5 /	40 m/sg	. Grab	45.6.7 8 9 50 1 2 3 A 55.6 7 8		Slivegy Clsilo	18, day ne 51.1	- 57.5		
3p	18 8 5	5 /	40 m/sg	. Grab	15 6 7 8 9 50 1 2 3 A 5 5 6 7		Slivegy Clsilo	18, day ne 51.1	- 57.5		

DRILLING CONTR.

	TION OF	BORING	,					JOB NO	C	LIENT		LO	CATION	
								DRILLING	METHOD:				BORING	_
														-28
													SHEET	, 1
								SAMPLIN	S METHOD:					of 4
													START	FINIS
								WATER	EVEL 50	·		1	TIME	TIME
								TIM					1	1590
ļ								DAT	1 2 (9 -				DATE	DATE
DATU	,		, , , ,	,	E	LEVATI	ON	CASING						122/2
SAMPLEP TYPE	INCHES DRIVEN	DEPTH OF CASING	SAMPLE NO DEPTH	BLOWS/FT SAMPLER		DEPTH IN FEET	SOIL	SURFACE CONDITI	ONS					
						60 -								
						1	1	Dr. Kny San	myler rai	45 5	5 Sing	Ų		
						2							•	
						3								
						4		Hoo		₩,				
ST	18/		7	22	//	1	7							
1	18/18	65		37/9	Grob	65		DK gu el	estone,	dr	je/	Some	12 -1	· ·
7	18	65		37/4 54	(510b	65		DK q. el	e stone, F. sa	long	in/	5000 51/43	12"-1.	<i>(,</i>
	/18	65		37 B	Grob	. }		DK gy el	eystone, F. Six	long	.e	5000 - S. 1 /2 / 2	\(\frac{1}{2} \))
A	18	65		37/A	Grob	6 4		DK g, el	aystone, F. six	Long	~/ · x -	Ser /23	13"-1.	· · · · · · · · · · · · · · · · · · ·
1	18	65		37/0	Jorob J	6		DK g. el	eystone, F. six	Louis	.x/	500 1/23	12"-1"	
A	18	65		37 B) (Trob	7		DK g. c.l.	aystone, F. sa	An house	- in f	50 / E3	13"-1")
	18	65		37 6) (Trob	6 7 8 9		DK g, el	aystone, F. six	Long		Ser / 25	13"-1")
	1/18	65		37/2	7706	.6 7 8 9		DK g. cl	eystone, F. six	long			12"-1"	
	1/18	65		37 B) (770b	6 7 8 9 7 0 1 2 3		DK q. el	Aystone, F. Six	An house			13"-1"	
			55-7			6 7 8 9 70 1 2		DK gr cl	aystone, f. sa	Claus	Lone	Ser / Li	12"-1"	
Calif			55-7		бтав	6 7 8 9 70 1 2 3 4 75		DK g. cl.	gy gy gy gy stone	Clay S	Lone		12"-1"	77
			55-7			6 7 8 9 70 1 2		Dkg, ch	gy gy	Clays	Lone		12"-1"	
Calif			55.7			6 7 8 9 7 1 2 3 4 7 5 6		Dkg of Hgy V	gr gy stelme	Claus	Lone	150me	12"-1"	
Calif			55-7			6 7 8 9 70 1 2 3 4 7 5 6 7		Dinegy- Soil Bon	gr gy stone	Clays	Lone	1500 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	15"-1"	

							Indu	istria	Compliance	Inc					
Γα	ATION OF								108 NO	CLIEN	_			ATION	
1	N	8H H	ful						2-1815		CS.I		1/	BORING A	10
	- V 8	87.		·					DRILLING METHO	00: CM	1E-75			BURNES	
,	T		56-1		01				Solid 4	" At	iger			SHEET	29
7	1	•	56-1		50-2	-					1-1	- 1 / /	/ /	,	+4
lı r					1		,	_	SAMPLING METH	OD: 7	al.F/5	PIT	9146	DRILL	
						٨_	246	عه (⁶						START	FINIS
							-	, , , ,	WATER LEVEL			T T		TIME	TIME
									TIME					1620	
									DATE					DATE	DATE
j DA	rum				ı	ELEVATI	ON		CASING DEPTH					2/22/88	
,	Z.	1 4	## /	Fα			T	SURFA	CE CONDITIONS:		<u> </u>				
SAMPLER	INCHES DRIVEN	DEPTH OF CASING	SAMPLE NO.	BLOWS/FT. SAMPLER		DEPTH IN FEET	SOIL	Br	51 50 n	/50n	e class	, de	<u>'</u>		
SAM	C C S	DEP	SAMPLE	BLO		O Z	SEG					<i></i>			
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DRILLING CONTR.	/		1/1	P-		1	350								
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						2	1								
	+	+			 	1 _	1					-			
						3				5/					
						4		B1.	me - co 4	stzj:	sa u/	Som	o Cla	- 12 m	255
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1 1 4	18/18/	10	9 /	5	grab	/0	7	150	- FI- CO	5/3	a	nais		VIII 1	
	21	10	1	<i>y</i> -	1	1.	X								
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1 11						3	1			,					
188	1/	1	17	1		A									
12/2	\perp	<u> </u>	<u>/_</u>			_ ″.					<u> </u>				
3	1/					5	Her					<u></u>			
10 m	- 	+	+-	 	-	- -	エング	1/2.5	- Augus cul	rine 5	Br Si	c1.m	25+		
PATE						6		711.7.2	- Augusta	0					
¥ -	1/	1	1/	1		7									
	/		¥.,	 			Н		· · · · · · · · · · · · · · · · · · ·			,,			
18-1	1/		\perp		1	8	Н	_							
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-	_//					9		31	515a c	di	- 31 ma	54			
	59 18	1	1/	78	Links	20	LK-	-		· · · · · · · · · · · · · · · · · · ·	1				
: 13°	1 10	20	V_{-}	1 13	1 2100	10	<u> </u>								

FOCY.	TION OF	BORING							OB NO	CLIENT		L	OCATION	
								-	DRILLING METH	100			BORING	NO
													5/5	-29
													SHEET	
									SAMPLING MET	HOD.			12	'-
											 		DRIL	
								-	WATER LEVEL				START	FINISH
								-	TIME	Dry 740				
									DATE	8/23/88			DATE	DATE
DATU	м		_		!	ELEVATI	ON	T	CASING DEPTH	37 candie				
	\ ž /	5.0	SAMPLE	<u> </u>				SURFACE	CONDITIONS:					
SAMPLER TYPE	INCHES BRIVEN FICOVERED	DEPTH OF CASING		BLOWS/FT. SAMPLER		DEPTH IN FEET	SOII. GRAPH							
SAI	PECO	E O	SAMPLE	BLC SA		ΩZ	ن ا							
4 L	18/			70 8.	Doub	20								
P5.	18	20	/	15	Orâb	~~ A	A		-					
,						1	CO.	Grade	5 SANJU	er of it	Z/.	1+61-	br 5	/
						1	+ CL	7000	sa, e calcite	se sosi t	in sta	lo		
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						3	-							
	\forall					 	\dashv		-	· · · · · · · · · · · · · · · · · · ·				
_						4								
Calif	18/	25	55-1	15/35	grab	25 =	+	Br.	5; , 5a C	elay, o	V/sme	thin f	sa lan	515
_	192	スンコ	/	سي م	0		11	1						
	12	-			1	1	H	mois	/-					
	16					6	<i>†</i>	mois						
	16					4	#	Mois						
	16					6		Mois		/				
	(6)					4	CL	MOIS						
	(6)					7	# CL	mois						
						7 8 9	# CL							
alit						7	CL		, sa e),	moist				
alif		30		I) AT		7 8 9	CL			moist				
alit						7 8 9	CL			maist				
Calif						7 8 9	CL			moist				
alif						7 8 9 30	CL			mais t				
alif						7 8 9 30	CL			maist				
alif						7 8 9 30	CL	Bn. 8	, sa e),					
		30	55' 3	13/61		7 8 9 0 1 2		Bn. 8						
alif alif		.30	55' 3	13/61		7 8 9 3	C	Bn. 8.	, sa e),	728 w/ se	me 1/4	(" pea		
		30	55' 3			7 8 9 3 1 2 3 4		Bn. 8.	sa Cl,	1728		(" pea		
		30	55' 3	13/61		7 8 9 30 1 2 3 4	C	Bn. 8.	sa Cl,	728 w/ se	me 1/4	(" pea		
		30	55' 3	13/61		7 8 9 3 1 2 3 4	C	pulled so	isa C), Si Si C/	728 w/ se d/ (6r +	mp /4	(" pea		
		30	55' 3	13/61		7 8 9 30 1 2 3 4 35	C	pulled so	sa Cl,	728 w/ se d/ (6r +	me 1/4	(" pea		
		30	55' 3	13/61		7 8 9 30 1 2 3 4	C	pulled so	isa C), Si Si C/	728 w/ se d/ (6r +	mp /4	(" pea		
		30	55' 3	13/61		7 8 9 30 1 2 3 4 35	C	pulled so	isa C), Si Si C/	728 w/ se d/ (6r +	mp /4	(" pea		

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LOCA	ATION OF I	BORING							JOB NO	CLIEN	Т		ro	CATION	
									DRILLING METH	DD				BORING	NO
									The state of the s						-29
									,					SHEET	d
									SAMPLING METH	IOD.					of T
															LING
									WATER LEVEL			1	T	START	FINISH TIME
									TIME			 		-	
									DATE				ļ	DATE	DATE
DATU	JM				1	ELEVA	ΓΙΟΝ		CASING DEPTH						
	2 Z Z	ት ፡፡	SAMPLE	Εœ			T _	- 1	CE CONDITIONS:						
SAMPLEP	INCHES DRIVEN NCHES RECOVERED	DEPTH OF CASING	1 / !	BLCWS/FT. SAMPLER		DEPTH IN FEET	SOIL								
SAI	INCHE RECO	E O	SAMPLE	BLC		ΔZ	ی ۳۰								
11-	118/	1-		16 25 (4) 30	asab	40		1.4	by si	veFi.	sa c	1, 10	1500	me va	
Cali	1/8	40		2 9 C		′ -	$\mathbb{H}_{\mathcal{A}}$	Sife	enry mc	Lay	(1:1)	ly - 5	mas	/	
				•		1	U CZ	· m	OCCUSIONAL S	· Fisal	(6/)	Sand 3	tringa	- S	
				-		2									
	+	·····				-	Н				<u></u>				
						3	Н								
						4				r.fi				, ,	
	1		35-5	<u> ২৪</u>			HALL	11/	r-br 511	15a in	Some	clay,	d	14-5/4	2015/
111	∧! <i>!\$/</i> .!		155						- 11		//	1.1.1	/.	11 -	- 1
12/1	1/15	45		46	Grub	45.	24	30	me iron sta	ining	Cie	less'	clay	Han ab	ome)
Cd.+	f 1 5	45		28 46 17//3"	Grub		EL	30	me iron sta	ining	'()x	less'	clóy	Han ac	(ou.e.)
(21.1	15	45		46 17//311	Grub	¢5.	CL	30	me iron Sta	ining	' (less'	clóy	Han 60	ons)
Cálit	15	45		46 17//31	Grub		e L	30	me iron ste	in ing	' () &	. /ess '	clay	Han ab	(oma)
Cédit	15	45		46 71/31	Grub	6	CL	30	me iron sta	ining	(//	/ess '	cláy	Han ab	(our)
Cid+	15	45		46 171/311	Grub	6	- CL	30	me iron sta	ining	' <i>(</i>	. /ess '	clay	Han ab	(aux)
Cid	15	45		46 171/311	Grub	6		30	me iron sta	in in	() () ()	less'	clay	Han ab	
						6 7 8 9		30	me iron sta v Fi sitsa u	It was	(10	/ess /	clay wass	Han al	
Cal				46 171/311 337 85/41		6 7 8 9		30	me icon sta VE: Sitsa u Side Garrel	/ true	(j.e	less'	unais.	Hando	(aux)
						6 7 8 9		30	me iron sta V Fi Sipsa w Side Garrel of Probably its	/true	colar	less'	eláy massi chire	Hando	Sove
						6 7 8 9 50	Z W	30	me iron sta y fi sitsa w side banel in pelo	/true	e cla	less!	inois Jane	Hands	Save
						6 7 8 9	Z W	30	me iron sta y fi sitsa u postably in polo	/true	c cla	less!	elóy massi chre	Handa f-Asserting	Jane Source
						6 7 8 9 50	Z W	\$0 \$0 \$0 \$0 \$0 \$0	J. Fi. 5245a w Side Garrel in probably in	true took	colonial de la coloni	less'	eláy massi clare	Handa f-das most chy	Sove Since
						6 7 8 9 50 1 2 3	Z W	\$0 \$0 \$0 \$0 \$0 \$0	me iron sta y fi sitsa u side band in polo	/true	e de sta	less'	elay mais clive	Handa	Sove Sove
						6 7 8 9 50 1 2 3 4	Z W	\$0 \$0 \$0 \$0 \$0 \$0	J. Fi. 5245a w Side Garrel in probably in	true / true / satu	colar	less'	elay mais clare	Handa f-San most chy	Sove Sove
						6 7 8 9 50 1 2 3	Z W	\$0 \$0 \$0 \$0 \$0 \$0	J. Fi. 5245a w Side Garrel in probably in	true / true ice f)	classia.	less'	eláy mais clare	Hando	Some Since
Calc						6 7 8 9 50 1 2 3 4 5 5	Z M	\$0 \$0 \$0 \$0 \$0 \$0	Sipsa u Sipsa u Side band i Probably in	/true	c classia.	Jess'	inais Jana	t-Asset as strictly	dove fra
						6 7 8 9 50 1 2 3 4	Z M	Br So	Sipsa u Sipsa u Side band i Probably in	In my	c classia	less'	elay massississississississississississississi	Handa f-dan most chy	Sove Since
Calc						6 7 8 9 50 1 2 3 4 5 5	Z M	Br So	Sipsa u Sipsa u Side band i Probably in	It was	e cla sla casal	less'	eláy mais chre	t-da	Sine (Asia
Cale						6 7 8 9 50 1 2 3 4 5 5 6 7	Z M	Br So	Sipsa u Sipsa u Side band i Probably in	true / true	c classia	less'	clay mais clay lt. b	Handa f-San most chy	Sove
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8 9 Truga Cuting Br 3150 W 30m2 Clay to SISKEL		17					6							,				
* Trugo Cutmap - Br 3150 w/ 30mm Clay to sished	·	/					-	H-250	trac	e of chy	16.	<u> </u>	16.5'					
* Trugo Culmay - Br 31 Sa W 30m2 Clay to SISKEL							7	Н										
* Trugo Culmay - Br 31 Sa W 30m2 Clay to SISKEL	11.80)	1/					8											
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LOCA	TION OF E	BORING	,							JOB NO	CLI	ENT			LOCATION	
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DATU		L.	50 /	F or	<u></u>				SURFAC	E CONDITIONS:	<u> </u>					
SAMPLER TYPE	INCHES DRIVEN S ERED	DEPTH OF CASING	SAMPLE	BLOWS/FT. SAMPLER		ОЕРТН	FEET	SOIL								
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LOCA	ATION OF E	ORING							JOB NO		CLIEN	Τ		LOC	ATION	
									DRILLING	G METU	DD:				BORING	NO.
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									SAMPLIN	G METH	00				 	of 4
															ļ	LING
									WATER	LEVEL	48	48.2			START	FINISH
									TIM		1630	0920			1	16/5
									DAT		7/23/88				DATE	DATE
DATU	ML				ĺ	ELEVAT	TION		CASING	DEPTH	51.510x	3/5 cpm			1	8/23/8
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SAMPLER	INCHES DRIVEN INCHES RECOVERED	DEPTH OF CASING		BLOWS/FT. SAMPLER		DEPTH IN FEET	SOIL	HAP -								
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[a];	1/2		35-37 45	39 65/6"	grab	1	1 3	A 3	AA , S	5/ C	cursor stool	sa Cit	e fi af or	-me	gtz	54) 1546
Cali	1/2		55°3/ 45	39 65/6"	grab	45	34	P 5	AA,	5/ C 210/ 20n	cuisor 5+-pt	to either	n coz	partar		54) 1005 1005
Cali	12/12		55°3/ 145	39 65/k"	grab	1	- J. J.	¥ 5	AA , 3	5/ C 2101 2001 Sond	cursor 5+-pt cleane	to either	n con	partar		SA) Insus
Cali	112/12		55°37 45	39 65/6"	grab	45			(AA , S	5/ C 210/ 20n sond	cursor 5+-pt f. So cleane	to either	n coz	partar		sa) Salansos
Cali	12/12/12		3537 45	39 15/k"	grab	45			TAA, S	5/ C 2001 2001	cursor 54-pt e So Cleone	to either	n coz	partar		Sa) Fath Jonses
Cali	7/2		55°37 45	39 45/k"	grab	45			AA, S	5/ C 210/ 2001 50nd	cuiser 54-06 e. Si cleone	to either	n coz	partar		(on sos
Cali	12/12/12		55°37' 45	39 15/k"	grab	45			TAA, S	5/ C Maj zon Sond	cuiser 54-pt 6. So Cleane	to either	n coz	partar		54) 1916 (on sos
	1,2					45 6 7 8 9			Kbr me	200 20nd	cuisor 54-pt e. So cleane	to either	n coz	partar		SA) Employees
	1,2			39 65/6"		45 6 7 8 9				200 20nd	cuiser 54-06 e. Si cleone	to either	n coz	partar		54) 1900 1000 S
	1,2					45 6 7 8 9				200 20nd	cuiser s+-pt s cleane	to either	n coz	partar		SA) Sansos
	1,2					45 6 7 8 9				200 20nd	cuisor 54-pt e. So clean	to either	n coz	partar		SA) Fall Consus
	1,2					45 6 7 8 9				200 20nd	cuiser 54-06 e. Si cleone	to either	n coz	partar	£ 55.0	Sa) Sas Sonsos
Palit Alit	1,2					45 6 7 8 9		M A		200 20nd	cuisor 54-pt e. So cleane	to either	n coz	partar	£ 55.0	SA)
	1,2					45 6 7 8 9 50 1 2 3				200 20nd	Cuiser 54-06 e. Sc clean	to either	n coz	partar	£ 55.0	Sa) Fansos
	1,2		35.47	29 47 57	t arab	45 6 7 8 9 50 1 2		NA A	K br me	200 20nd	cursor 54-pt e So Cleone	to either	n coz	partar	£ 55.0	SA) Fall Janus S
			\$5.00 55.00	29 47 57	t arab	45 6 7 8 9 50 1 2 3		M A	K br me	200 20nd	cuisor 54-pt e. So clean	to either	n coz	partar	£ 55.0	SA) Fansos
217			35.47		t arab	45 6 7 8 9 50 1 2 3 4 5		NA A	K br me	200 20nd	54-08 8. 50 Cleane	to either	n coz	partar	£ 55.0	Sa) Sa) Sas
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			\$5.00 55.00	29 47 57	t arab	45 6 7 8 9 50 1 2 3 4 5		NA A	K br me	sond	54-08 8. 50 Cleane	to either than the wet	alare	pantan Samp	£ 55 L	
217			\$5.00 55.00	29 47 57	t arab	45 6 7 8 9 50 1 2 3 4 5 6 7		NA A	K br me	sond	54-08 8. 50 Cleane	to either than the wet	alare	pantan Samp st	£ 55 L	
21.7			\$5.00 55.00	29 47 57	t arab	45 6 7 8 9 50 1 2 3 4 5 6		NA A	K br me	sond	54-08 8. 50 Cleane	to either than the wet	alare	pantan Samp st	£ 55 L	
217			\$5.00 55.00	29 47 57	t arab	45 6 7 8 9 50 1 2 3 4 5 6 7		NA A	K br me	sond	54-08 8. 50 Cleane	to either than the wet	alare	pantan Samp st	£ 55 L	

.OCATI	ION OF E	ORING			· ·				JOB NO	CLIEN	7		Loca	ATION	
					•				DRILLING MET	<u> </u>	έ.			BORING	NO
									DRILLING MET	HOD.				36-	30
														SHEET	
									SAMPLING ME	rhod:				4	or 4
								* سر						DRIL	LING
											, 	 		START	FINISH
									WATER LEVEL	70.1	48.1			TIME	2920
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		*							DATE	8/25/88	7				8/25/8
MUTAC			1 = 1		E	ELEVAT	100	SL	CASING DEPTH	65 open	75 open	i		l	1 /00
PER	PACHES DRIVEN S	H OF	ON NO	S/FT		ET		<u> </u>							
SAMPLER TYPE	INCHES PECCYFRED	DEPTH OF CASING	SAWPLE DEPTH	BLOWS/FT. SAMPLEP		DEPTH IN FEET	SOIL	GR/							
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Set	14/14		15	27 48	Grub		5/	SI I	Brfisa	A g	tr sa	u/ a	bun a	1 K ma	trix
Spt Spt	14		165	27 48 60/3"	Grub	65.	5/5/5/	SID I	Br fi sa	£ 3	fz Sa Gy Clst.	uf a	bun A	Kme	trix 1
Spt Sp	14		165	27 48 60/9"	Grub			S I	3r \$1,5A 65.6'-65. Csa above	8' - 0 + below	tz Sar Gy Clst.	u/a lense st-u	bun a sl.	K ma	frix 1
Spt	14		15	27 48 WB"	Grub	65.		SI I	3r \$ 50 65,6' - 65, (sa above	8' - 0 + below	fr Sa Gy clst. I mo:	uf dense st-w	s bun A 5/. cf)	K ma	trix 1
Spt	14		15	27 48 60/5"	Grub	65.	5/3		3r \$ 5A , 65.6' - 65, Csa Above	3' - c + below	tr sa Gy clst. Mai	u/a lense st-u	sbun d sl. et)	K ma	frix 1
Spt Spt	14		165	27 48 60/9"	Grulo	65.	5/5/5/		3r \$1,5A 65.6' - 65. (sa above		fr Sa Sy Clst.	u/a Lense st-u	s bun A 5 l. et)	K ma	frix 1
Spt	14/14		15	27 48 60/5"	Grub	6 7 8	5/5/		3r \$1,5A , 65.6' - 65, 65.8' - 65,		fr Sa Sy clst. ma:	u/a Lense st-u	s bun A 5/. et)	K ma	frix 1
Set	14/14		165	27 48 60/5"	Grub	6 7 8 9			3r \$1,5A 65.6' - 65. 68.000		fr Sa Sy Clst.	u/a lense st-u	s bun A 5/. et)	lk ma	trix
Spt	14/14		15	27 48 60/5"	Grulo	6 7 8	5/5		3r \$ 5A , 65,6' - 65, 68 Above		tr sa Gy clst.	u/a Lense st-u	s bun A 5/. et)	K ma	frix /
Spt	14/14		As	27 48 60/5"	Grub	6 5 - 6 7 8 9 7 0	5/5/5		3r \$1,5A, 65.6'- 65, 68 Above		fr Sa Gy Clst.	u/a lense st-u	ibun A sl. el)	K ma	trix
Spt	14/14		165	27 48 60/5"	Grulo	6 7 8 9	5/5		3r \$; 5A , 65,6'-65, 68 above		fr Sa Sy Clst.	u/a lense st-un	shun A 5/. ef)	lk ma	trix
Spt	14/14		As	27 48 60/5"	Grubo	6 5 - 6 7 8 9 7 0	\$155 \$155 \$155 \$155 \$155 \$155 \$155 \$155		3r \$1,5A , 65.6' - 65, 68 Above		fr Sa Gy Clst.	u/a lense st-u	shun A sh.	enois:	trix
Sep	14/14		15	27 48 60/5"	Grub	65.678970	5/3/5/		3r fi 5a , 65.6' - 65. 68 Above		fr Sa Sy Clst.	u/a lense st-un	s bun A 5/. et)	lk ma	trix
Spt	14/14		As	27 48 60/5"	Grubo	65.		SIP W	3r \$1,50 , 65.6' - 65, 65.6 Above		fr Sa Gy Clst.	u/a lense st-u	shun A sh.	L ma	trix
Spt	14/14		A5	27 48 60/5"	Grub	65.678970	5/3/3		3r fi 5a , 65.6' - 65, 63.8 Above		tr sa Gy clst.	u/a lense st-u	s bun A	lk ma	trix
Spt	14		15			65.678970112334			3r \$1 5A 1						
3er 3er	14 14 14 14 14		15			65.678970112334			(5,6'-45, (5a Above			lense st-u			
30°F	14				Grub	65.678970112334		3	(5) (1st w/ clay						
3pt 3pr 3pr 3pr	14					65.678970123		3	by clst w/ clay	3 y 9 f					
3et 3et	14					65.678970123		3	by clst w/ clay						
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Depth	Graphic Log	SB-20 Lithology and Physical Condition	Well Construction Desail	Notes
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-80		Bhe Sh. Denvertm. W/INTBD SAND Lyen < 1: Toe 79'	S#	waler meas in pieg. @ 72'5"
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E	lev	7. 7 6. 961	of Rig Depth	one 55
		SUBSURFACE EXPLORATION LOG	Job No Date Ligare	

Dzeth	Graphic Log	53-21 Lithology and Physical Condition	Well Construction Desail	Notes
		SAND Y CLAX Med BROWN - 51 MOIST SAND UF-FORA.N. 52 CLYPEY, med. BROWN m- well 56 R-red.	CL	confort tell where structed & Drive @ 9
-10		CLY, med-ik. BROWN. SL. MOIST SL-MOD SANDX	SC	15/D"
-20 -		CLAX EMNGDE BROWN - GREY BROWN FRESTM. Lightle Layer & DRIC ELSY CLAY OKBROWN - GREYBROWN	CS	Poetr 15-29' Deve @ 29'-
-30		DK BEOMN - CKTH DEON -		18/12"
-40 - -				
50				
E	levi	7. 25-80	e of Rig al Depth ged By	CM2-55
	511	Industrial Compilance Inc. Orchard Street Goden, CO BOAG	Date	

Well Construction Desail 5B-21 Lithology and Physical Condition Notes - Danver Fm SH 160 HARD SS LAYERS, 21'
possible wales after 65' SH **7**0 Pull on 1 @ 29"
WAIT LHOURD FOR
WAY 9 - NO water 54 න 1' HARD 55. -possible water. V, SANDY - VFgr. Saturatel natera 100 Project _ Type of Rig Elevation _ Total Depth Date Drilled 7-25,-4 Logged By SUBSURFACE EXPLORATION LOG Job No Date

Bore Hole No. _____ 3 Sheet ___ of ___

			,
Depth Graphic Log	SB-∂∂ Lithology and Physical Condition	Well Construction Detail	Notes
-15 -2.0	Med-LT, BROWN SI MOIST. INCR MOIST SAND, SOME CLY, Med Brown F gramed m-W Siled water approx 11' SAND F med GRI. Nod, med Brown SICHYPY, West. BDRUE 17' GYBRN CLS MOIST-VMOIST TOCZO		DRING 17 = NATION (12) DRING 17 = NATION (12) PIEZO.
Pro)d Elev	ration T	ype of Rig otal Depth	20°
Date	Drilled L	ogged By _	-84U
	SUBSURFACE EXPLORATION LOG	Job No: Date:	
	Industrial Golden, Colorado Incorporated	Figure	

5B-23 Depth Lithology and Physical Condition Notes SAND/CLAY DRJ-SI moust مح 30eu ? 55 Simonst. 13C DRIVE 9/=
18/12" -10 CIS DK CLS MGYBEN-BEN, SL SNDY SL MOIST, TR. LIG. -90 MED GYBEN-BEN, TR. SNO) SL MOIST 18 NT FESTA. CCS DRIVER 29 = CLS 31/12" -30 Bemino DARNON,
31 MOIST -40 Daye 49 fter TDC 49' Type of Rig Me -55 Elevation ___ Total Depth Date Drilled 7-25-88 Logged By SUBSURFACE EXPLORATION LOG A1 140 Date Lyme

Beir Helt fin ___

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Depth	Graphic Log	Lithology and Physical Condition	8-24	Well Construction Desail	Naics
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160					
P E D	ro)e leva	ct 2 18/5 Islon	Total	of Rig. I Depth.	79'
		SUBSURFACE EXPLORATION LOG		Johnson Date Ligure	

Best Helt No . Sheet ____ (d ___ 80 -K 5B-25 Lithology and Physical Condition Notes Stroy cuty, med BROWN.
51 mois 7 75c DRIVE@91 = 12/12" Decreasing 81700

MOD. SANLY CLAY

med Brown N

Bedrock -10 OFF DR. ve@ 19'= 20/12" DK GREY BROWN- BROWN

FR. CARB MATRE

NOD - SL SINDY. ·90 Dear. STUD Droney BROWN - Drone Chty Sil moist, pass some Ly? Drine@ 29'= 26/124 -30 Benny med grey clay, -DRLG Essed up - POSS. water. Beming incr. moist \$ SLTY- SNOY WL @ 45 property. -be 491 50 Type of Rig CME-55
Total Depth 491 Elevation . Logged By _ SUBSURFACE EXPLORATION LOG Joh No Date Equire

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5B-26 Notes Lithology and Physical Condition Bedrock of STND - Driver 9'= 19/12', Festn. CLS ABNOT GYPSIM XLS some stroy rayers 21' By DRIG. CRS · Some organic matel. mad Geer-Gray brown CLAY CLS Poss Lig ezi coniton rext sheet Type of Rig Me 75 Project 2-1815 Elevation _ Total Depth Date Drilled 7-26-88 Logged By _ Job No: SUBSURFACE EXPLORATION LOG Date: Golden, Colorado Figure

Bore Hole No. -

5B-26 Notes City. Dank oray, tow restr DRIVE @ 29'= DH poss, water a 56 me 5 5 Type of Rig Elevation . Logged By Date Drilled . Job No: SUBSURFACE EXPLORATION LOG Date:

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100.17	TON OF E	OPING							JOB NO.	CLIEN			LOC	ATION	
LUCAT	ION OF B	- CHING							1		٠				
									DRILLING METH	OD				BORING	NO.
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DATU				····	<i>¥</i>	ELEVAT	ION	Leuber	CASING DEPTH	61-5am	61.5 ggs	415gm	"	1	<u> </u>
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	ION OF BORIN	<u>. </u>						OCATION
	J. J. DOMIN	BH 1	AND					Wagner Fain
_							DRILLING METHOD: CME-75, 4"	BORING NO
		0	8-1		22	_	sold sten sugars	J515-30
./		-		_	+	\sim 2	10.40	SHEET
N						U	SAMPLING METHOD: Calif SALIT SAL	7/0+4
Λ								DRILLING
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DATUM	4	•		Ε	LEVAT	ION	CASING DEPTH	1°3/28
	~ L	12 g	Fα		L.		SURFACE CONDITIONS: L+ br-br siv. 6:	sa w/one
SAMPLER TYPE	INCHES INCHES INCHES RECOVERED DEPTH OF CASING	SAMPLE	BLOWS/FT. SAMPLER		DEPTH IN FEET	SOIL	cky, dry - r	ecenth
SAR	CAP	SAMPLE	SAN		ΩŽ	0.49	diskal Field	
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-st	18	+	6				Br sic/ w/ some u ti sa s/.	no 5+
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got 3p	18	10	7 18	}		<u> </u>	Br sic/ w/ some u ti sa s/.	most
7p.	18	10	7 18	}		Y X	Br sic/w/ some v ti sa s/.	most
3p	18/19	10	7 /8	}		Y	Br sic/w/ some v ti sa s/.	nost
5p.	18/13	10	7 /8	7	1 2	Y L	Br si c/ w/ some v ti sa s/.	most
The state of the s	18/13	10	6 7 /8	,	/ O	¥ 4	Br sic/w/ some v ti sa s/.	ns, 5 f
3p.	18/13	10	7 /8	,	1 2		Br sic/w/ some u ti sa s/.	most-
3p.	18	/10			1 2 3		Br sic/w/ some v ti sa s/.	no st
Sp.	18/18	/10			1 2 3			noist
St.	18	15	1366) 0 1 2 3		Ltbr fi-me gt sa, 5/ moist	no st
5p.	18	10) 0 1 2 3		Ltbr fi-me gt sa, 5/most	ns.5 f
spt sp.	18	10			/ 0 1 2 3 4)5		Litbr fi-me gt sa, s/moist	no 5 f
5pt - 5p	18	15) 0 1 2 3		Ltbr fi-me gt sa, 5/most	no 5
spt sp	18	15			/ 0 1 2 3 4)5		Ltbr fi-me gt sa, 5/moist	no 5
57t 5p	18	15) 0 1 2 3 4)5 6		Lit by fi-me gt sa, s/moist trace of chy 16.2-16.5'	
5p	18	15) 0 1 2 3 4)5 6		Ltbr fi-me gt sa, 5/moist	vo sished

LOC	CATI	ON OF B	ORING							JOB NO CLIENT LOCATION
										BORING NO.
										DRILLING METHOD SB-30
										SHEET
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										SAMPLING METHOO: DRILLING
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				,						CASING DEPTH
	TUM			T= /		E	ELEVA	TION	'	SURFACE CONDITIONS:
LER	TYPE	INCHES DRIVEN S	DEPTH OF CASING	SAMPLE	BLOWS/FT. SAMPLER		Ŧ	-	표	
AMP.	בֿ	INCHES INCHES RECOVERED	EPTI CASI	SAMPLE	AME		DEPTH	z	SOIL GPAPH	
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	_	10		/A0	al -			Ж	ML	(i.e. calcite?) dry-st morst
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€ √.	77						1~		C25	14 to - dV b - sixtisael , 5/ most
9	30	18		25	9 (2)	bag	25	H'	کام	some calcite & iron stracks
5/3	p	18		25	90	bag	1	\mathbb{H}	25	
16	g.	18		25	9 (2)	bag	≥ s	\mathbb{H}	کاح	
16 J	P	18		25	901	bag	1		ک	
	}ρ	18		25	9 (2)	bag	6		225	
19	β	18		55	9 (2)	bag	6		25	some ration & iron strongs
19	β	18		25	9 (2)	609	6		C15	some ration & iron strongs
	βp	18		25	9 (2) /2	bag	6		CB -/	some rate to x iron strongs
		18		(51)	36,		6		cis ~/ as	Hit lock @ 29 thous 2/ thick - probably comp. sol
	p Bp	18			36,		6 7 8			Hit lock @ 29 thous 2/ thick - probably comp. sol
		18		(51)	36,		6 7 8			Some rakito x iron strongs Hit lock @ 229 how 2 / thick - probably cong. sol
		18		(51)	36,		6 7 8 9		cus	Some rate: he x iron strongs Hit lock @ 29' thin & 2 / thick - probably comp. some level of a Clayste Granish-Gy Cls w/ some thin v.f. sa to string- Singles to iron standy v.f. sa (neath. 5.5) @ 31.2'
		18		(51)	36,		6 7 8 9			Some rate: he x iron strongs Hit lock @ 29' thin & 2 / thick - probably comp. some level of a Clayste Granish-Gy Cls w/ some thin v.f. sa to strong- Singles to iron standy v.f. sa (neath. 5.5) @ 31.2'
		18		(51)	36,		6 7 8 5 5		cus	Some rakito x iron strongs Hit lock @ 29' than & 2 / thick - probably comp. sold lend w/m claysh Greenish-by cls w/ some thin v.f. sa to string- Line - 5/ moist Grades to iron stained yv. fi sa (weeth. 5.5) @ 31.2' b-yell
		18		(51)	36,		6 7 8 5 5		cus	Some rate: he x iron strongs Hit lock @ 29' thin & 2 / thick - probably comp. some level of a Clayste Granish-Gy Cls w/ some thin v.f. sa to strong- Singles to iron standy v.f. sa (neath. 5.5) @ 31.2'
		18		(51)	36,		6 7 8 8 5 5 6		cus	Some rakito x iron strongs Hit lock @ 229 how 2 / thick - probably cong. sold land of a Claysta Something V. F. sa to strong- Grades & iron standy v. F. sa (weeth. 5.5) @ 31.2' Lange cutty iron stand 5i sa
0	12/2)	9",4		(5)	36 70/2"		6 7 8 8 5 5 5 6 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	2	cus	Some rakito x iron strongs Hit lock @ 29' than & 2 / thick - probably comp. sold lend w/m claysh Greenish-by cls w/ some thin v.f. sa to string- Line - 5/ moist Grades to iron stained yv. fi sa (weeth. 5.5) @ 31.2' b-yell
		9",4		(5)	36 70/2"		6 7 8 8 5 5 5 6 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	2	cus	Some rakito x iron strongs Hit lock @ 229 how 2 / thick - probably cong. sold land of a Claysta Something V. F. sa to strong- Grades & iron standy v. F. sa (weeth. 5.5) @ 31.2' Lange cutty iron stand 5i sa
	12/2)	7.8		(5)	36,		66 7 8 8 9 9 1		cus	Some rakito x iron strongs Hit lock @ 229 how 2 / thick - probably cong. sold land of a Claysta Something V. F. sa to strong- Grades & iron standy v. F. sa (weeth. 5.5) @ 31.2' Lange cutty iron stand 5i sa
0	12/2)	9",4		(5)	36 70/2"		66 7 8 8 9 9 1	2	cus	Some rakito x iron strongs Hit lock @ 229 how 2 / thick - probably cong. sold land of a Claysta Something V. F. sa to strong- Grades & iron standy v. F. sa (weeth. 5.5) @ 31.2' Lange cutty iron stand 5i sa
0	12/2)	9",4		(5)	36 70/2"		6 7 8 8 5 5 3 5 6 3 5		cus	Some rakito x iron strongs Hit lock @ 229 how 2 / thick - probably cong. sold land of a Claysta Something V. F. sa to strong- Grades & iron standy v. F. sa (weeth. 5.5) @ 31.2' Lange cutty iron stand 5i sa
	12/2)	9",4		(5)	36 70/2"		6 7 8 8 5 5 3 5 6 3 5		cus	Some rakito x iron strongs Hit lock @ 229 how 2 / thick - probably cong. sold land of a Claysta Something V. F. sa to strong- Grades & iron standy v. F. sa (weeth. 5.5) @ 31.2' Lange cutty iron stand 5i sa
	12/2)	9",4		(5)	36 70/2"		88 99 30 11 22 23 24 24 24 24 24 24 24 24 24 24 24 24 24		cus	Some rakito x iron strongs Hit lock @ 229 how 2 / thick - probably cong. sold land of a Claysta Something V. F. sa to strong- Grades & iron standy v. F. sa (weeth. 5.5) @ 31.2' Lange cutty iron stand 5i sa
	12/2)	9",4		(5)	36 70/2"		6 7 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	2 3 4 5 7 8	cus	Some rakito x iron strongs Hit lock @ 229 how 2 / thick - probably cong. sold land of a Claysta Something V. F. sa to strong- Grades & iron standy v. F. sa (weeth. 5.5) @ 31.2' Lange cutty iron stand 5i sa
	12/2)	9",4		(5)	36 70/2"		6 7 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	2 3 4 5 6 7	cus	Some rakito x iron strongs Hit lock @ 229 how 2 / thick - probably cong. sold land of a Claysta Something V. F. sa to strong- Grades & iron standy v. F. sa (weeth. 5.5) @ 31.2' Lange cutty iron stand 5i sa
0	12/2)	9",4		(5)	36 70/2"		6 7 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9		cus	Some rakito x iron strongs Hit lock @ 229 how 2 / thick - probably cong. sold land of a Claysta Something V. F. sa to strong- Grades & iron standy v. F. sa (weeth. 5.5) @ 31.2' Lange cutty iron stand 5i sa

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	LOCAT	ION OF I	ORING	•						JOB NO	CLIEN	1		Ιcα	ATION	
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															SHEET	<u>, l</u>
										SAMPLING METI	10D:				3	or 4
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										WATER LEVEL	48	48.2		 	-	163
										TIME	1530	8/24/98		+	DATE	DATE
				,				•		CASING DEPTH	8/23/88	0/04/00 0/50mm			1	DATE 8/23/8
	DATU	,		15e /	F ~		ELEVATI	ON	SURFAC	E CONDITIONS:	131.30	20 2200			4	
	SAMPLER	INCHES DRIVEN S	DEPTH OF CASING	SAMPLE	BLOWS/FT. SAMPLER		DEPTH IN FEET	SOIL								
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LOCAT	TION OF E	BORING			i				JOB NO.	CLIEN	Τ ,		LOCATION	
					•				DRILLING METH	10D:	<u> </u>		BORING	
											· ·		5/5-	30
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			** ***	٠.,					SAMPLING MET	HOD:			7	of 4
				•				į	1				DRII	LLING
							•				T	I	START	FINISH
									WATER LEVEL	170.1	48.1			2920
									TIME	0820	0940		DATE	
		•	,						DATE		925/82			B/25/
DATU		— т	. 1		F	ELEVAT	ION	SURFAC	CASING DEPTH	65 open	75 open			1 /6
E 11	INCHES DRIVEN S	မီ ပွ	SAMPLE	/FT.		ᆂᆸ] [±]							
SAMPLER TYPE	INCHES DRI- THCHES RECOVERED	DEPTH OF CASING	~ ZE	BLOWS/FT. SAMPLER		DEPTH IN FEET	SOIL		<u> </u>					
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Spt	14		15	27 48	Grab	1 1	54	Br	fi sa a	A g	ty sa	u/ abu	n dk ma	hix
Spt	14		165	27 48 60/6"	Grab	1 1		Br.	fisa a	<i>\$</i> 8' - 0	tz sa i	u/abu	in dk ma	bris 1
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			35-3	5		30 1 2 3		Grades to be si c/ 33.5-3 dry, skun fossilized /2000s, to Broy claystons, iron stormed dry 34.	4.5 wigs	
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			35-3	6		30 1 2 3	U/S	Grales to be si c/ 33.5-3 dry, seun fossilized leones, to Recy claystone, iron stained by 34. 35-35.7' Jkbr rom stained fraystone, dr. 35.7-35.9'- /+ be sive fight so, dry 35.9-37.3'- Saystone and - sillstand	4.5 wigs	£
			1 3 - 3 - 3 - 3 - 3 - 3 - 3 - 3 - 3 - 3	6		30 1 2 3 4 35	U/S	Grales to be si c/ 33.5-3 Ary, skun fossilized leaves, to Aroy claystone, iron storned dry 34, 35-35.7' dk be ron storned dry 34, 35.7-35.9'- 1+ be sive fights so, dry 35.9-37.3'- Staystone, and - sills to a some iron storney	4.5 wigs 5-3;	£
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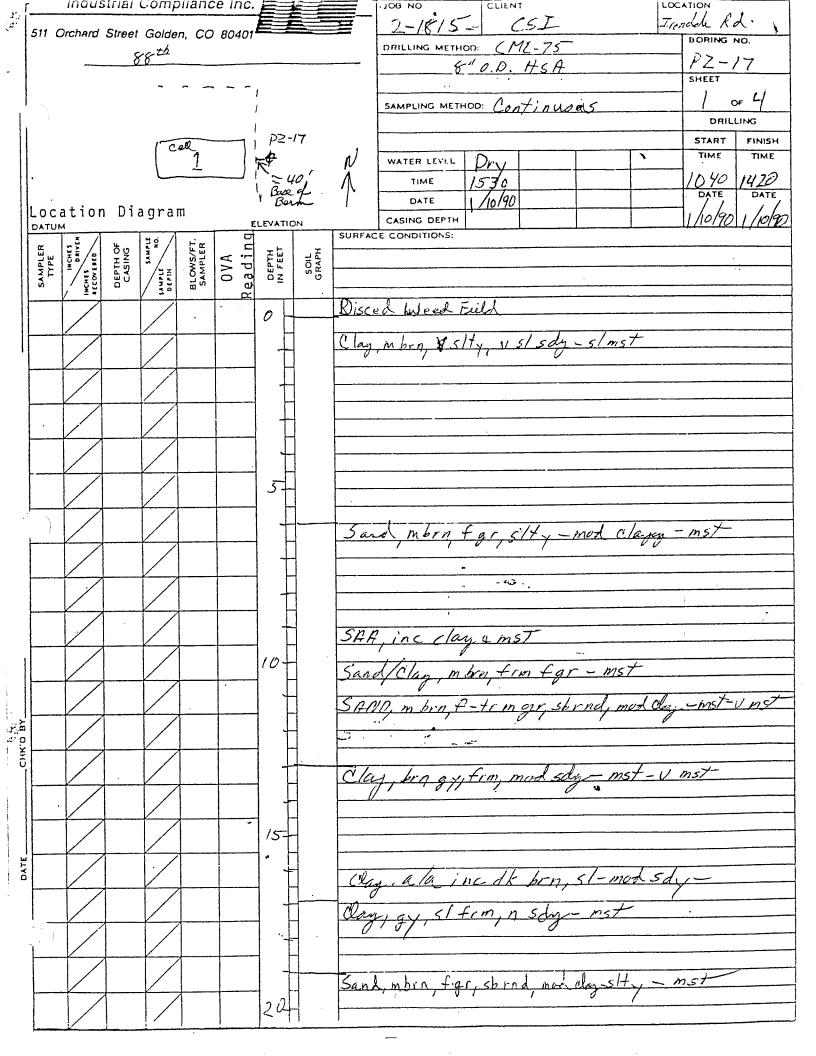
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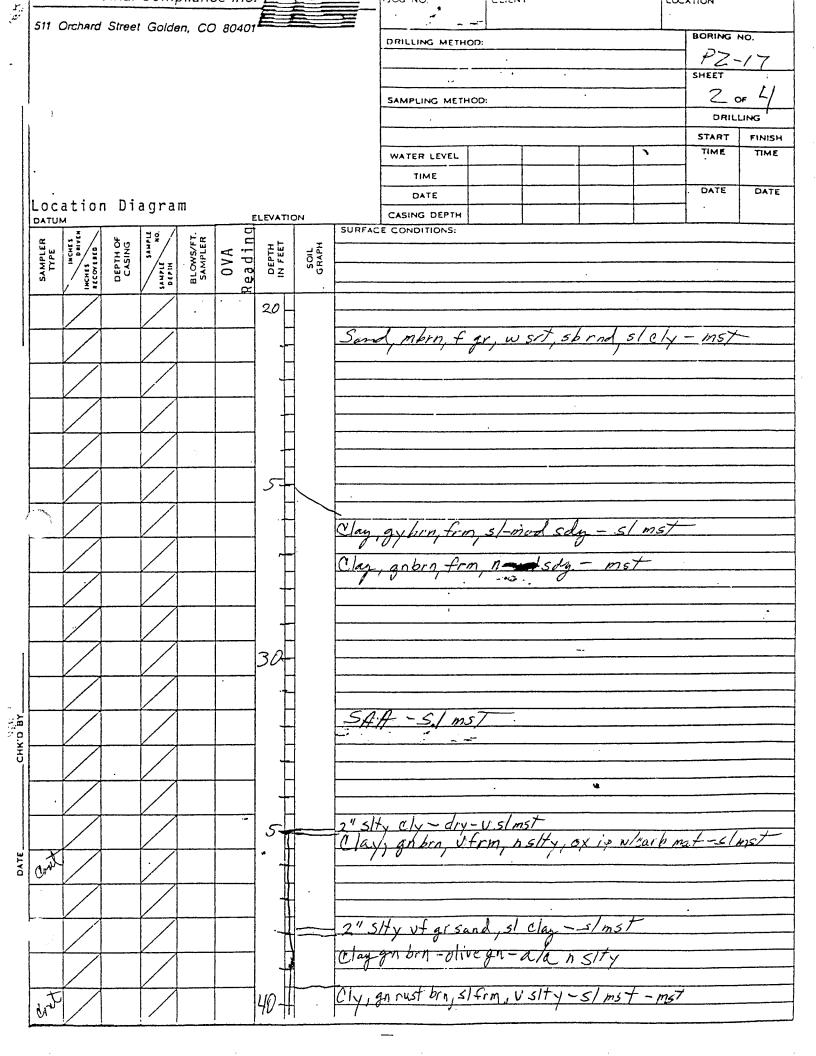
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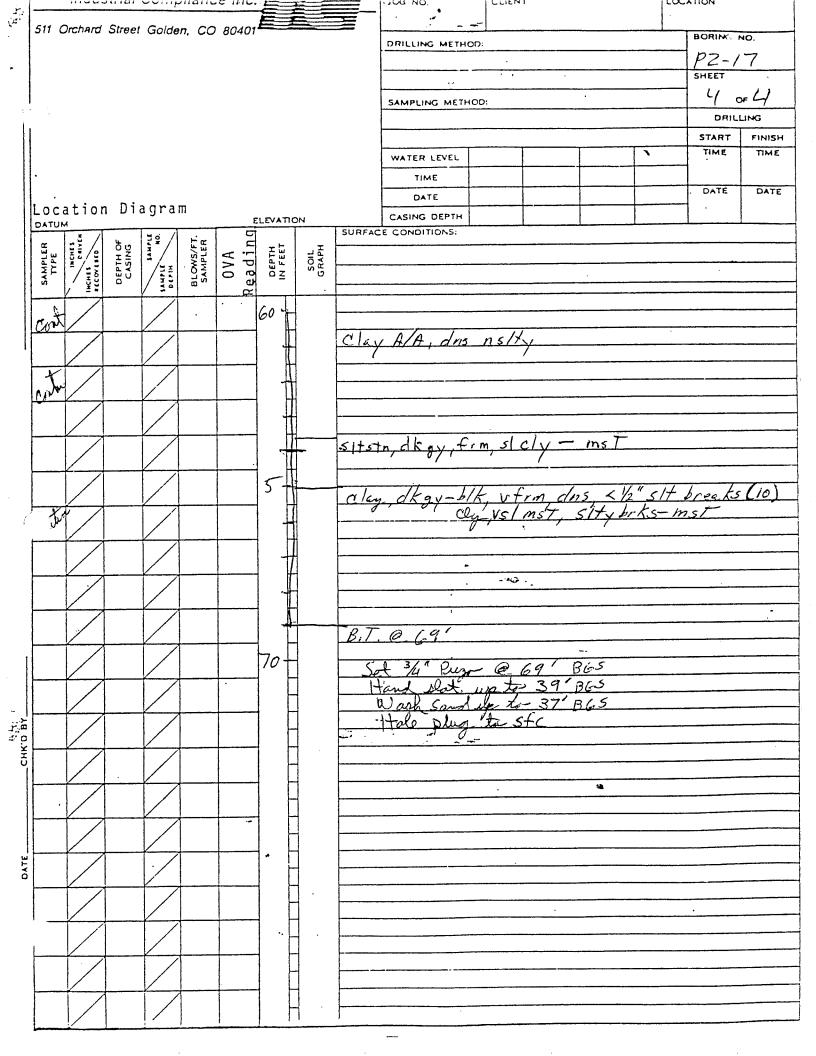
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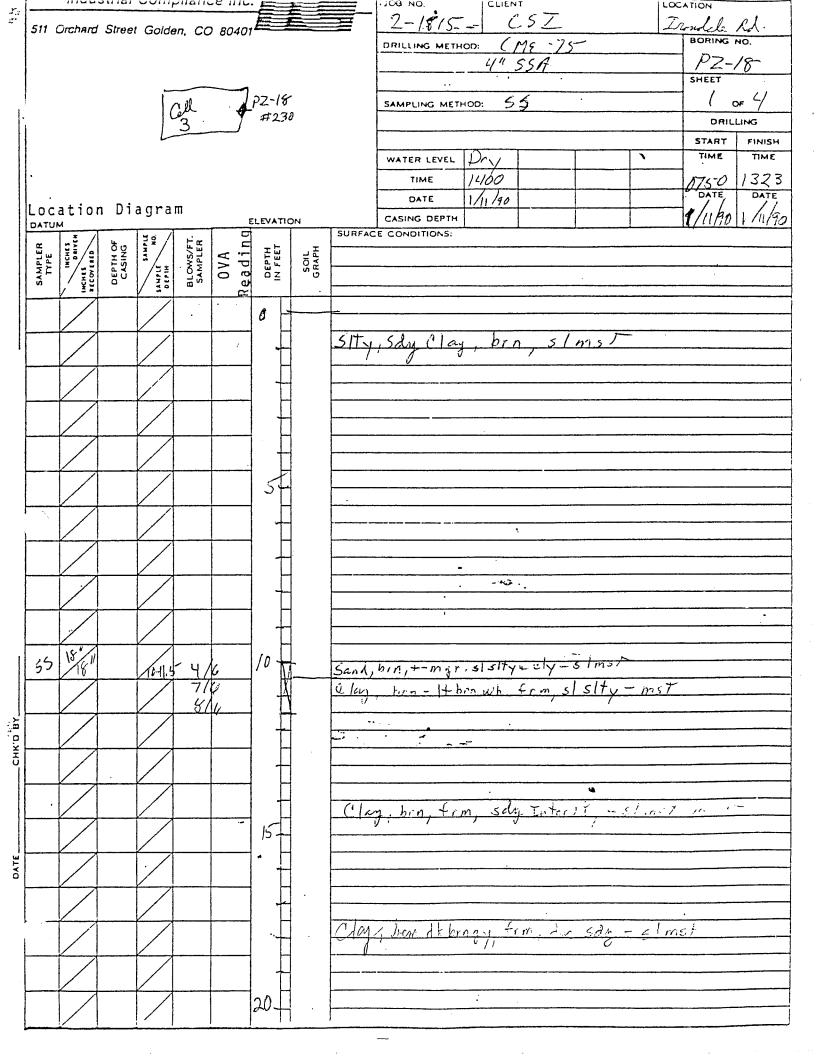
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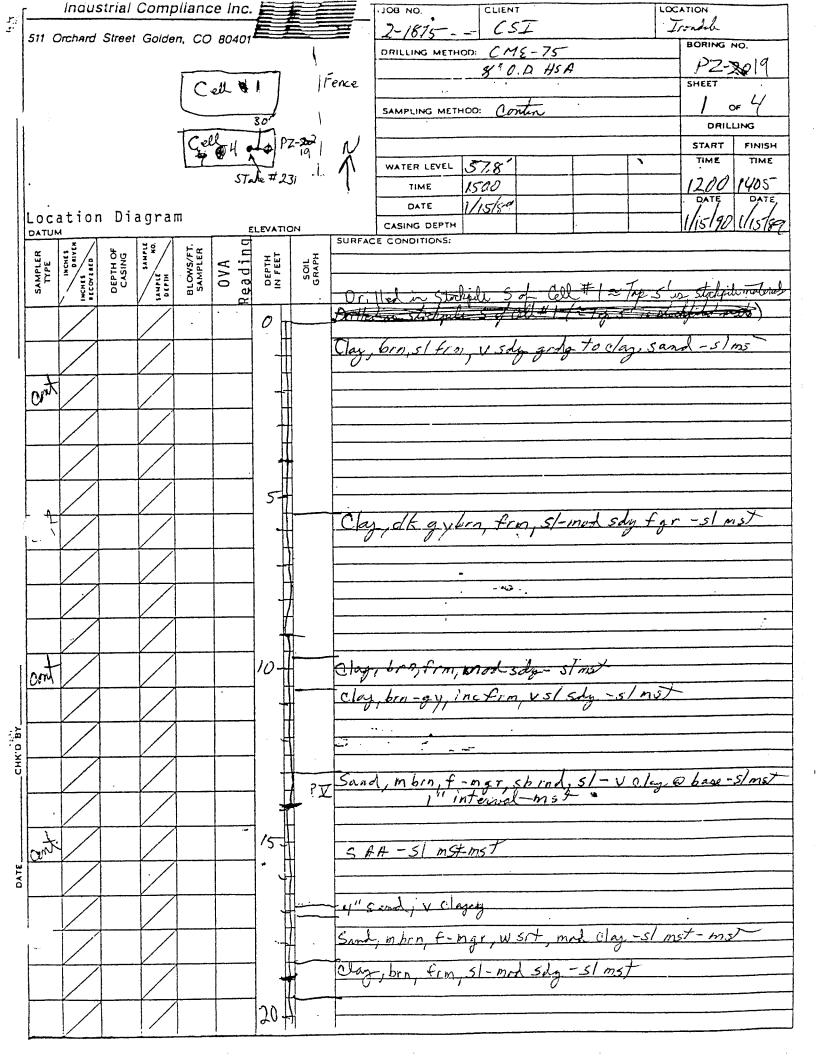






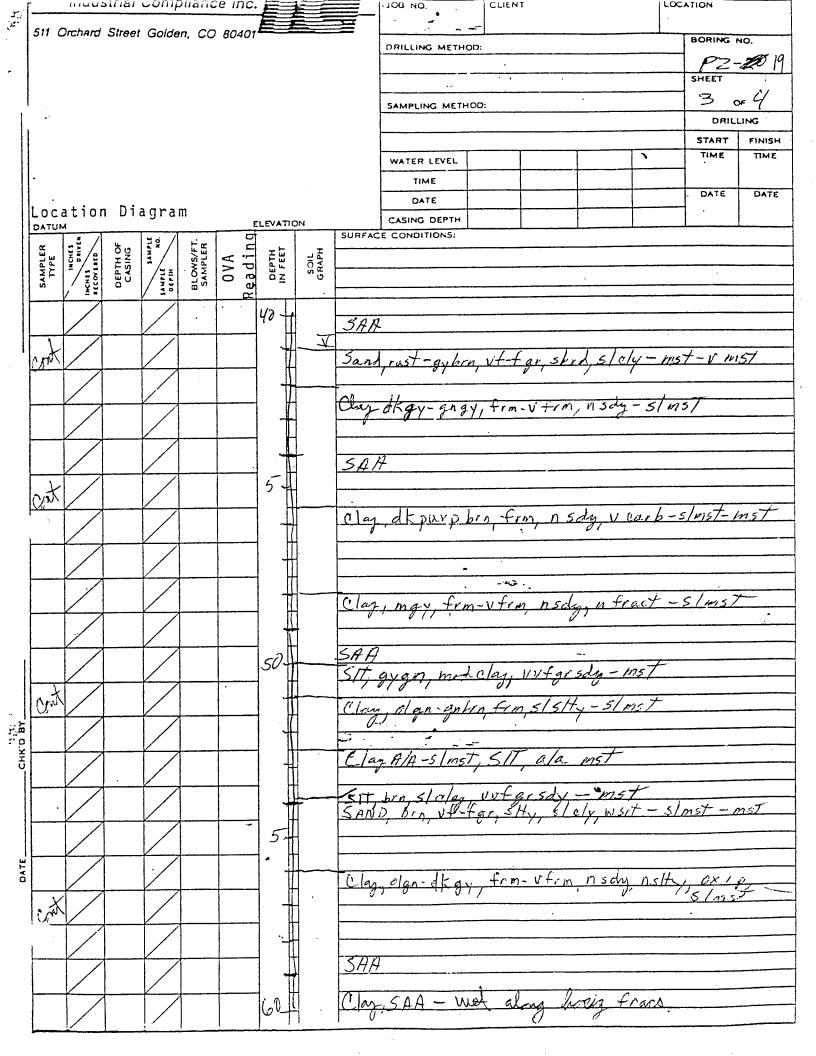


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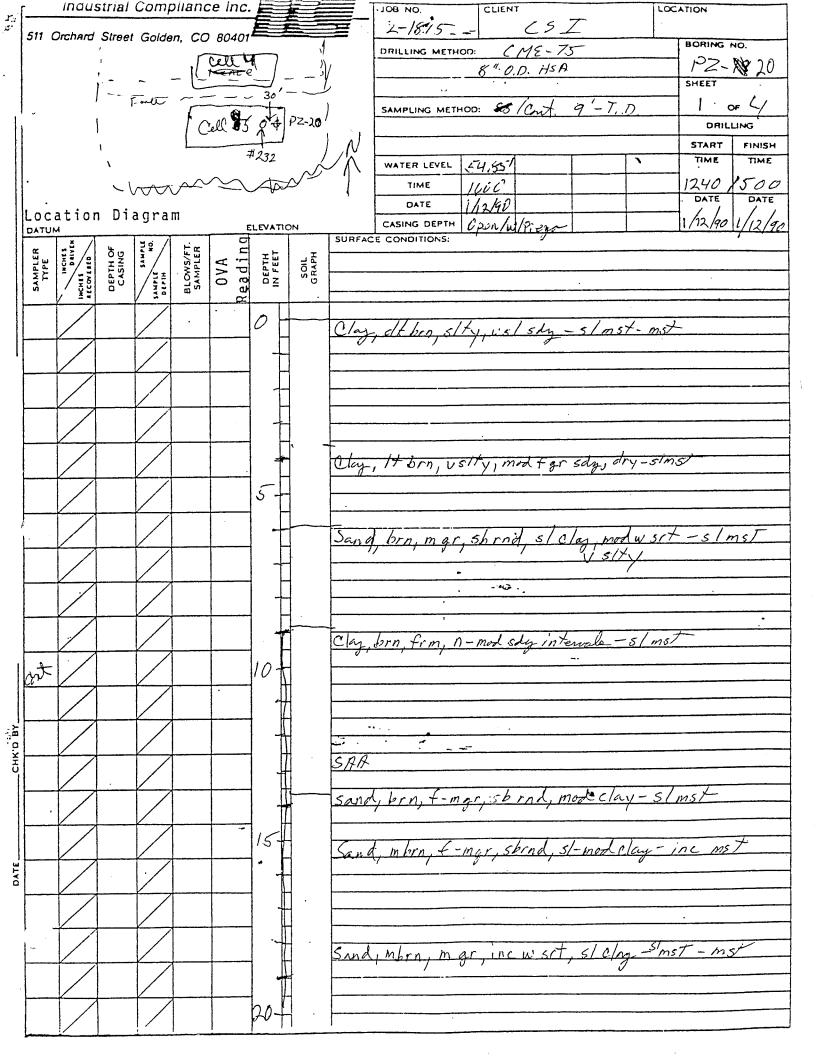


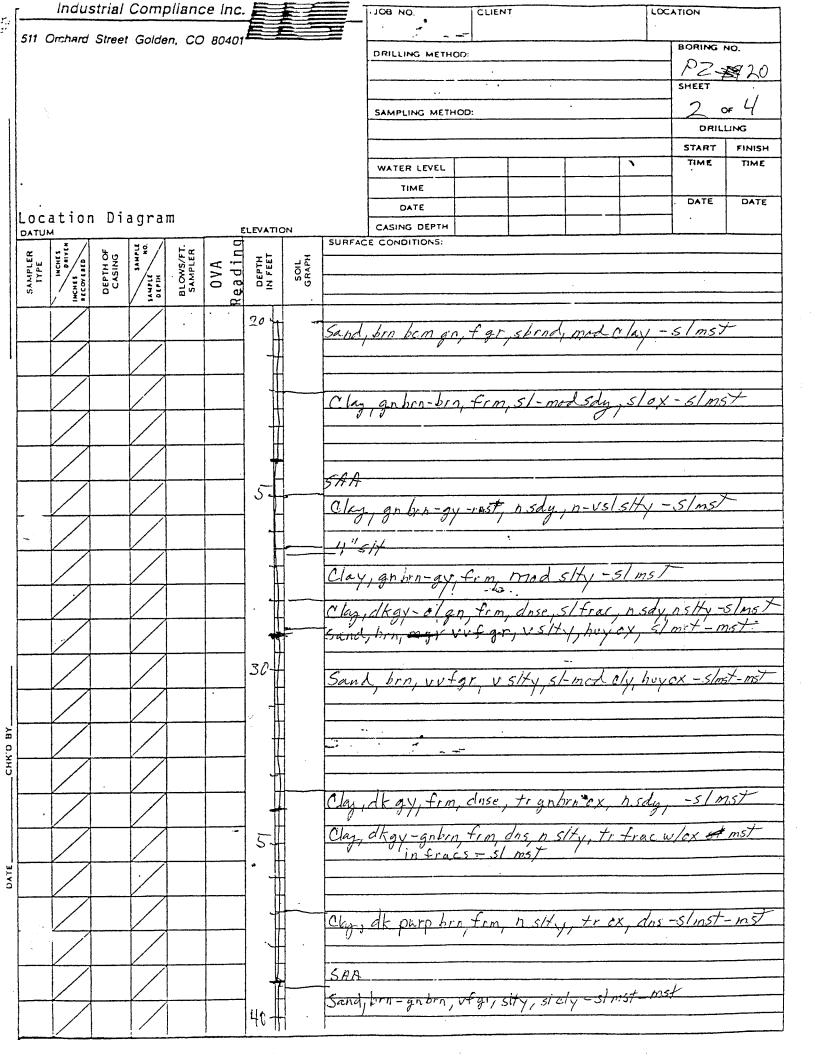
inaustrial Compliance Inc. CLIENT LOCATION JOB NO. 511 Orchard Street Golden, CO 80401 BORING NO. DRILLING METHOD: 2 or 4 SAMPLING METHOD: DRILLING START FINISH TIME WATER LEVEL TIME DATE DATE DATE Location Diagram CASING DEPTH ELEVATION SURFACE CONDITIONS: Sand, brn, mgr, shrnd, mod w sit, mod clay - MIST SAH, a/a inc clayey SAND/V clay, gray to clay - strist

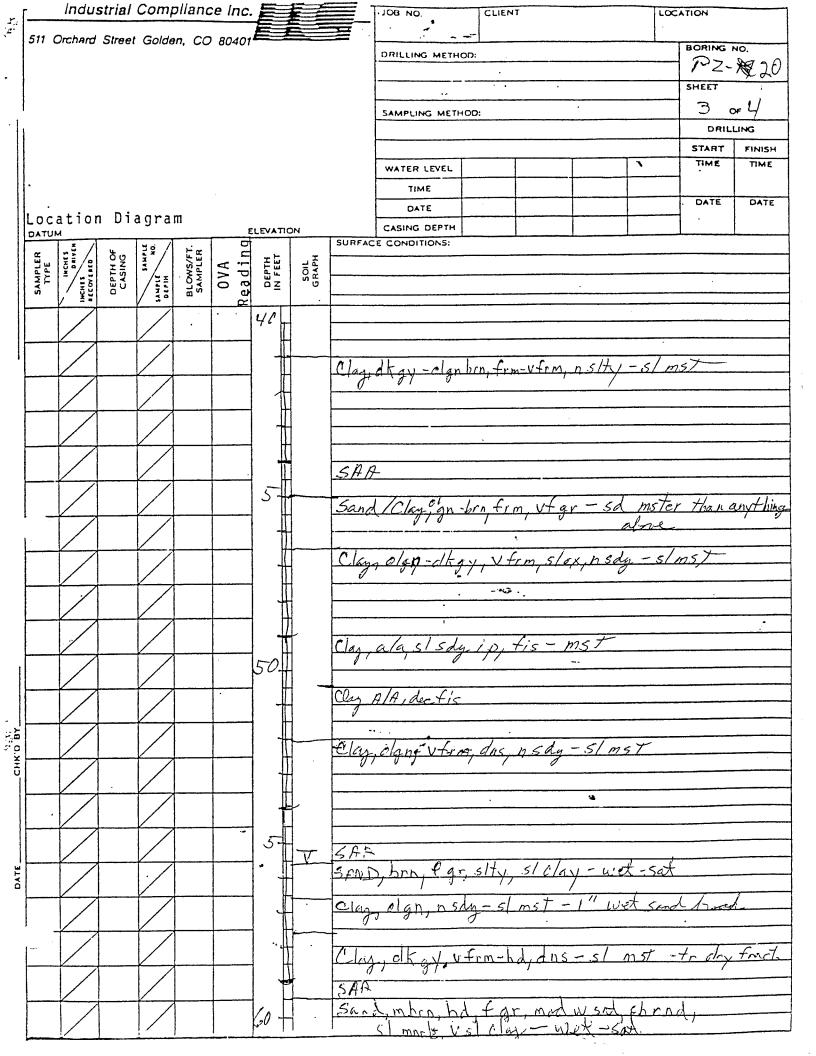
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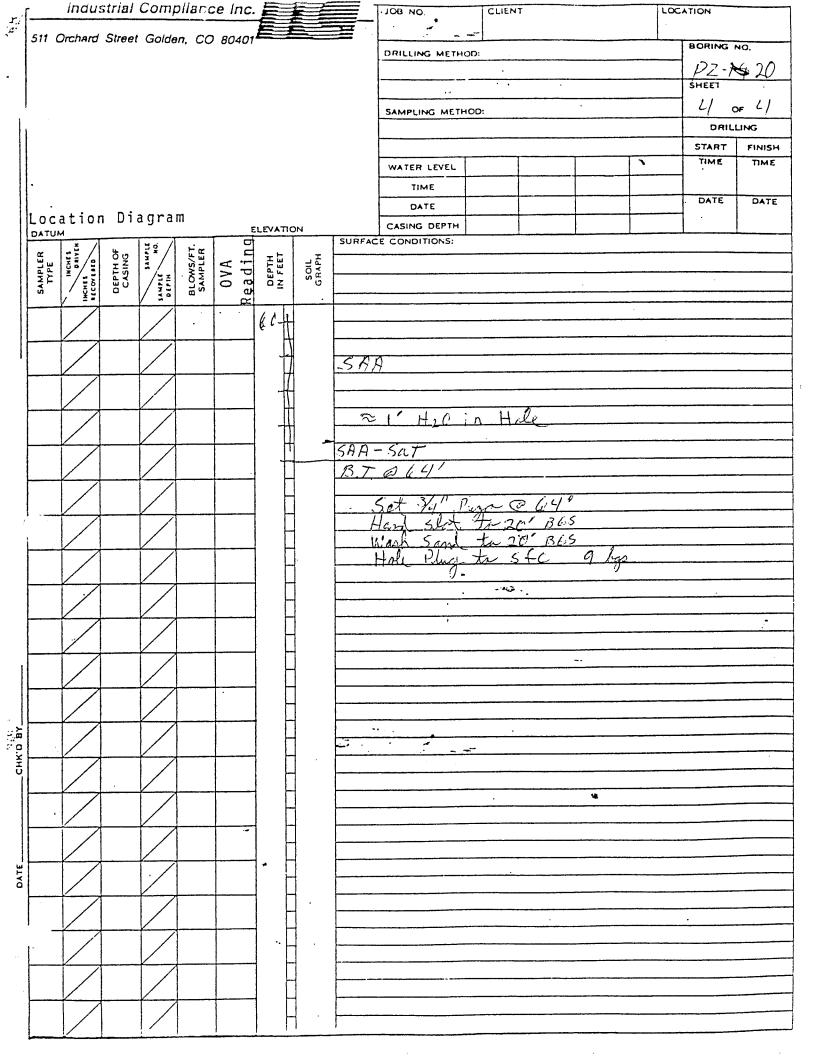


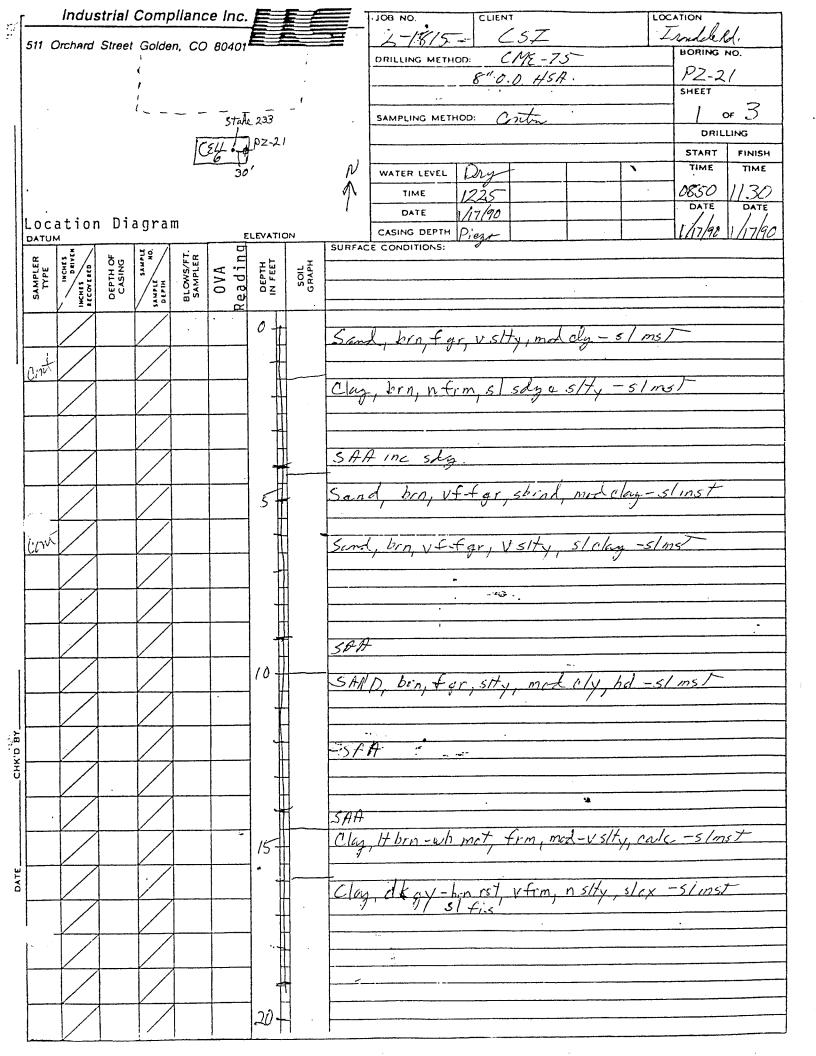
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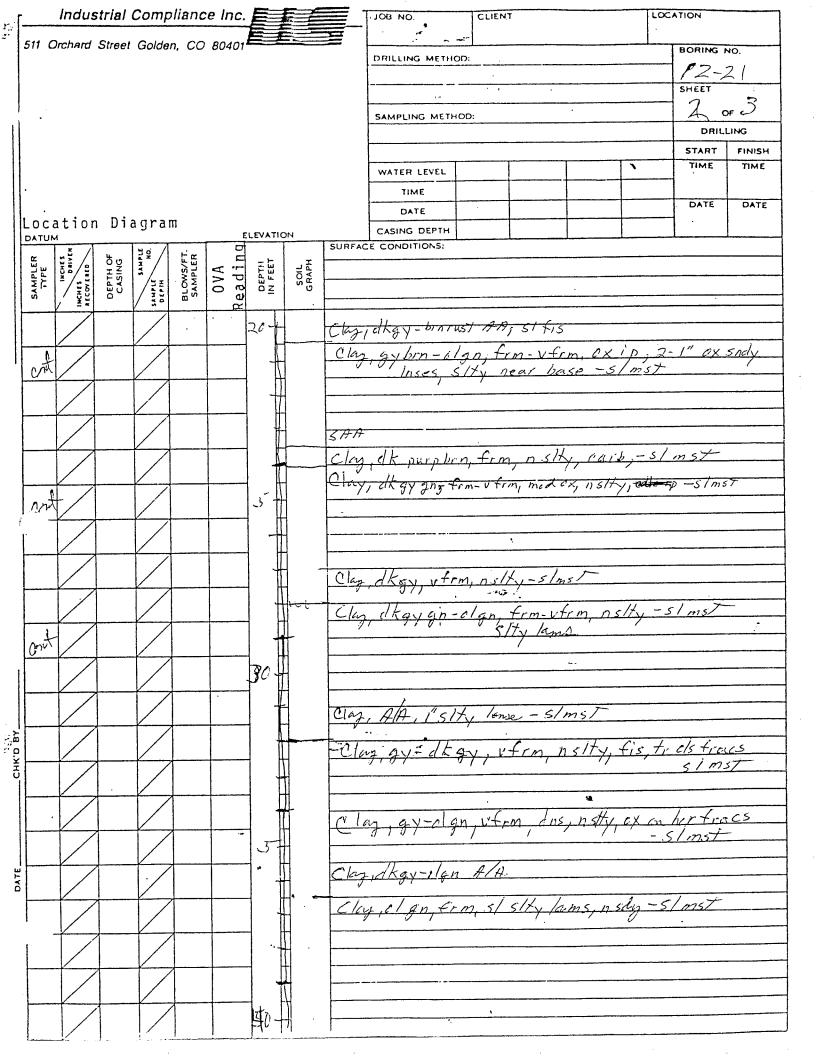


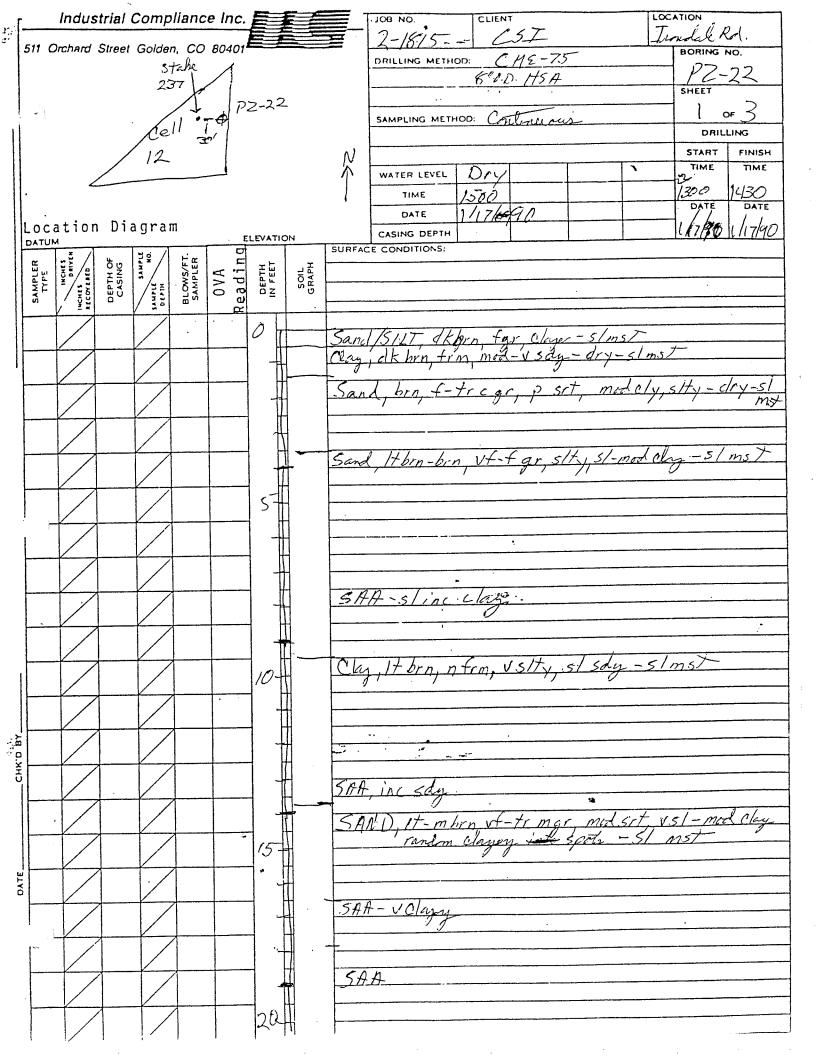


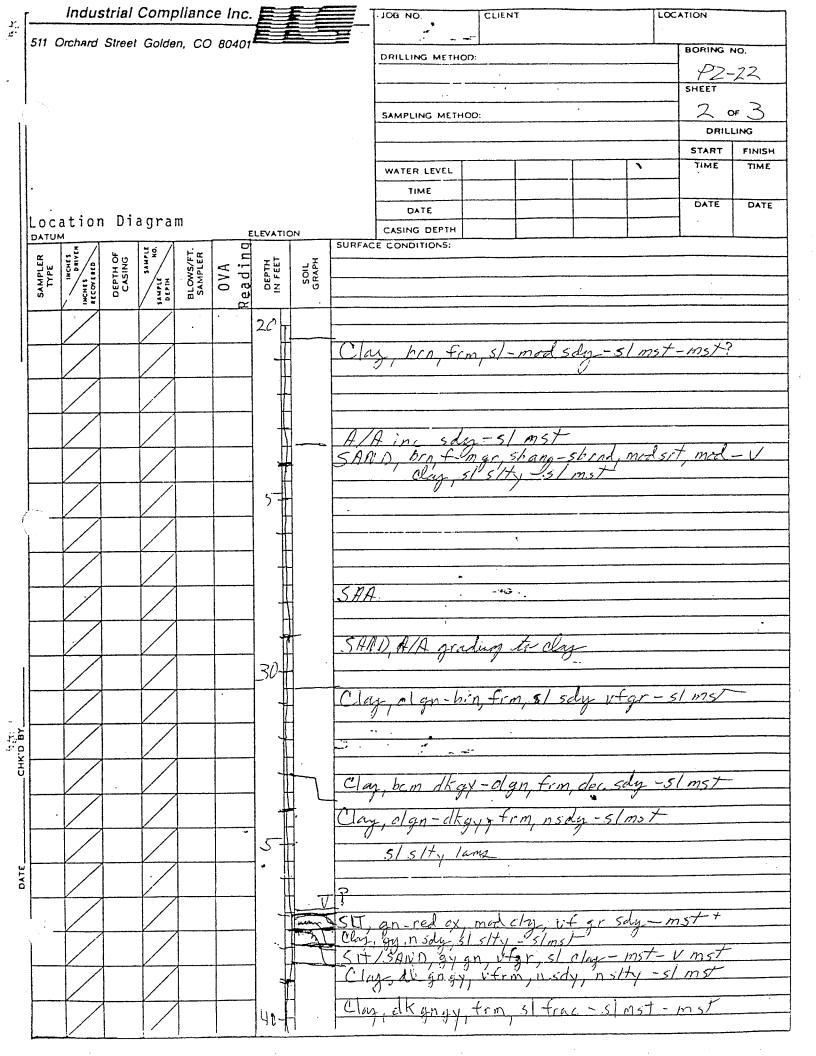


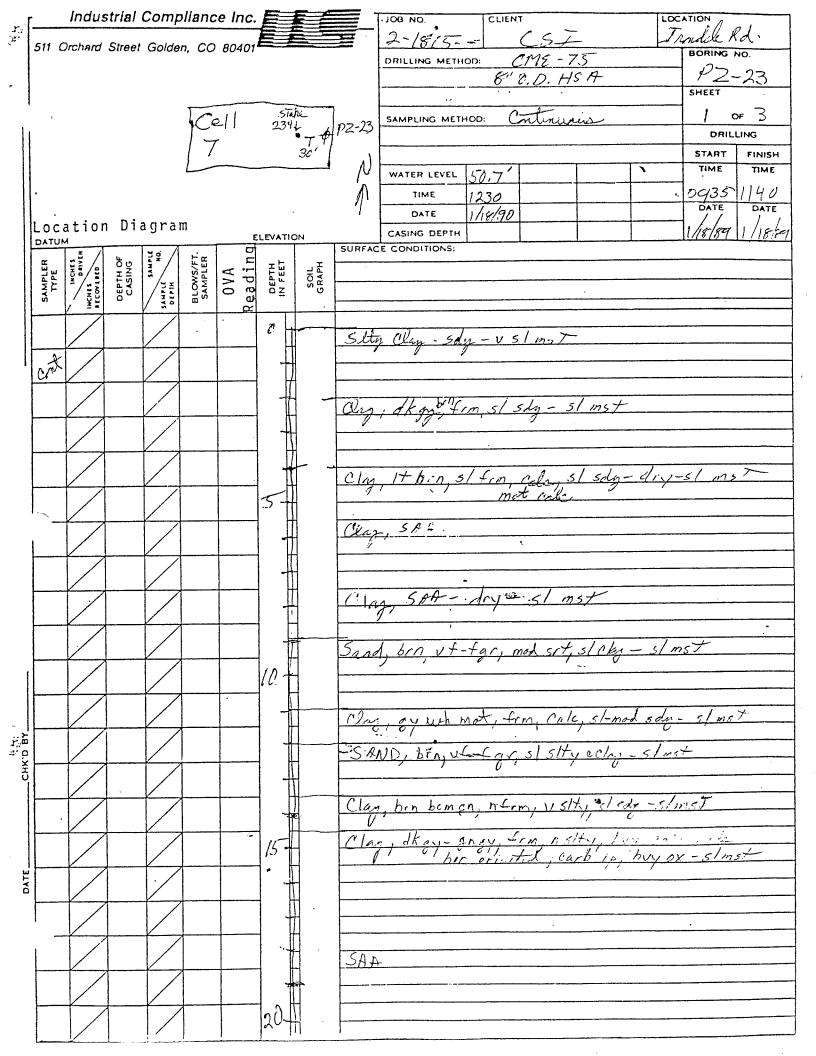




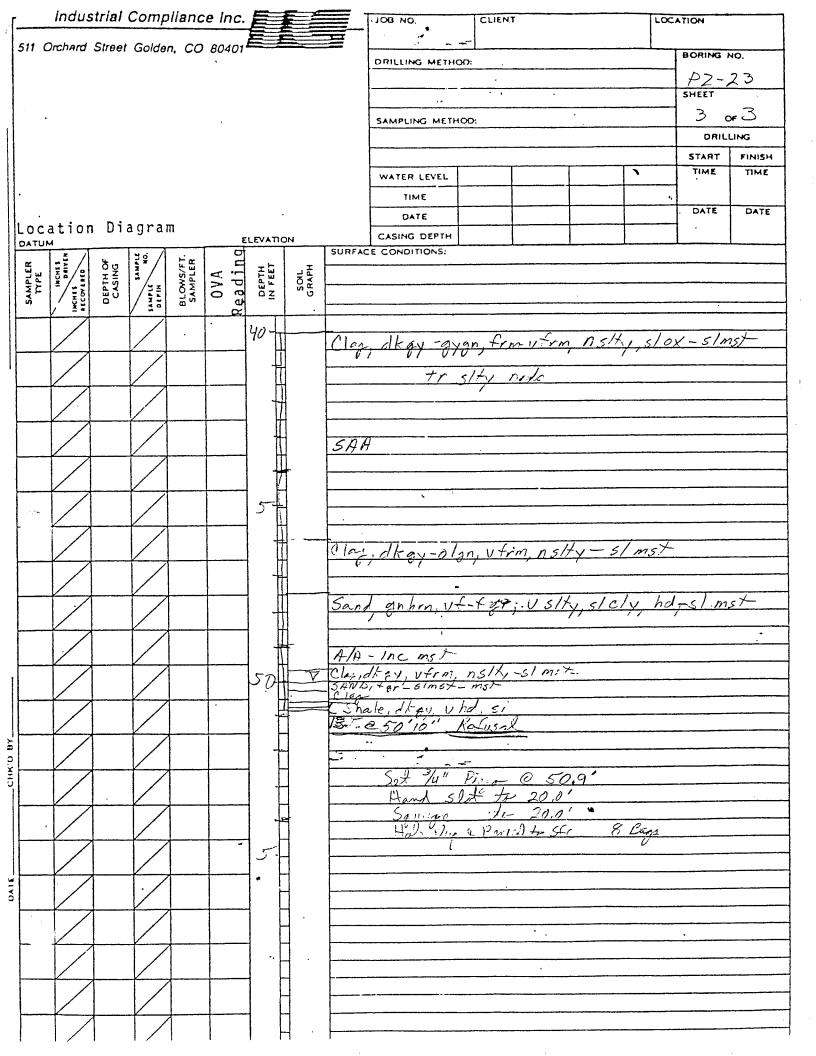


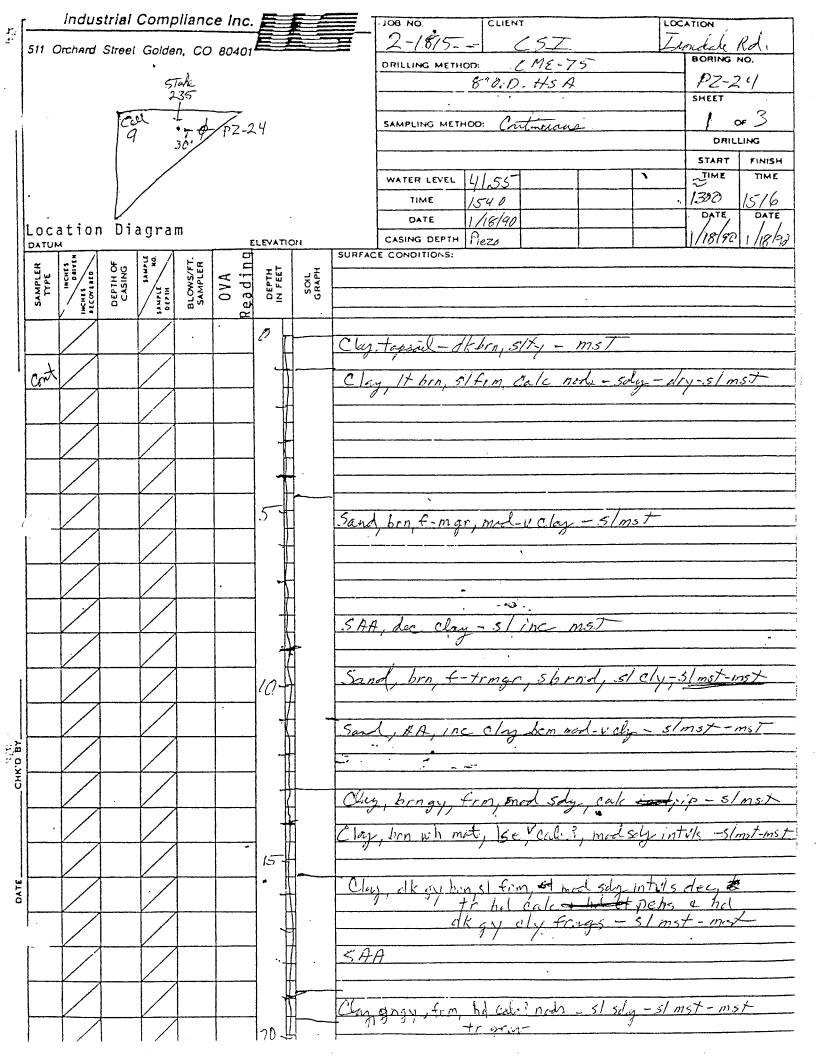


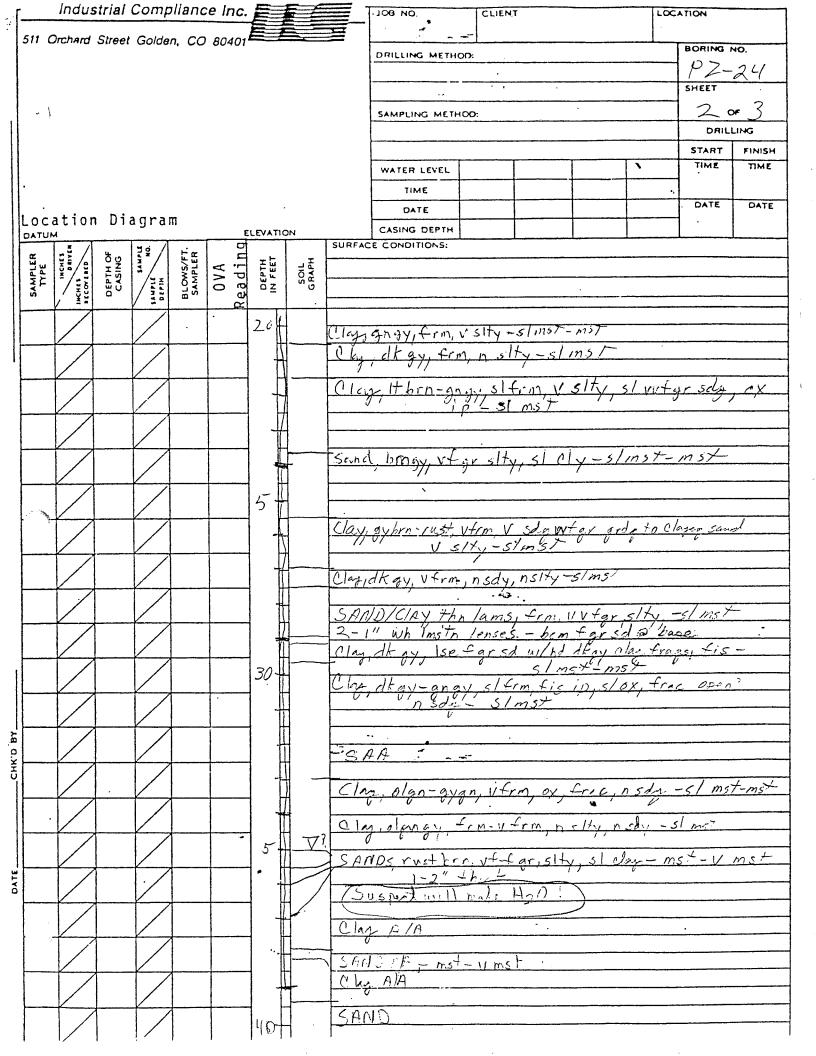




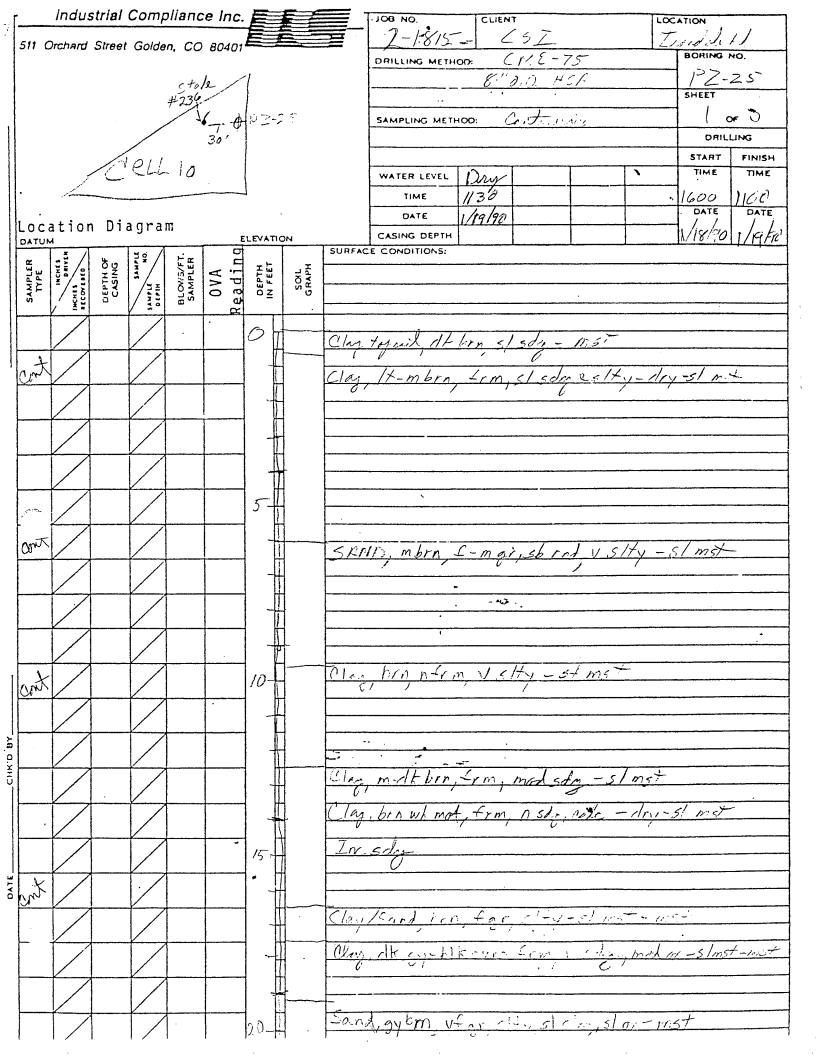
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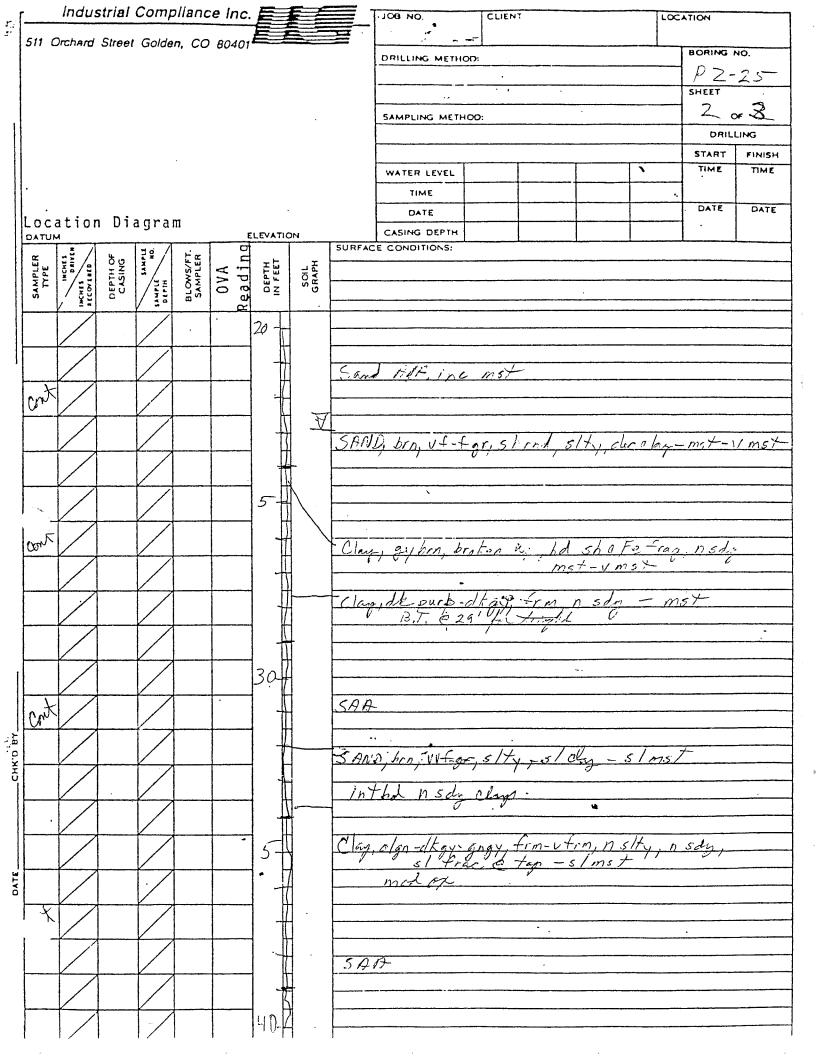


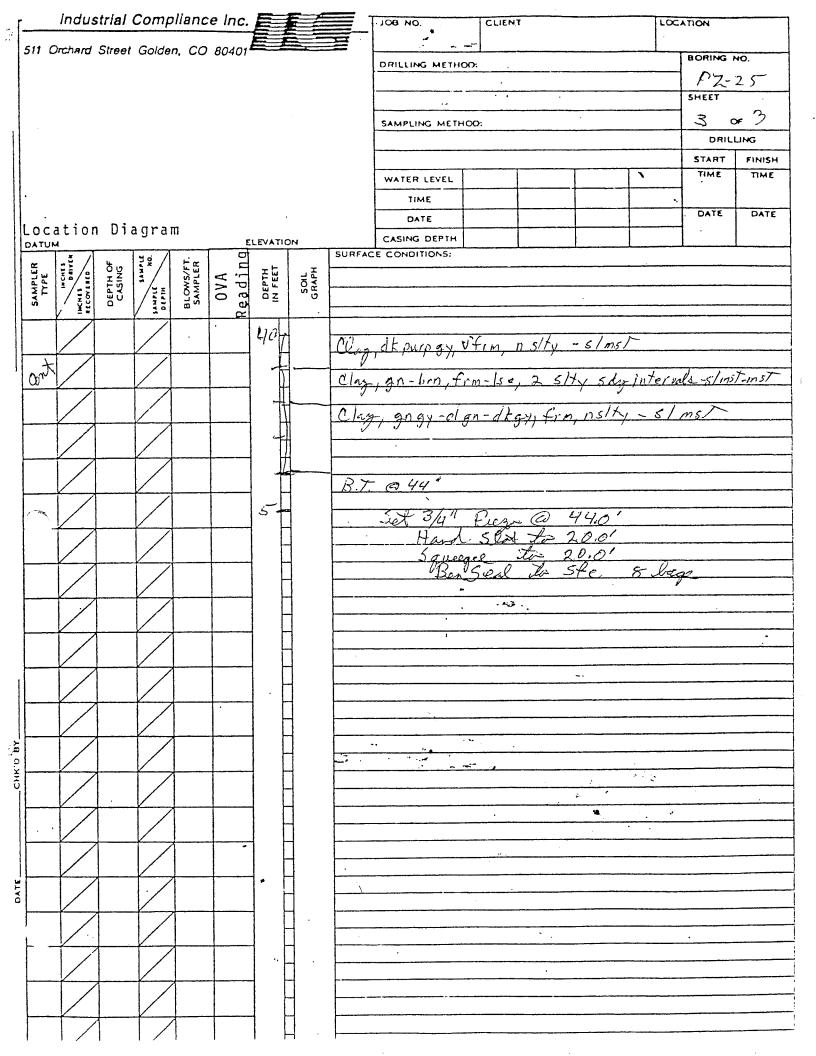


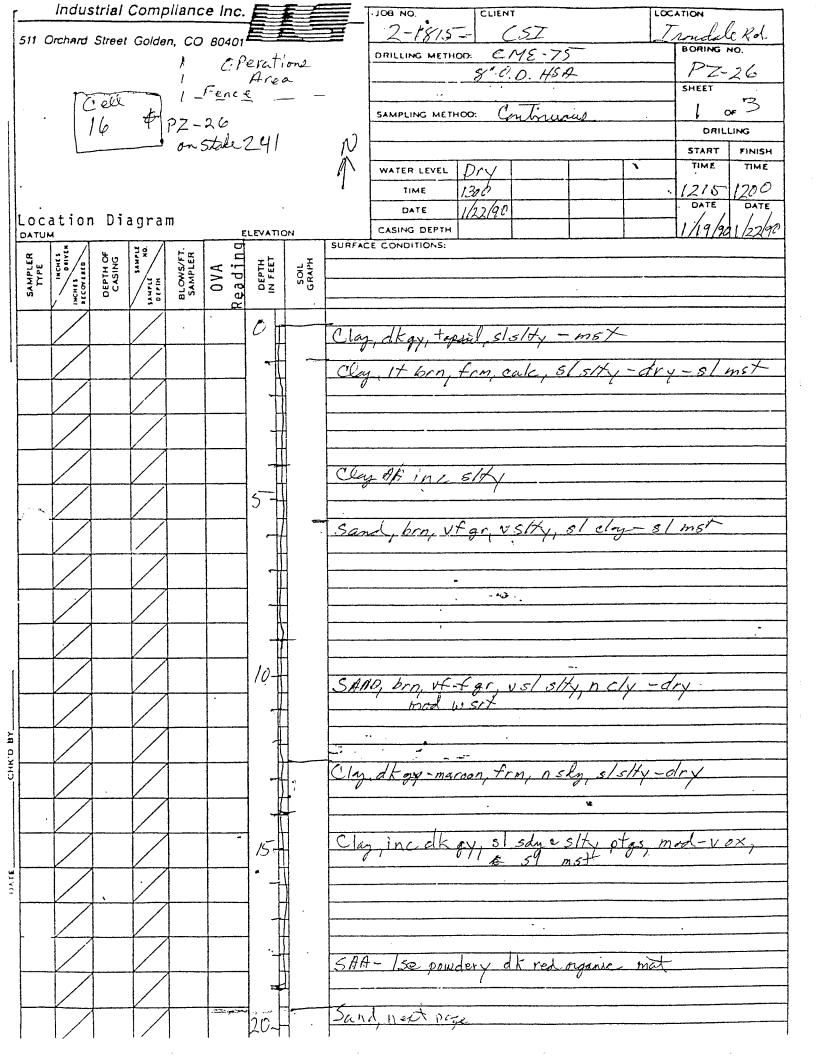


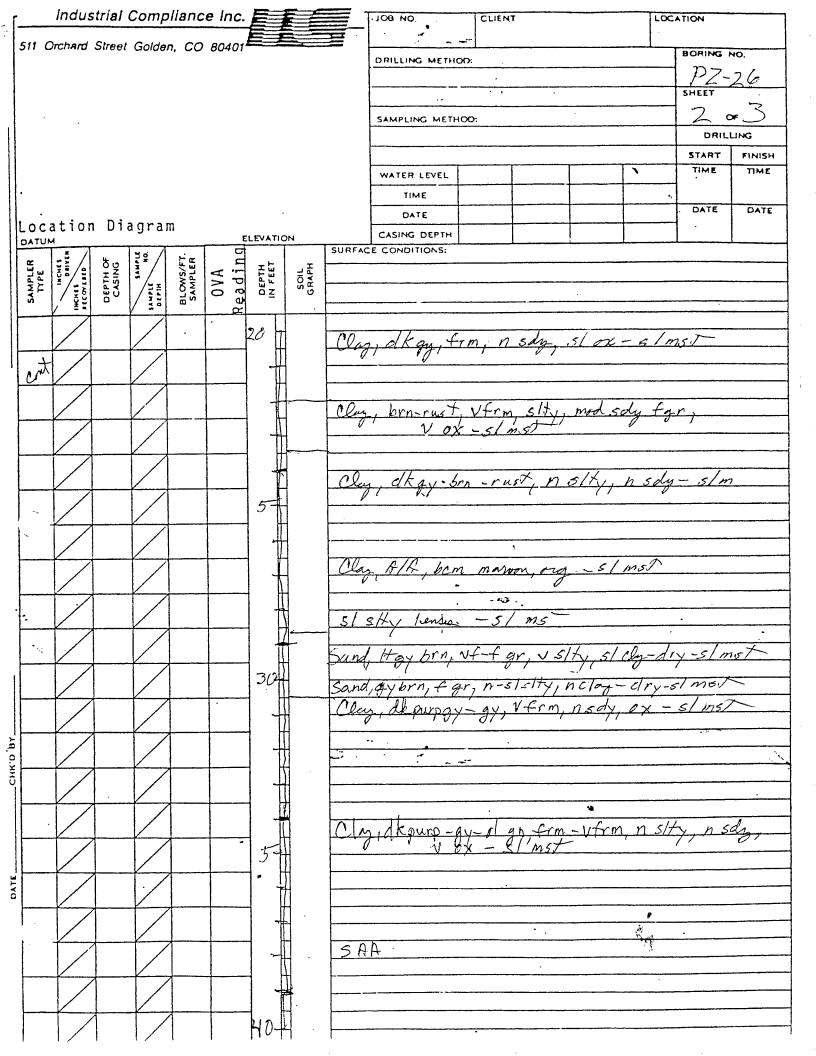
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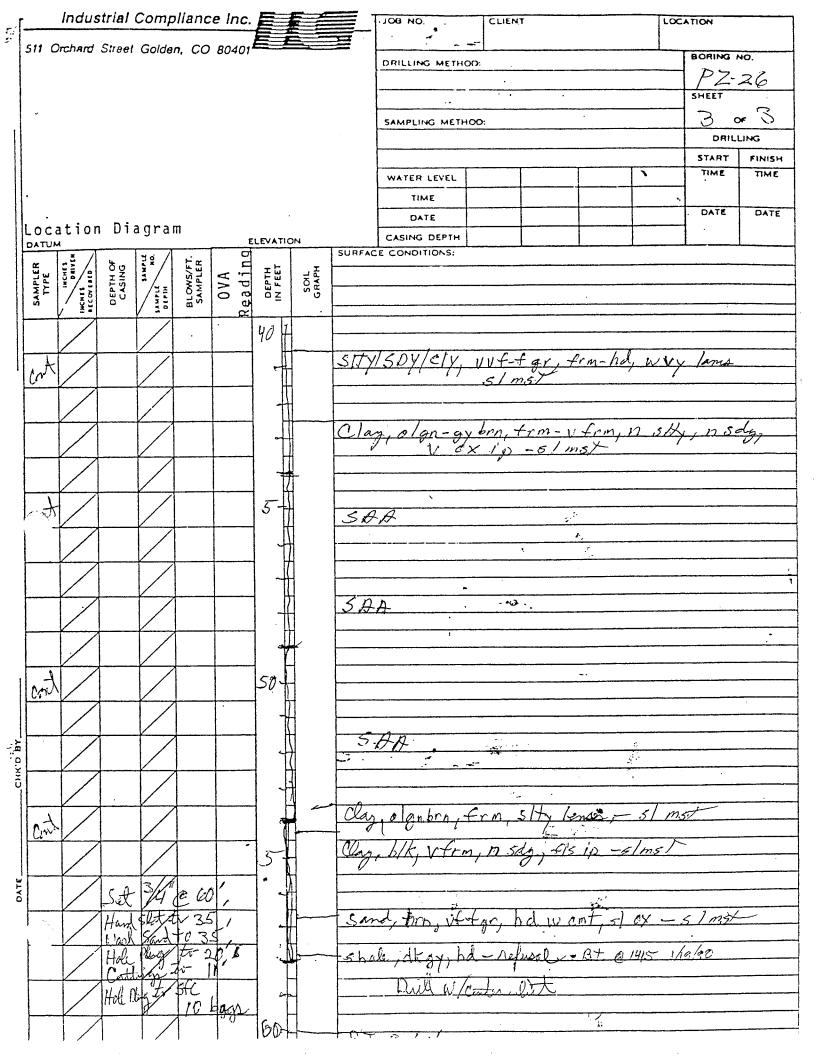


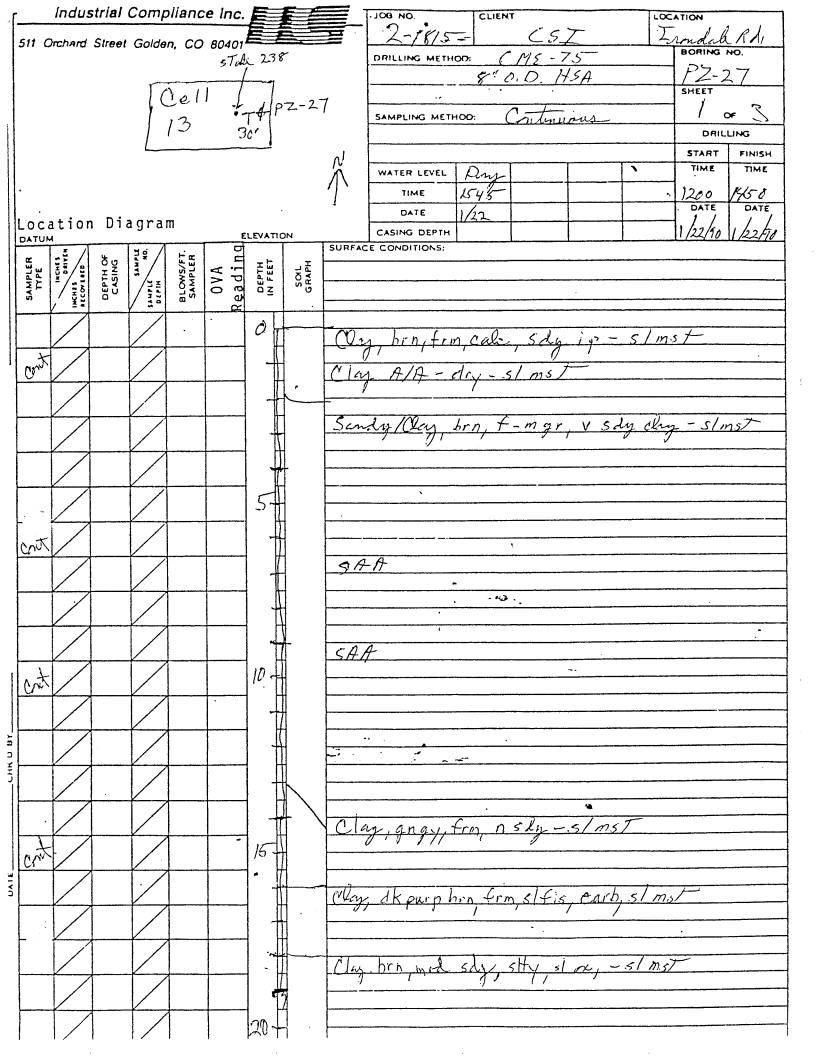






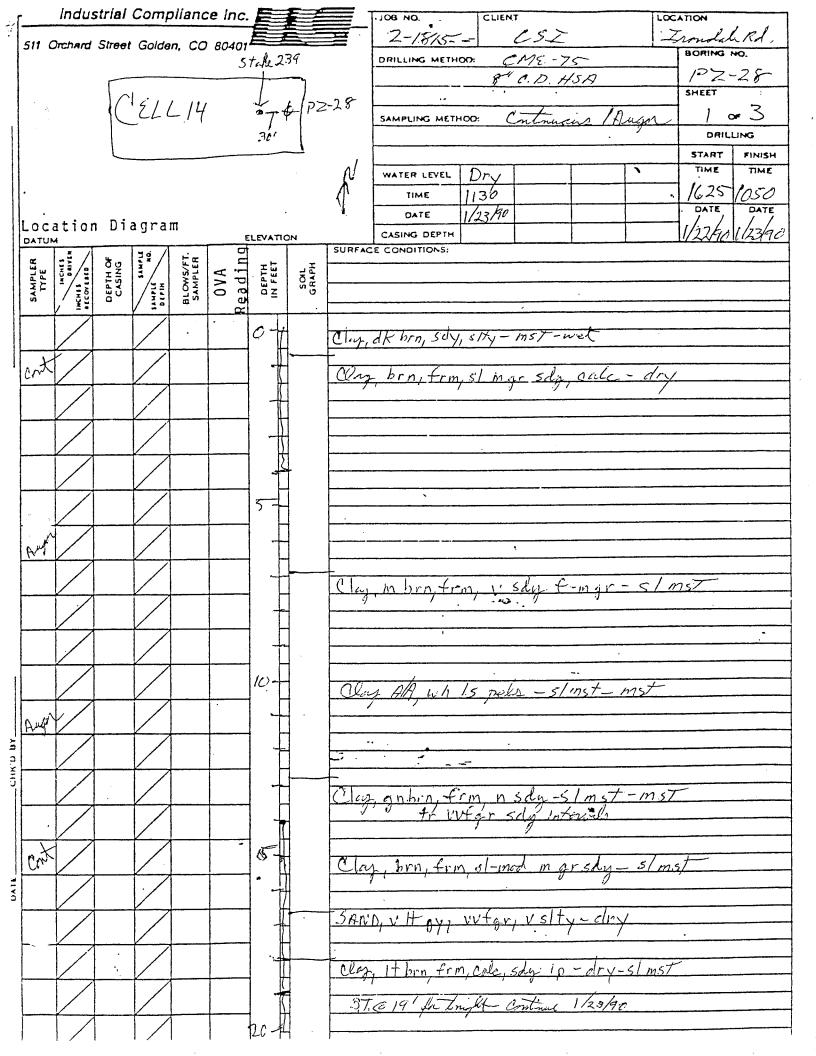


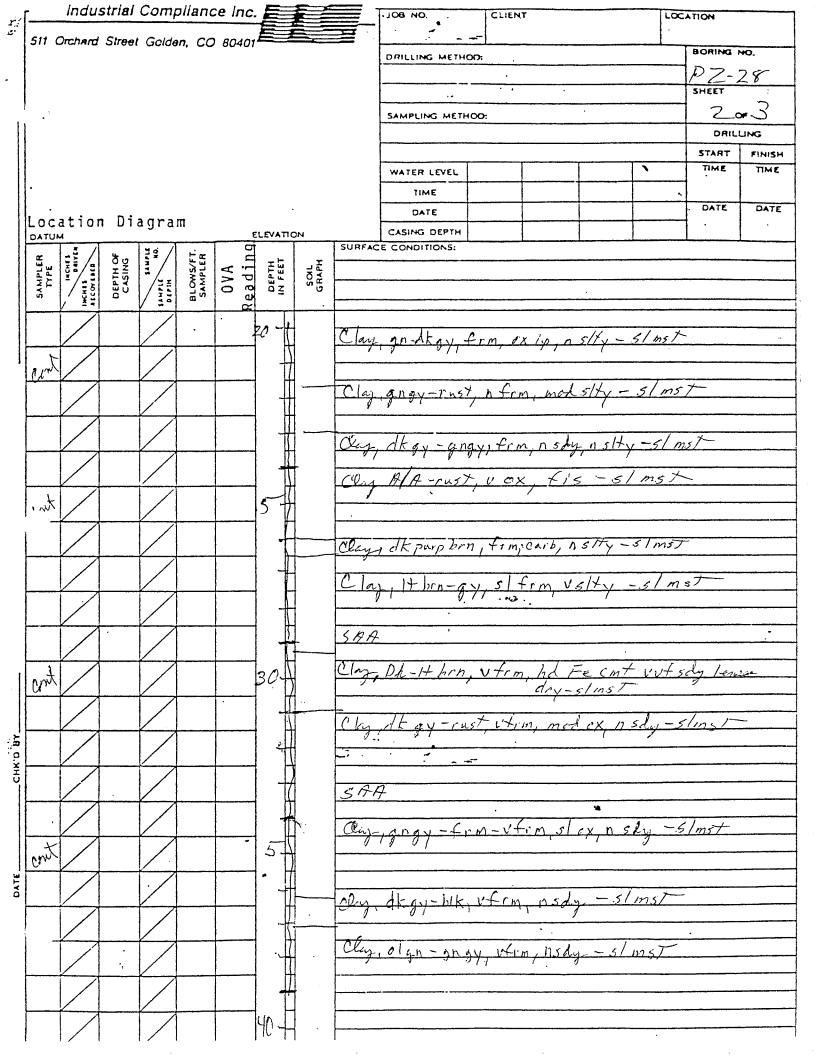


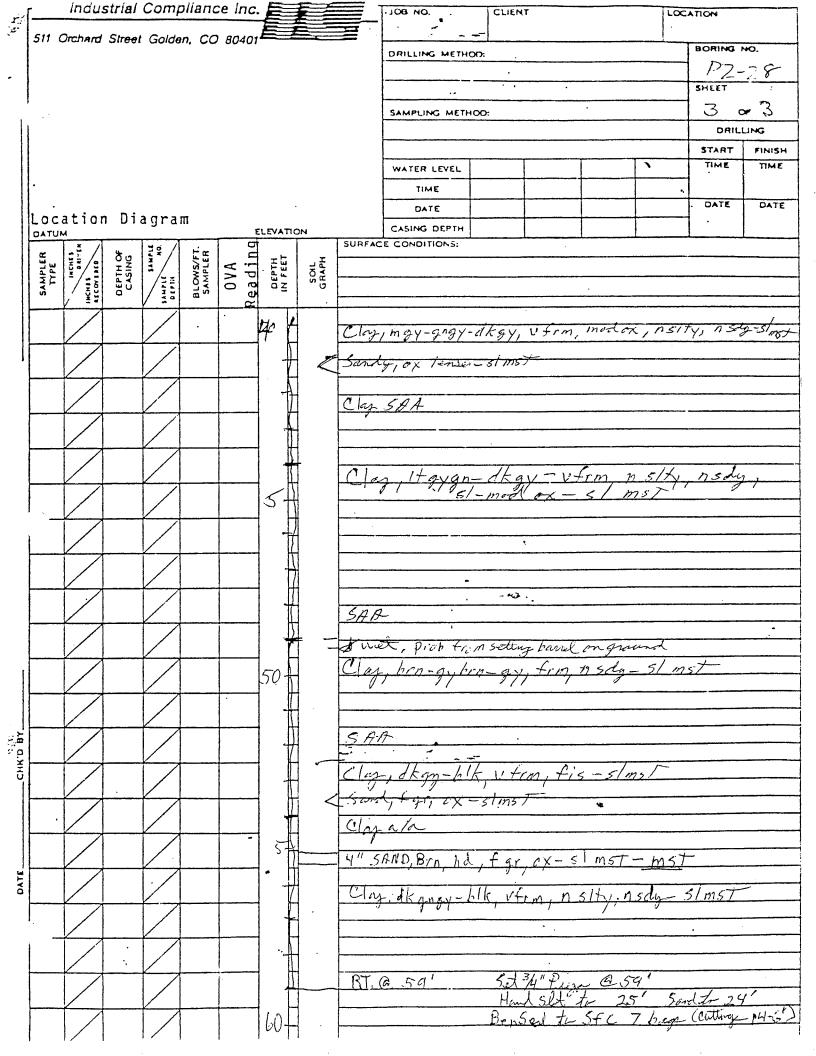


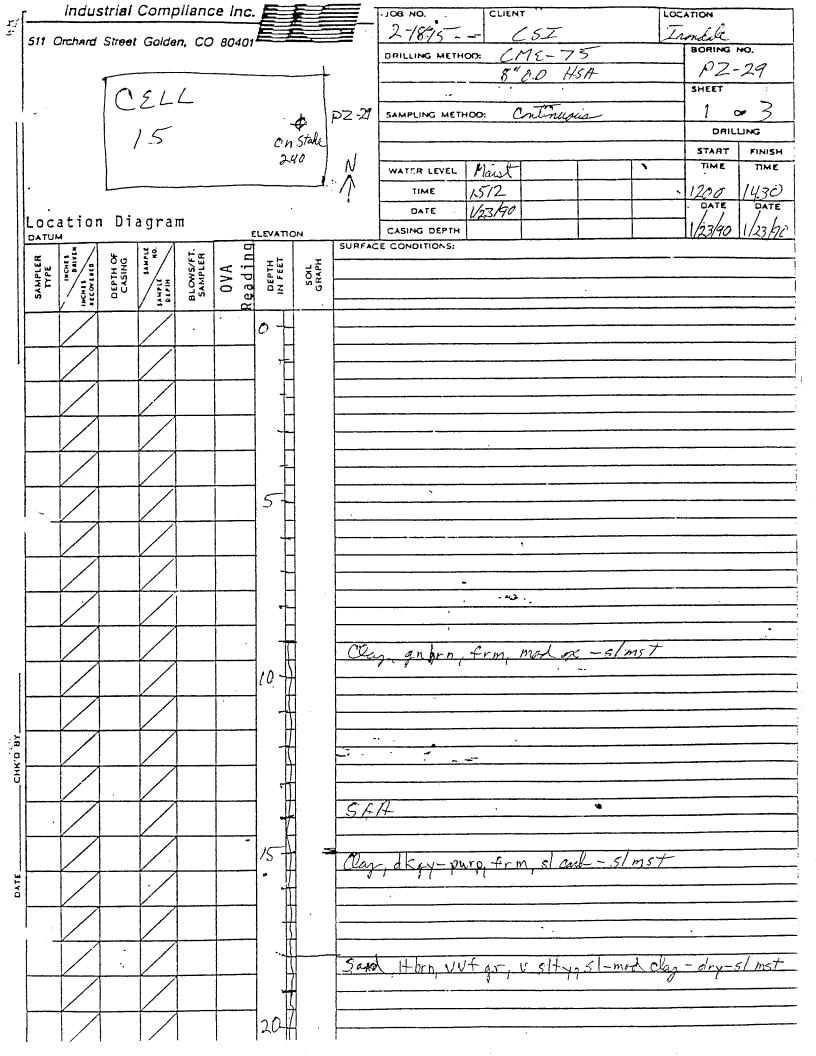
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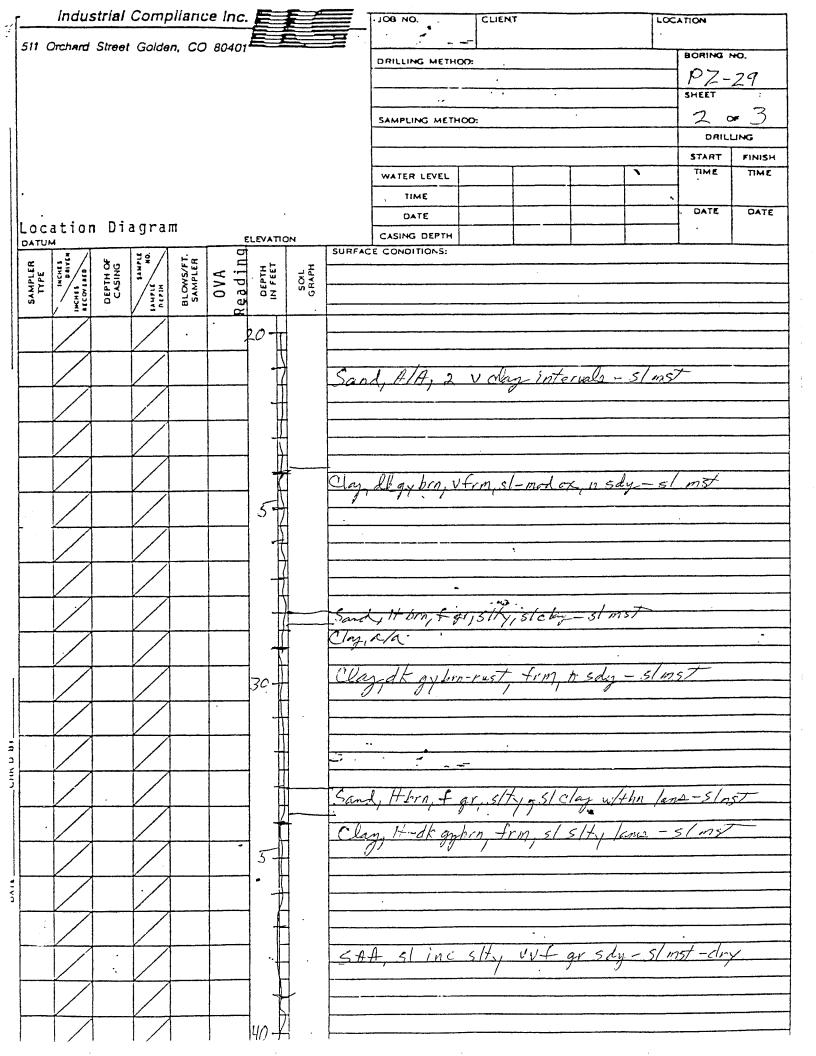
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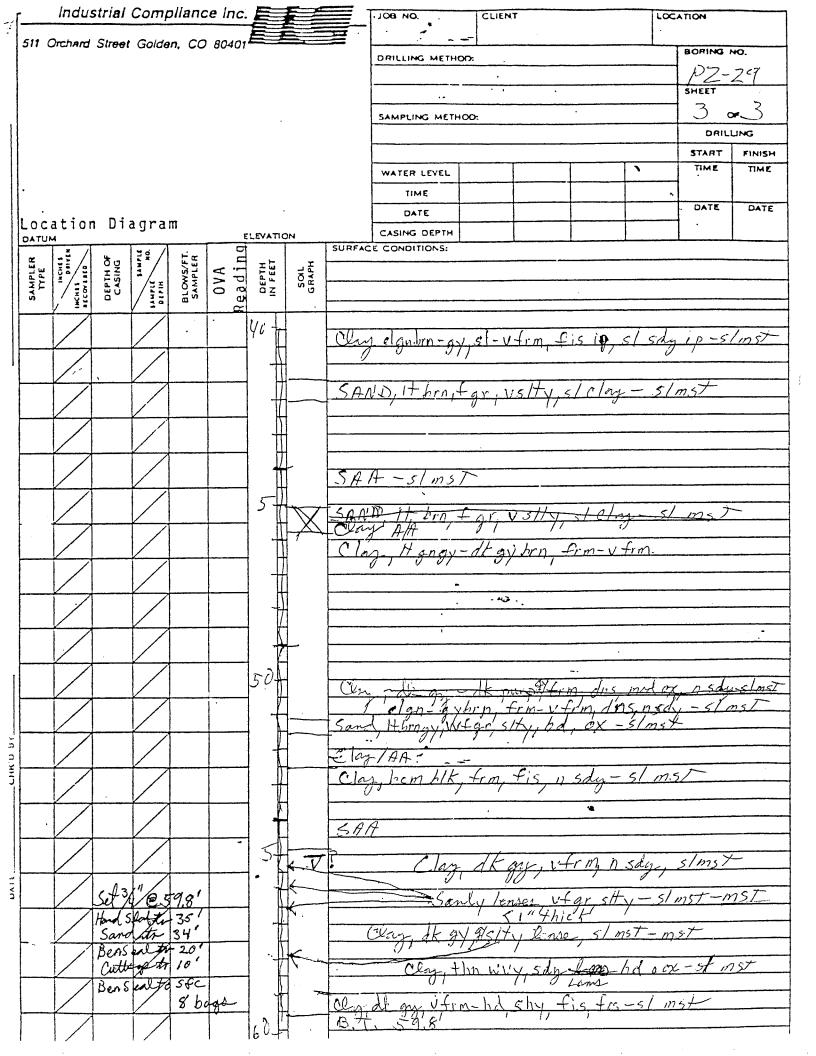


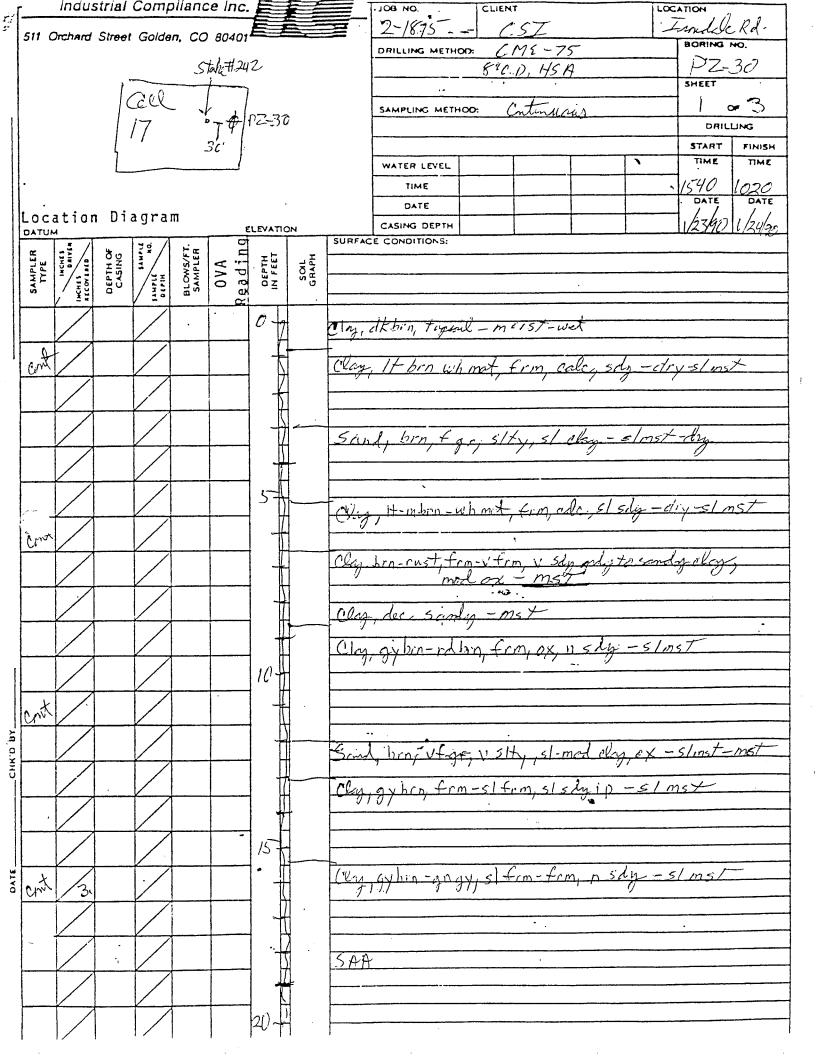


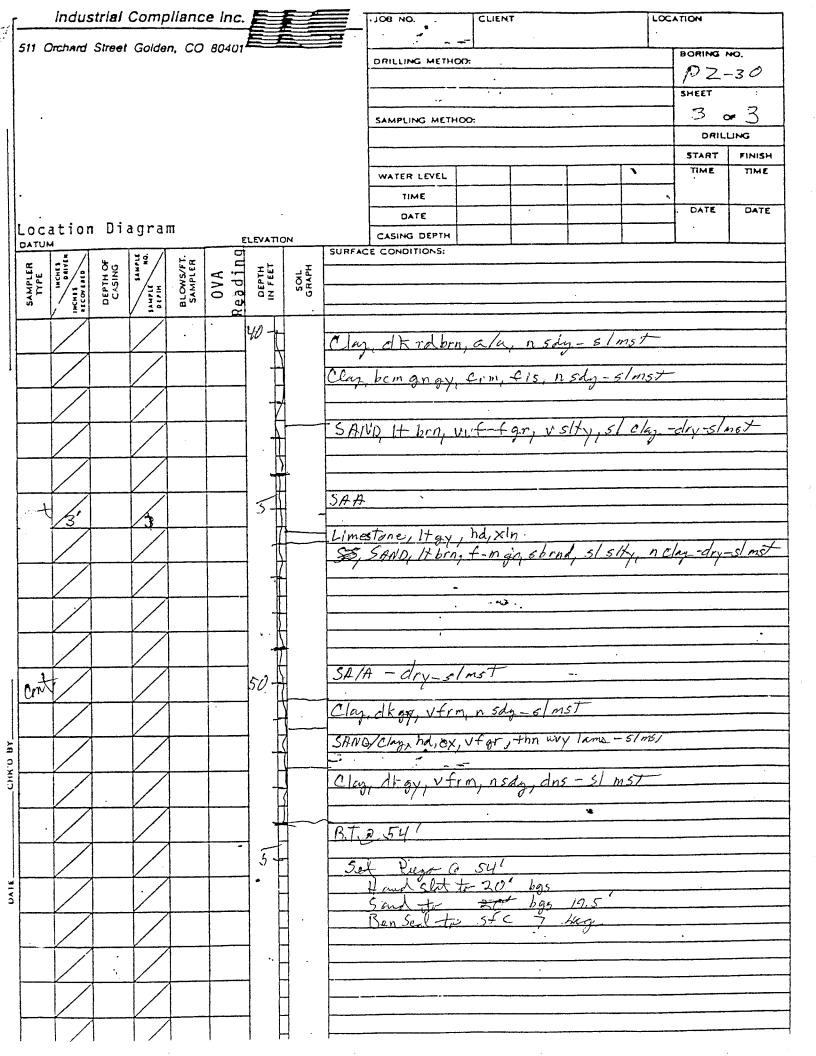


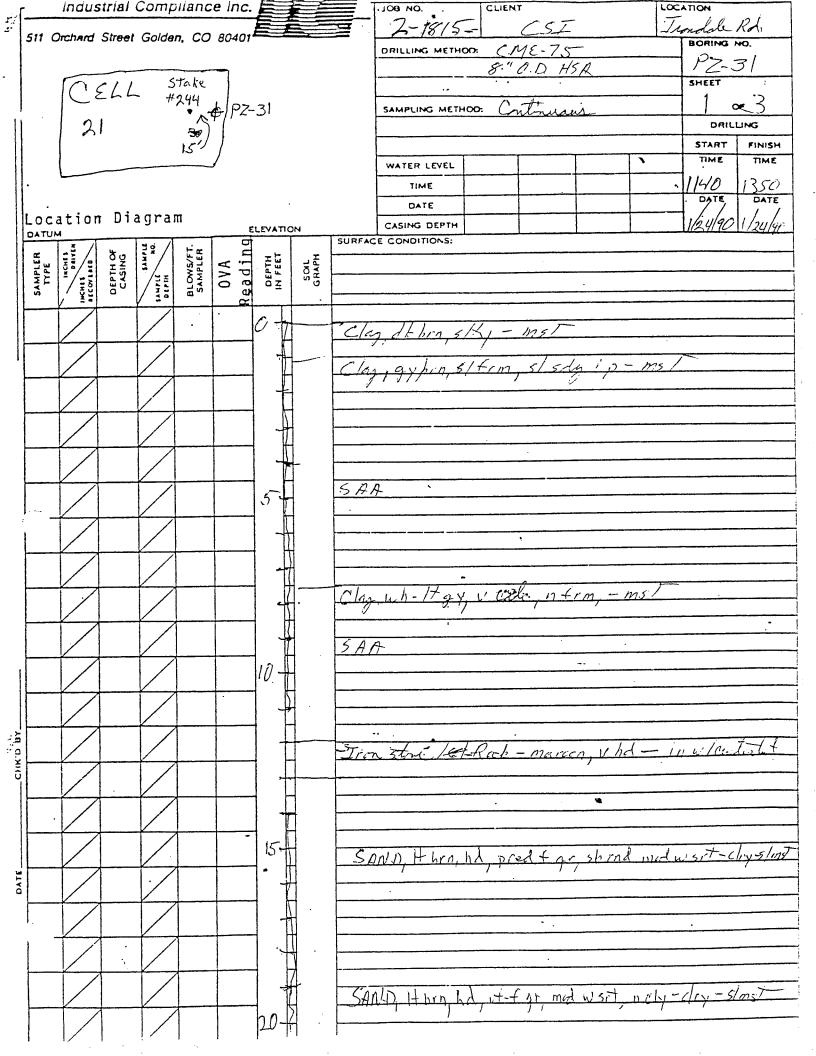


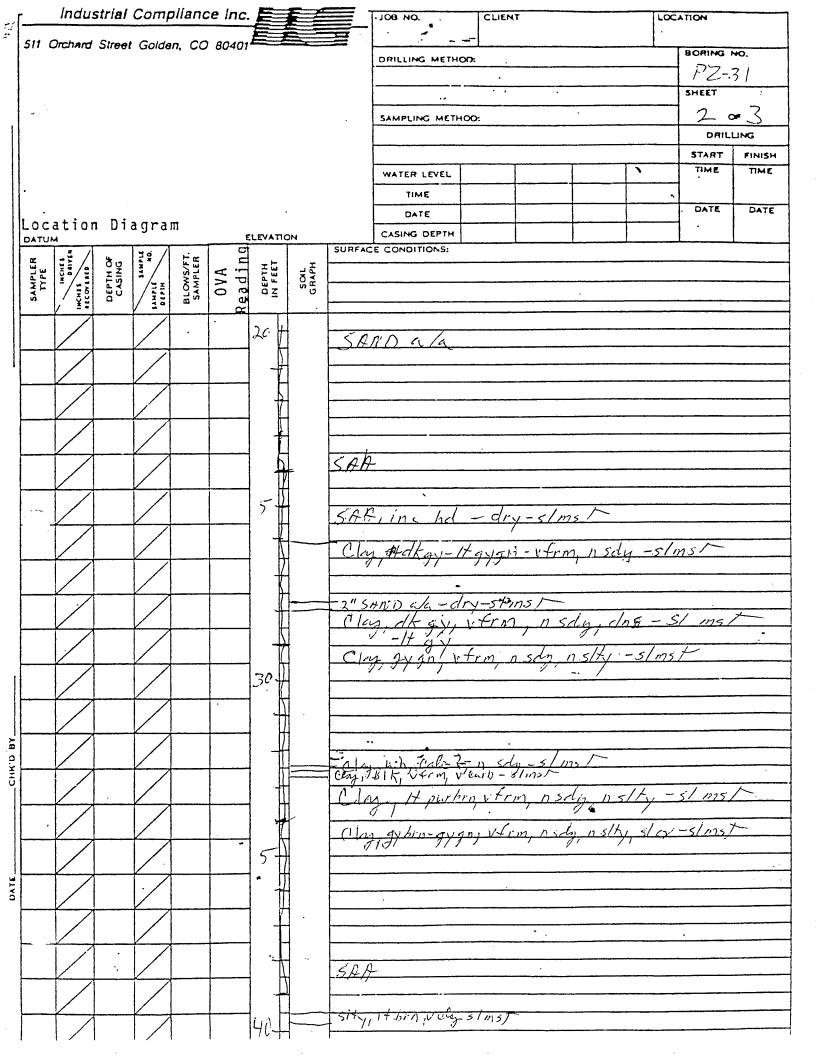


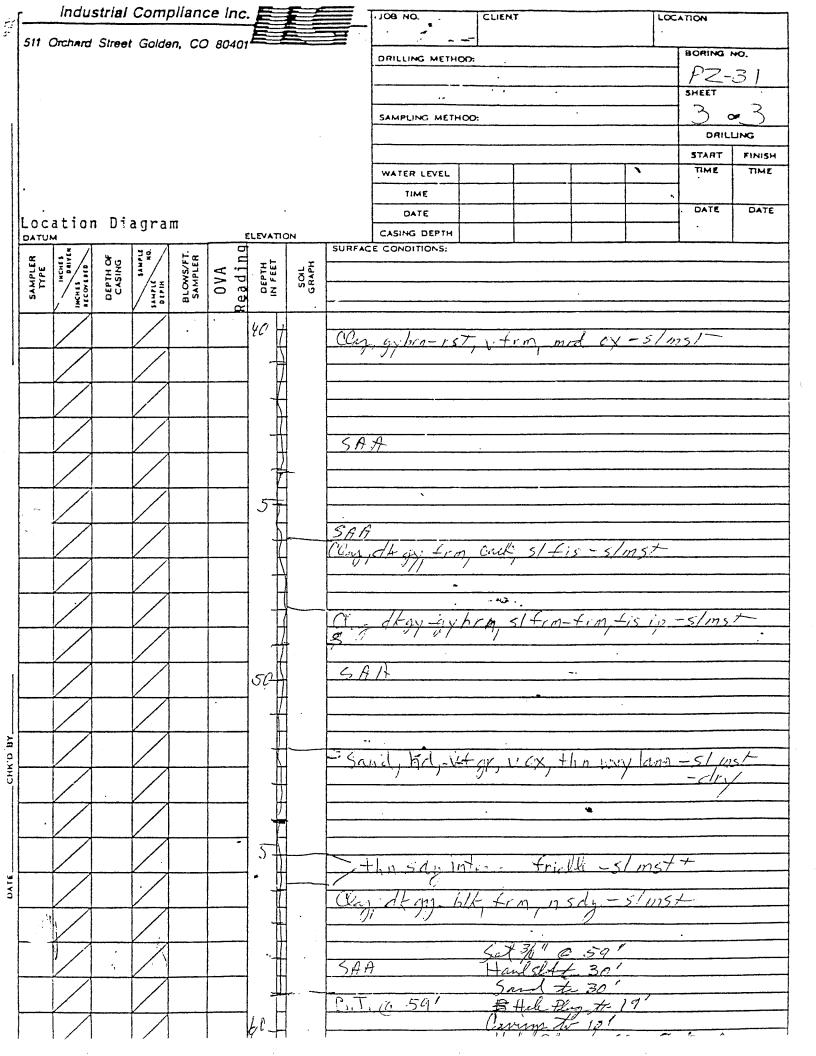


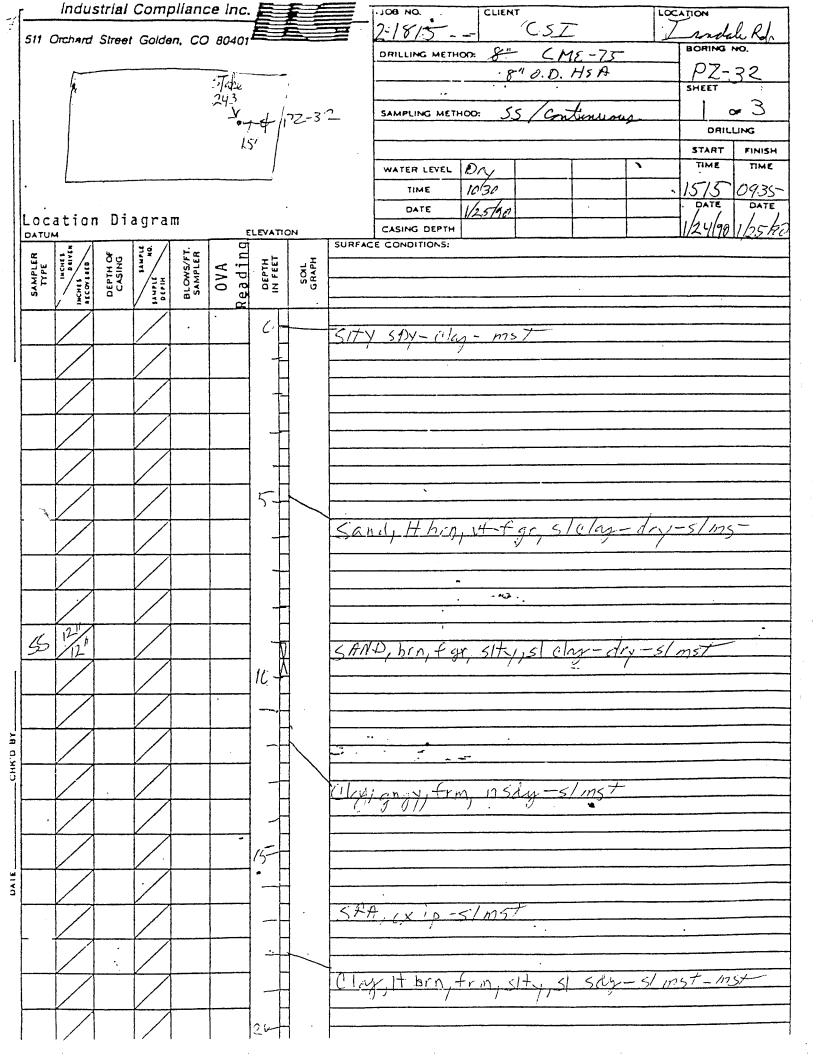




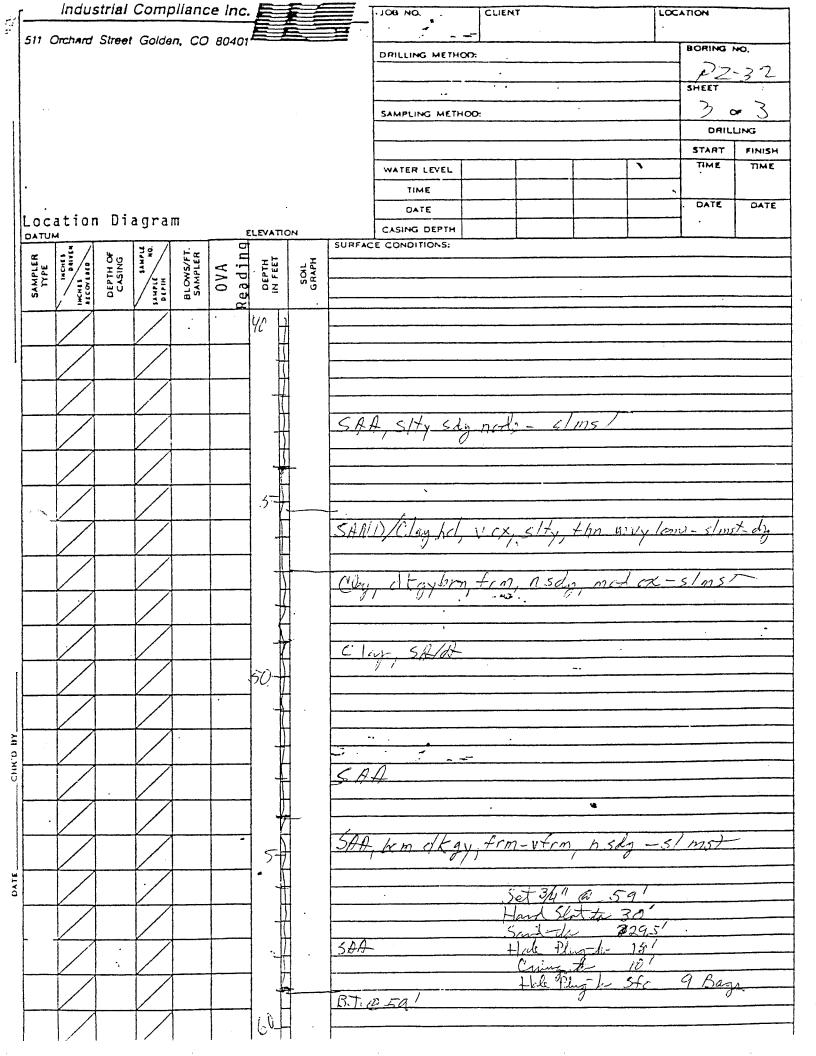


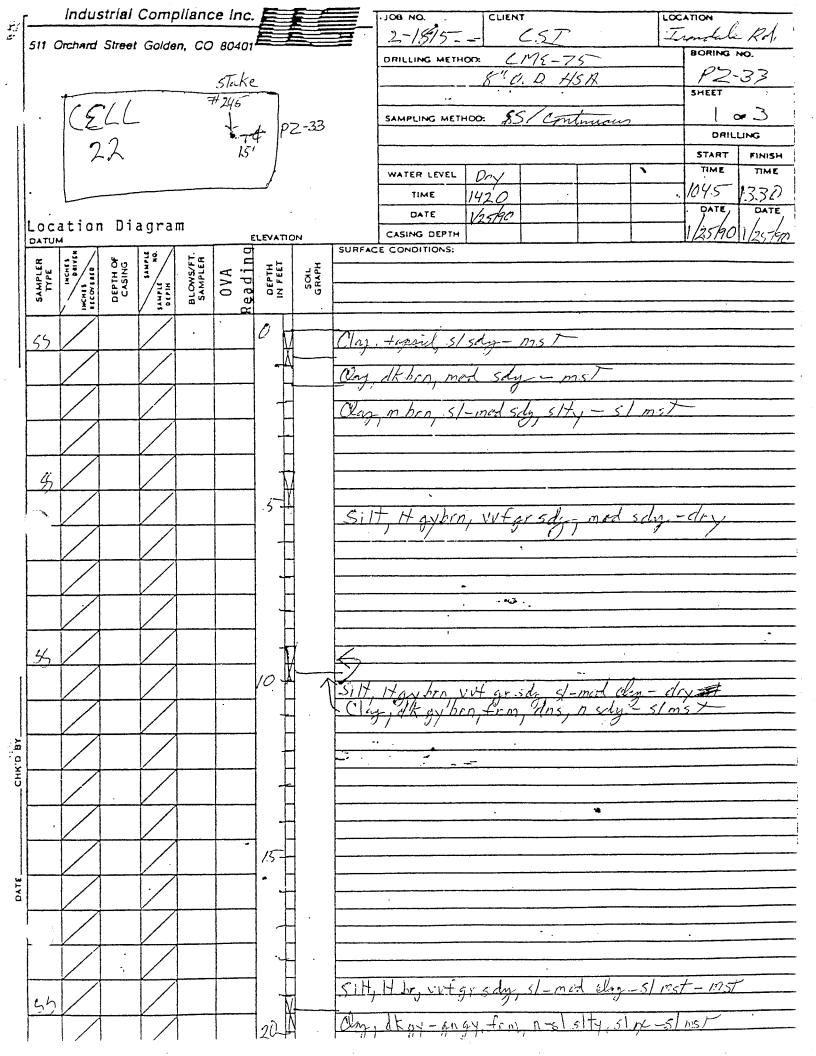


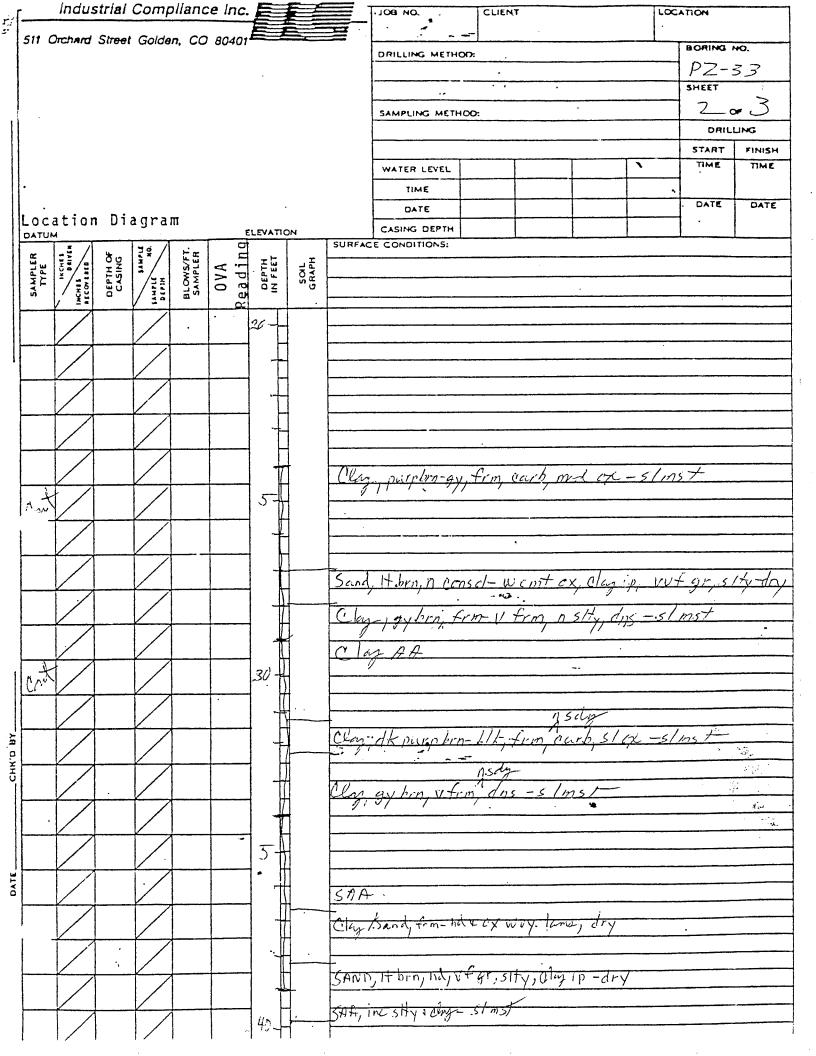


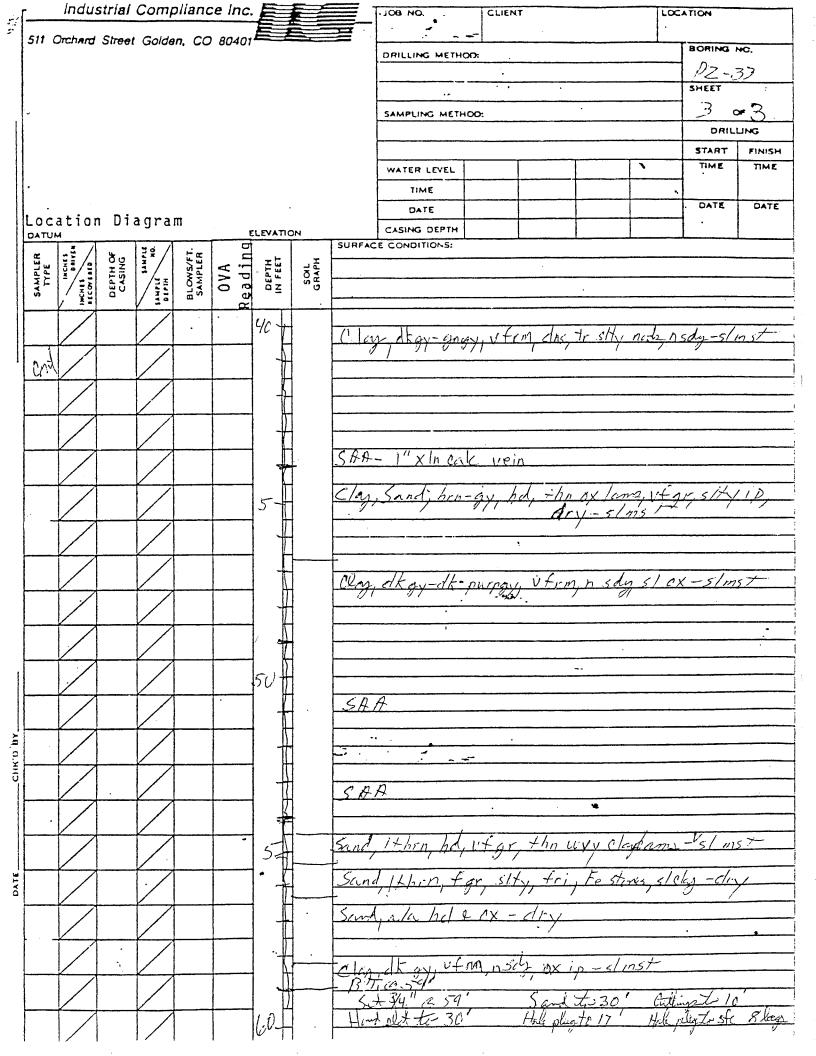


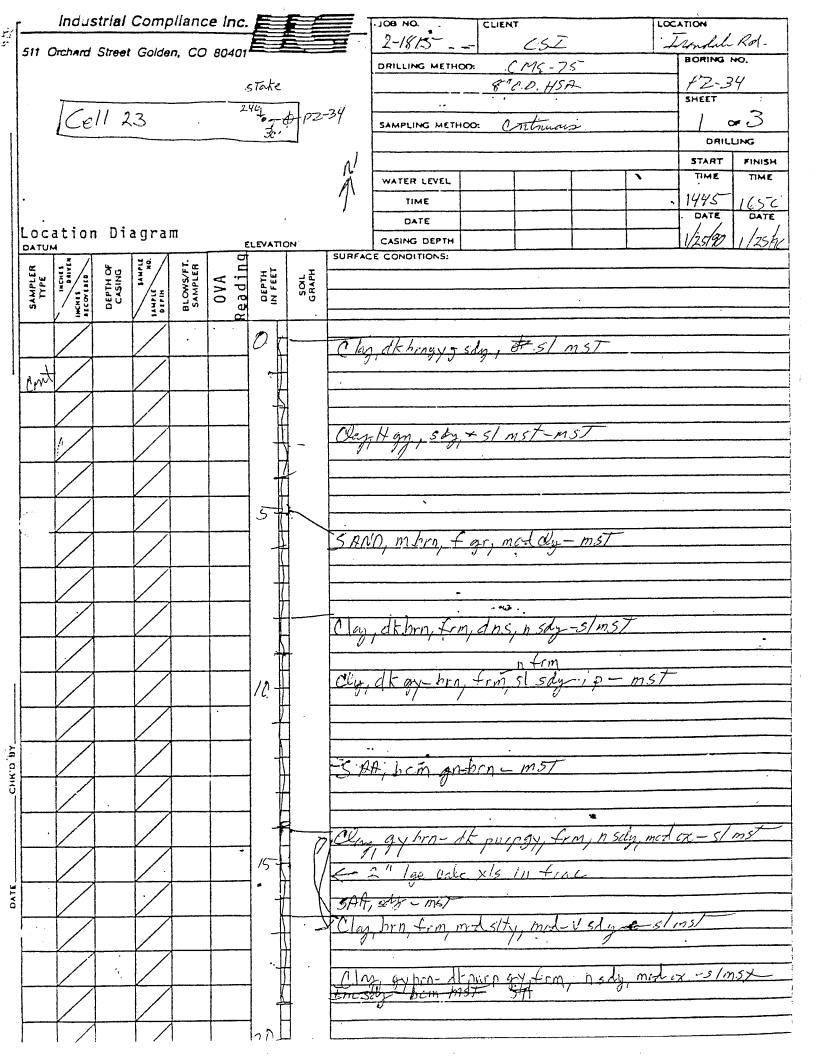
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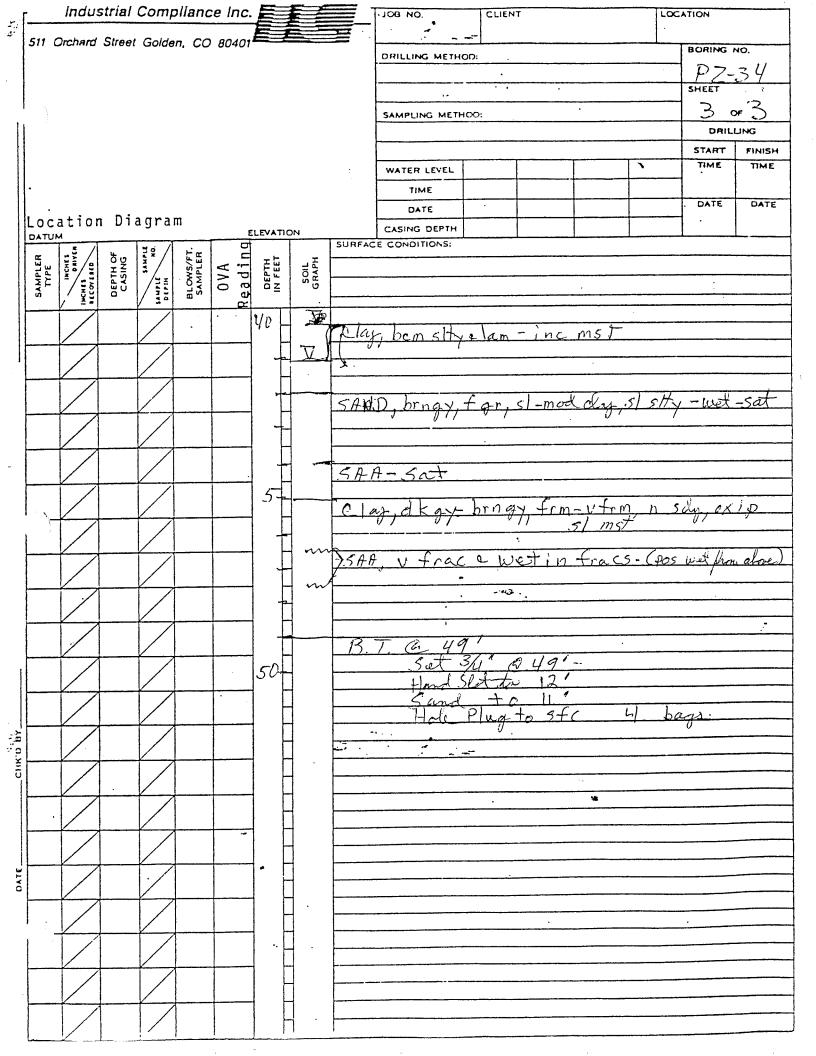


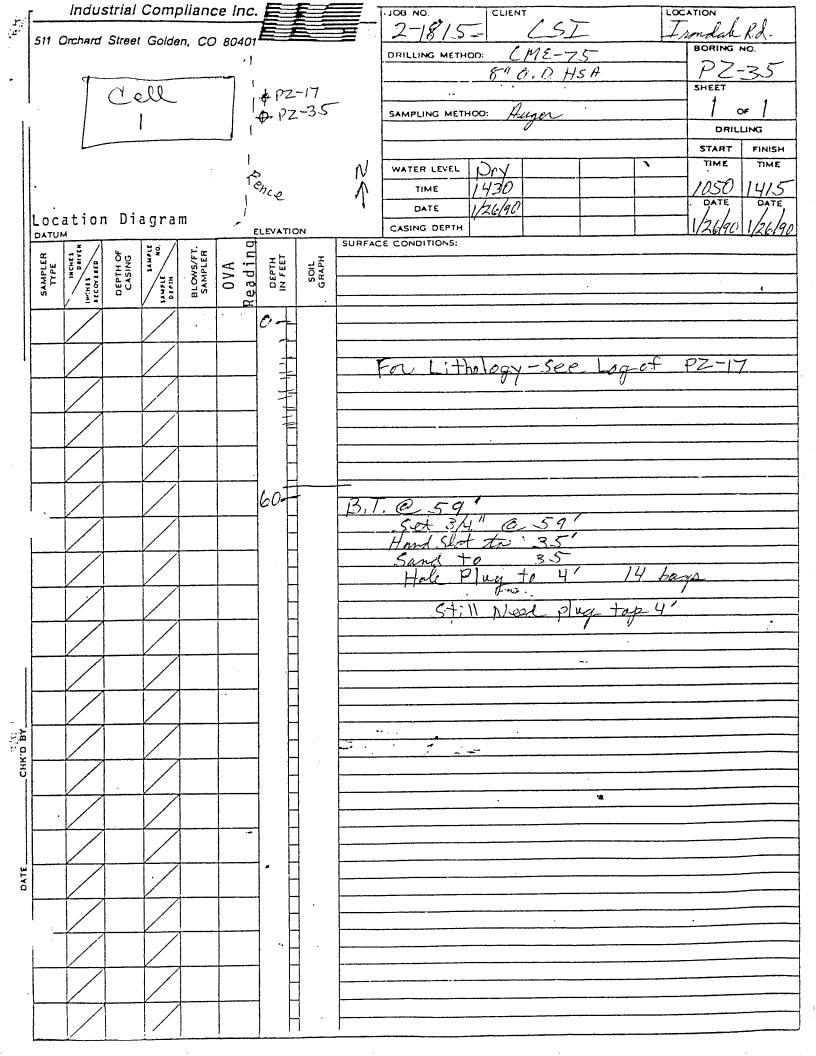


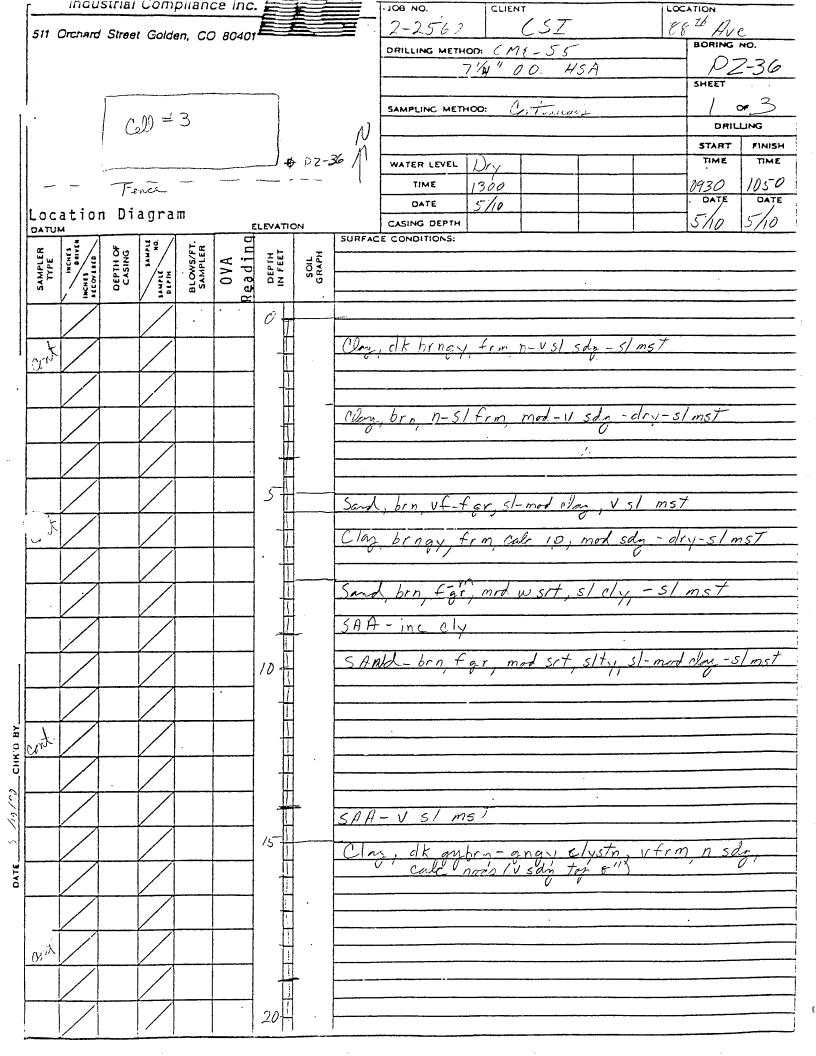












APPENDIX E - FIELD AND LABORATORY SOILS TEST RESULTS

Accept engineering, inc.

5624 Yukon Street Arvada, CO 80002 (303) 425-8700

RECEIVED SEP 2 3 1000

September 21, 1988

Industrial Compliance Incorporated 511 Orchard Street Golden, Colorado 80401

Job No. 88-00-42774-00

Attention: Nr. Curtis J. Ahrendsen

Subject: Results of Laboratory Testing

Dear Mr. Ahrendsen:

At your request, laboratory testing was performed on eight samples provided to our laboratory by your ofice. The laboratory testing consisted of three Standard Proctors (ASTN D-698, method A), three Atterberg Limits (Liqid Limits and Plastic Limits), three Mechanecal Analysis (Gradations), and eight Permeability tests. Three of the samples for the Permeability tests were remolded at 95% compaction of the Optimum Proctor values. The results of these tests are summerized in Table 1.

Please contact our office if you have any questions regarding this letter or when additional testing is required.

Respectfully submitted,

ACCENT ENGINEERING, INC.

H. Andrew Asgarian, P.E.

Project Engineer

HAA/pmk ici42774.haa

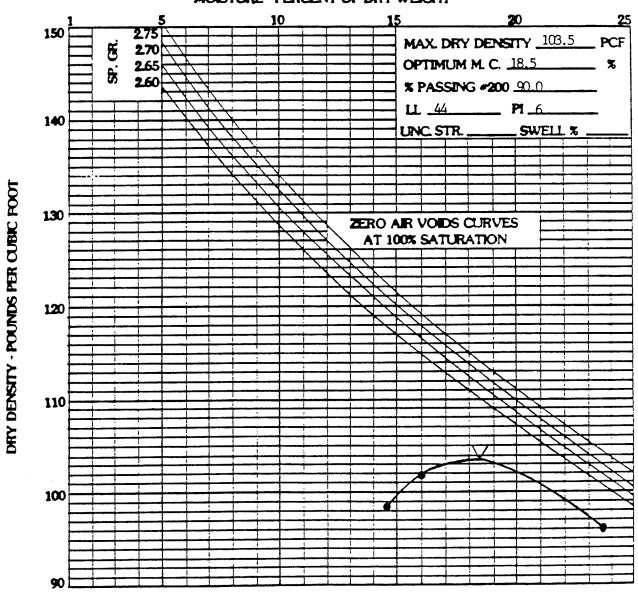
•						····								
		Remarks	Undisturbed Sample	Standard Proctor.Remolded Sample	Undisturbed Sample	Undisturbed Sample	Standard Proctor (ASIM-1698)	Remolded Sample	Standard Proctor.Remolded Sample	Remolded Sample		42774-00	20, 1988	
	sis	Silt & Clay (%)	ı	90.06	ı	1	95.4	1	97.4	I		NO.: 88-00-42774-00	September 20, 1988	1
-	Sieve Analysis	Sand (%)	1	10.0	1	1	4.6	ſ	2.6	ı		JOB NC	DATE:	TABLE
	Siew	Gravel (%)	ſ	0.0	ı	I	0.0	ı	0.0	J		JC	/Q	F
je 1940	Perme- ability	10 -8 ст/зес	5.05	2.71	4.04	3.11 x 10 ⁻⁷	5.12	2.99	3.47	3.26				
	Limits	PI	1	9	I	.1	19	ı	25	ı				
	Atterberg Limits	II.	1	. 4	ı	1	97	ı	45	1				0 2 -
	Optimum Moisture Content	E.	1	18.5	ı	1	18.5	1	19.0	ı				ENGINEERING
	Maximum Dry Density		ı	103.5	ı	ı	102.5	ı	103.0	ı		RESULTS		ENG.
	Soil Description		Claystone Bedrock	Claystone Bedrock	Claystone Bedrock	Claystone Bedrock	Claystone Bedrock	Claystone Bedrock	Claystone Bedrock	Claystone Bedrock		LABORATORY TEST RESULTS		ACCENT
	Sample	· •	SB-4	SB-5	SB-7	888	SB-10	SB-10	SB-14	SB-14	·			

- ...

SAMPLE OF Claystone Bedrock

SAMPLE NO. SB-5

MOISTURE - PERCENT OF DRY WEIGHT

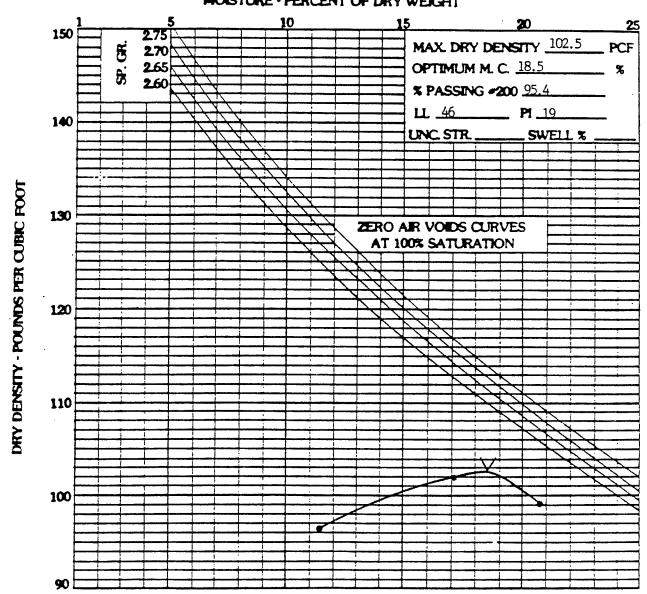


PROCTOR CURVE	JOB NO.: 88-00-42774-00
	DATE: September 21, 1988
ACCENT ENGINEERING, INC.	FIGURE 1

SAMPLE OF Claystone Bedrock

SAMPLE NO. SB-10

MOISTURE - PERCENT OF DRY WEIGHT



PROCTOR CURVE

ACCENT ENGINEERING, INC.

JOB NO.: 88-00-42774-00

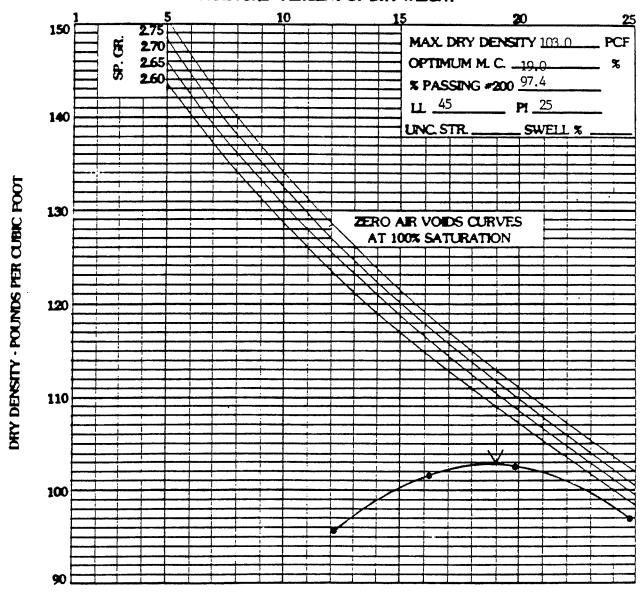
DATE: September 21, 1988

FIGURE 2

SAMPLE OF Claystone Bedrock

SAMPLE NO. SB-14

MOISTURE - PERCENT OF DRY WEIGHT



PROCTOR CURVE	JOB NO.: 88-00-42774-00
ACCEVE EXCLUSION TO	DATE: September 21, 1988
ACCENT ENGINEERING, INC.	FIGURE 3

		SIEVE ANALYSIS										
	Cobbles Gravel Sand											
[]	i	Clear	Square	Openi		COATE		Standard S			fine	
			3-1 1/2-								-100	•200
	90									7	T	Sample of Claystone Bedrock
	80								\dashv	+		from test hole SB-5
	70						\neg				\vdash	at depth feet.
SINC	60								7	1	_	Atterberg limits:
PAS	50								7	1		Liquid Limits 44
T.N.	40						\neg		7	+		Plasticity Index 6
PERCENT PASSING	30								寸	1		Classification: Unified A.5
PE	20								7	\top		AASHTO
	10						\neg		十	1		Group Index
							\exists		7	1		
	100											
	90						\perp					Sample of Claystone Bedrock
	80				_		\bot		\perp			from test hole SB-10
	70				_ _							at depthfeet.
N. 1	60	-				_						Atterberg Limits:
ASS	50				_				\perp	$oldsymbol{ol}}}}}}}}}}}}}}}}}$		Liquid Limits 46
T P.	40				_							Plasticity Index 19
PERCENT PASSING	30											Classification: Unified A 7-6
PER	20											AASHTO
	10											Group Index
	اه											•
1	100					TT			Ŧ	1		Sample of Claystone Bedrock
•	90				\dashv		十		\dagger	1		from test hole SB-14
	80			-	-		\dashv		╁	-	-	at depthfeet.
DNI:	70				\dashv	$\dashv \vdash$	╌	_	+	╁—		
ASS	60	1-1			\dashv		+	_	╁			Atterberg Limits:
7 T	50	++			- -			-	+	-		Liquid Limits 45
CEN	40								_	<u> </u>		Plasticity Index 25
PERCENT PASSING	30	+			_		4	_ _	_			Classification: Unified A 7-6
	20	- -			_ _	$\perp \!\!\! \perp$			1			AASHTO
;	10	44										Group Index
	اه				1	Ш			_			
	200	127 76.	2 38.1 19.								49 .0	74
				rain S	JIZ C	in M	11111	mete	278			
ŀ	MECHANICAL ANALYSIS TEST RESULTS										JOB NO.: 88-00-42774-00	
	ACCENT ENGINEERING, INC.								DATE: September 21, 1988			
									FIGURE 4			



ENGINEERING, INC.

5624 YUKON STREET ARVADA, CO 80002 (303) 425-8700

> 4 1988 RECEIVED OCT

September 28, 1988

Industrial Compliance Incorporated 511 Orchard Street Golden, Colorado 80401

Attention: Mr. Curtis J. Ahrendsen

Job No. 88-00-42785-00

Subject: Results of Laboratory Testing

Dear Mr. Ahrendsen:

At your request, swell/consolidation tests were performed on two samples provided to our laboratory by your office. The results of these tests are shown in Figure 1.

Please contact our office if you have any questions regarding this letter or when additional testing is required.

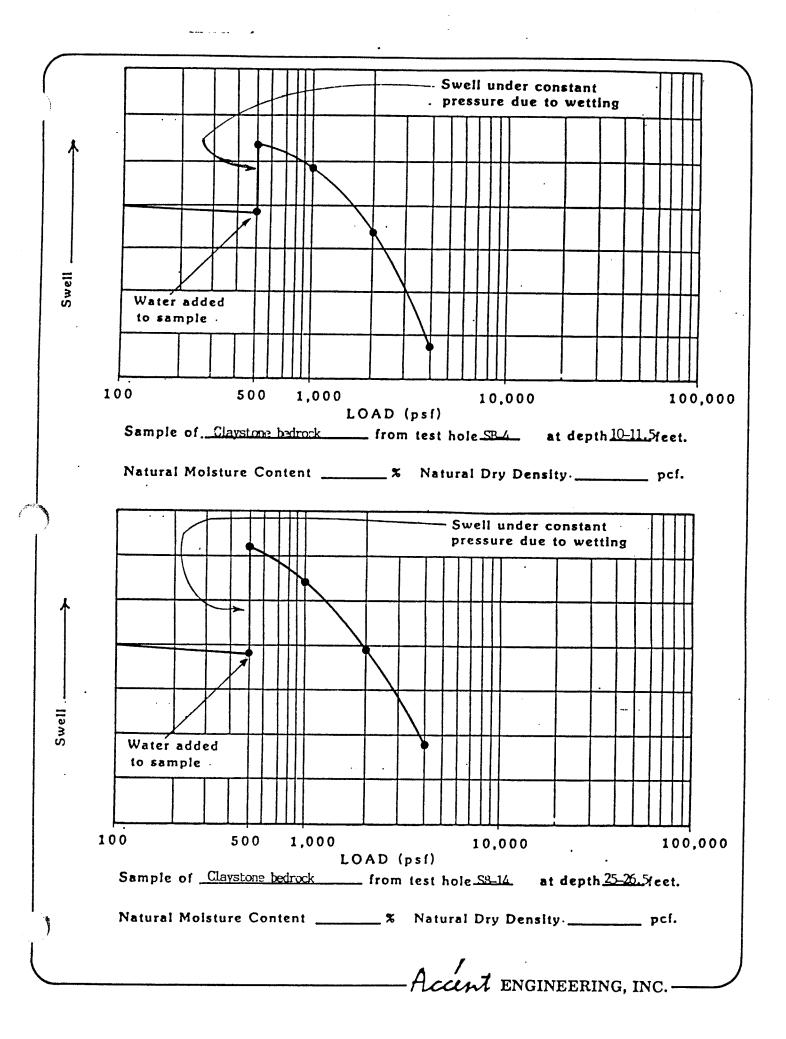
Respectfully submitted,

ACCENT ENGINEERING, INC.

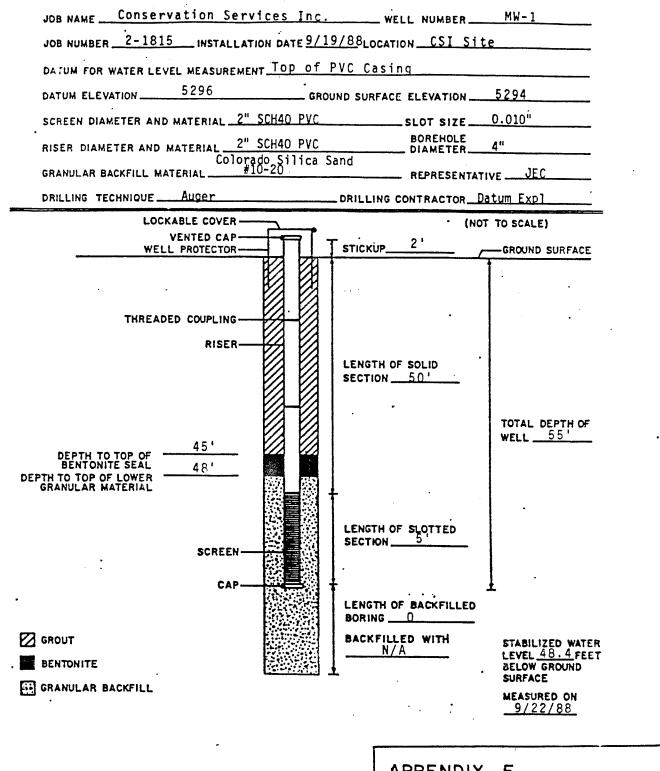
H. Andrew Asgarian, P.E.

Project Engineer

HAA/pmk ici42785.haa



APPENDIX F - MONITOR WELL AND PIEZOMETER COMPLETION FORMS

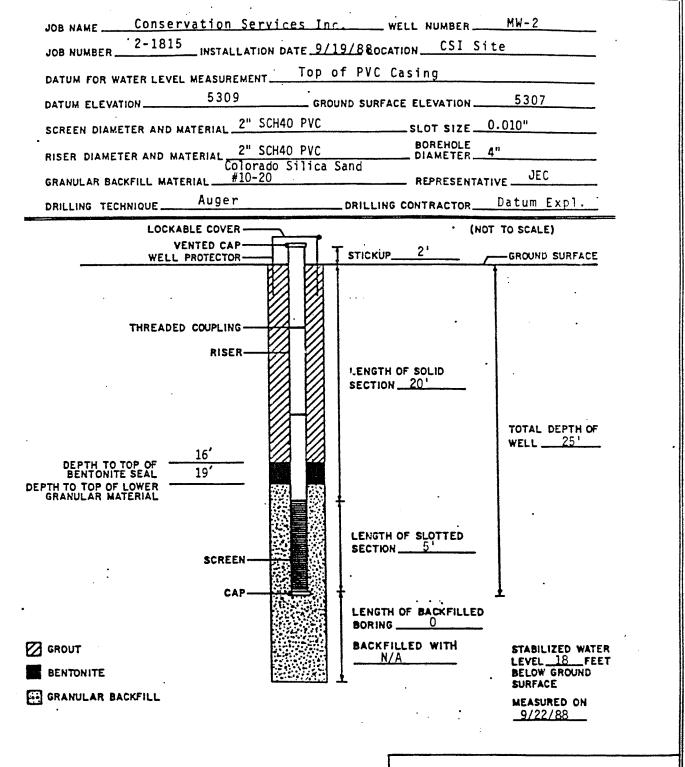


APPENDIX F

MONITORING WELL (MW-I)
COMPLETION DIAGRAM

Industrial Compliance Inc.
511 Orchard Street
Golden, Colorado 80401



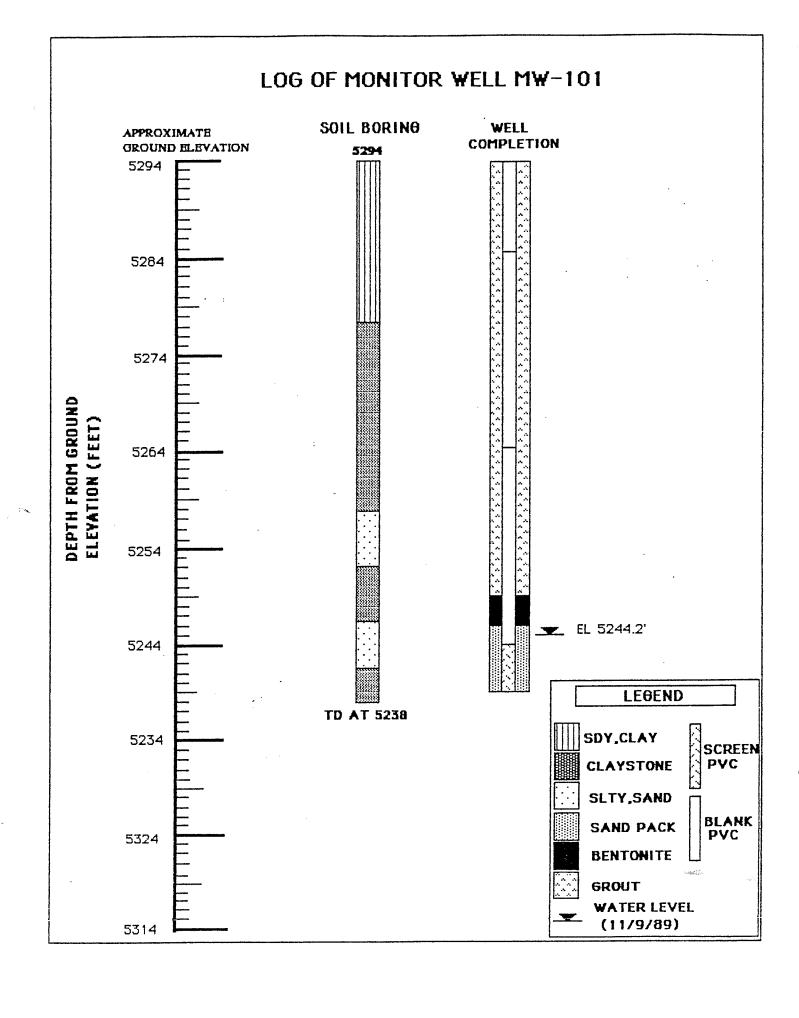


APPENDIX F

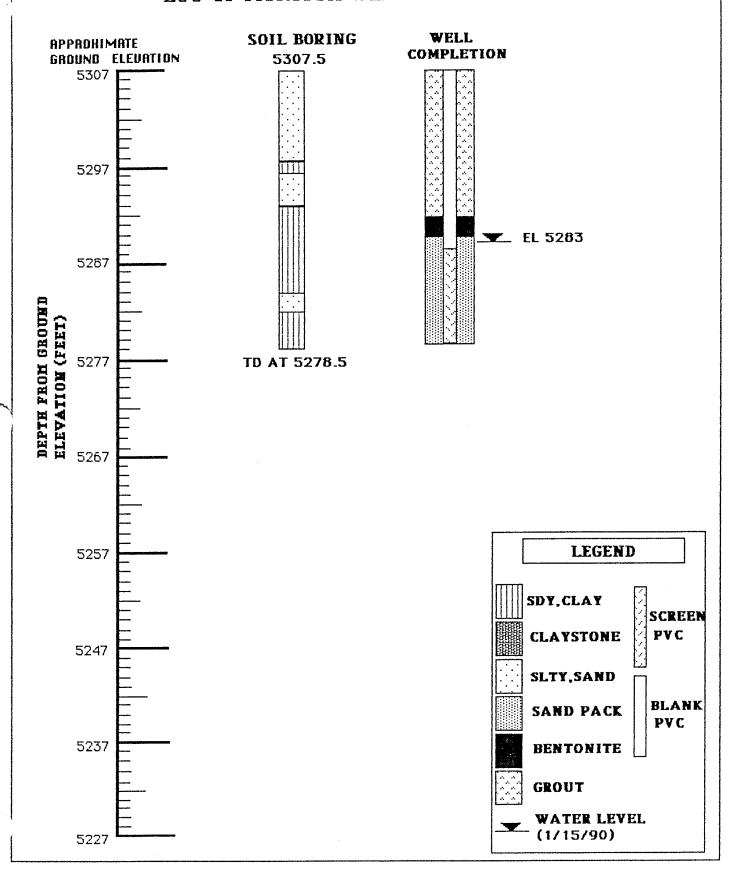
MONITORING WELL (MW-2) COMPLETION DIAGRAM

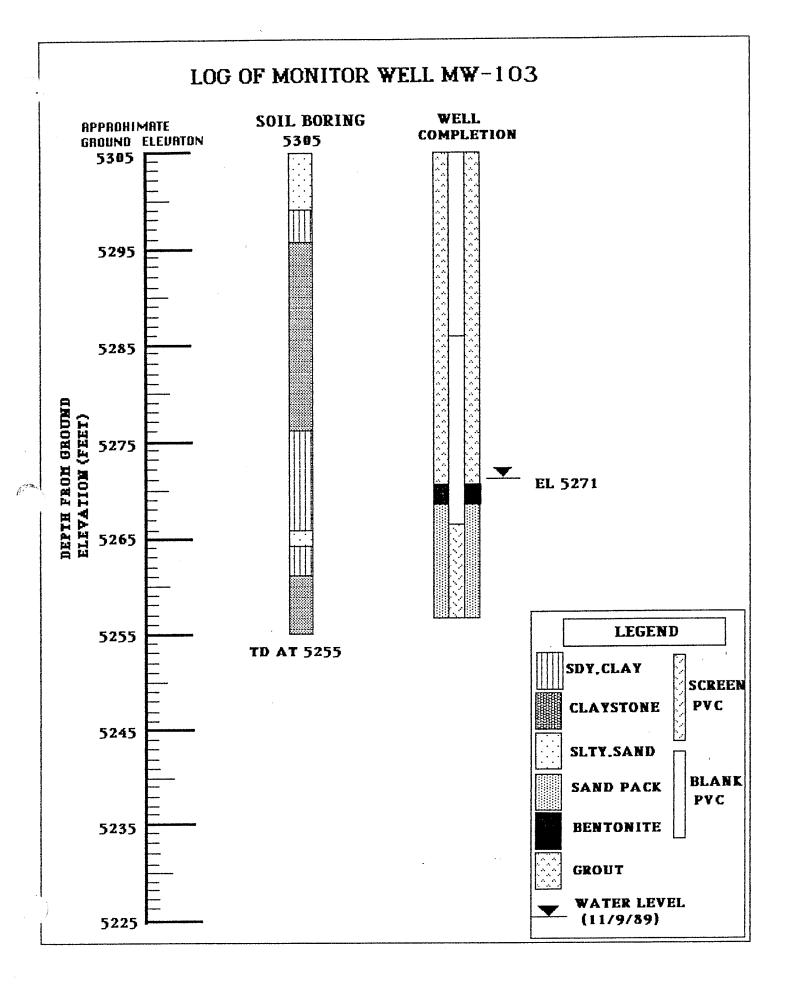
Industrial Compliance Inc. 511 Orchard Street
Golden, Colorado \ 80401

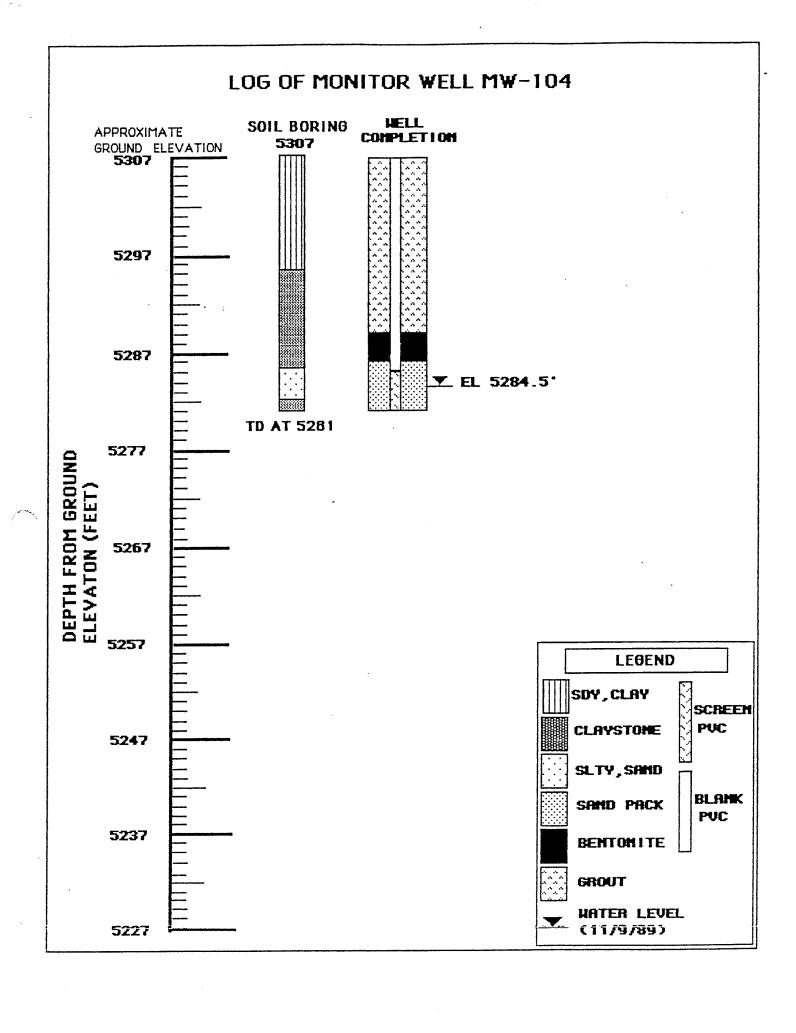


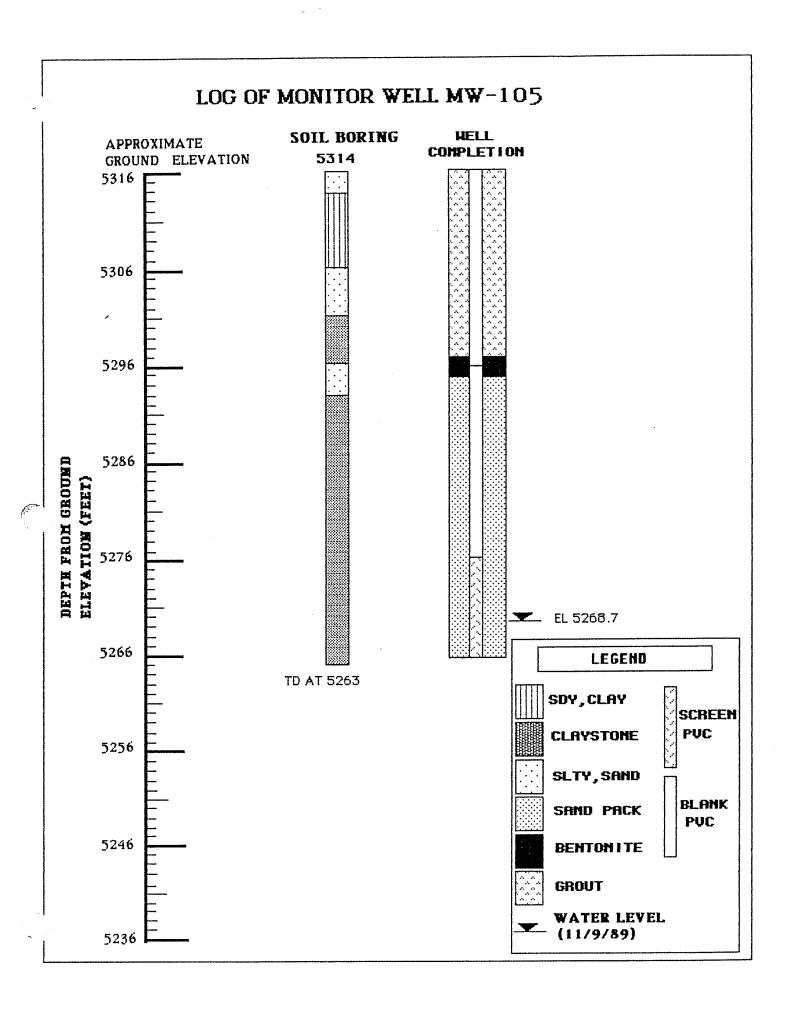


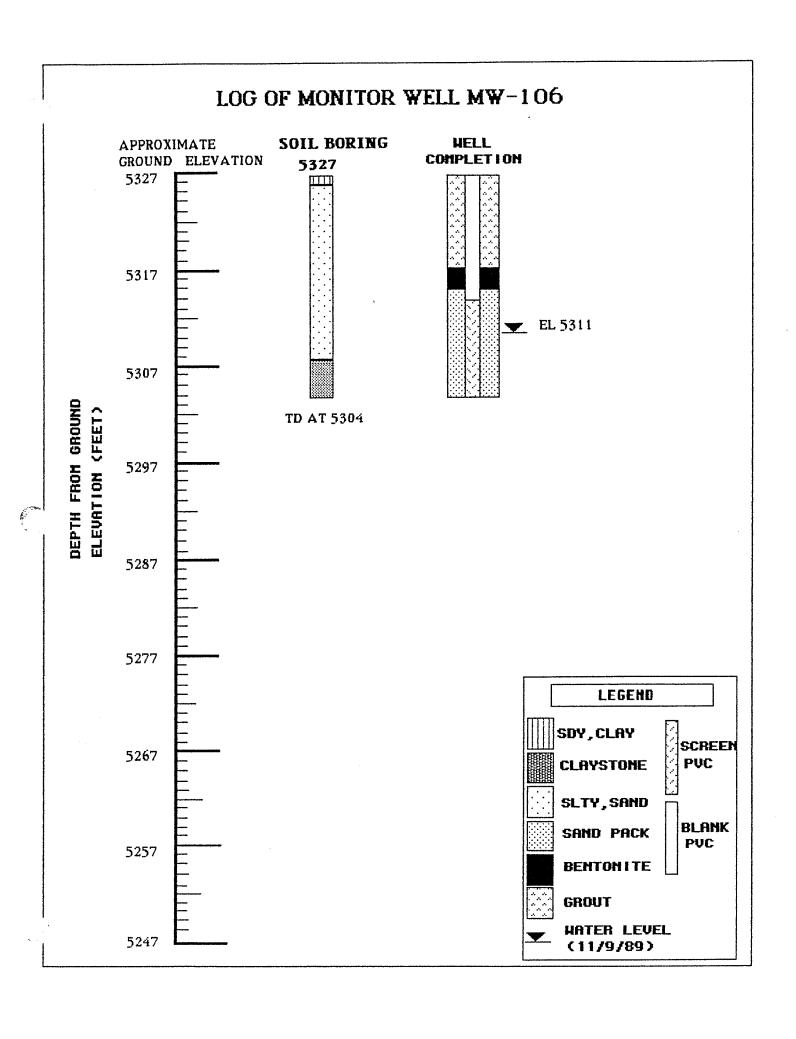
LOG OF MONITOR WELL MW-102

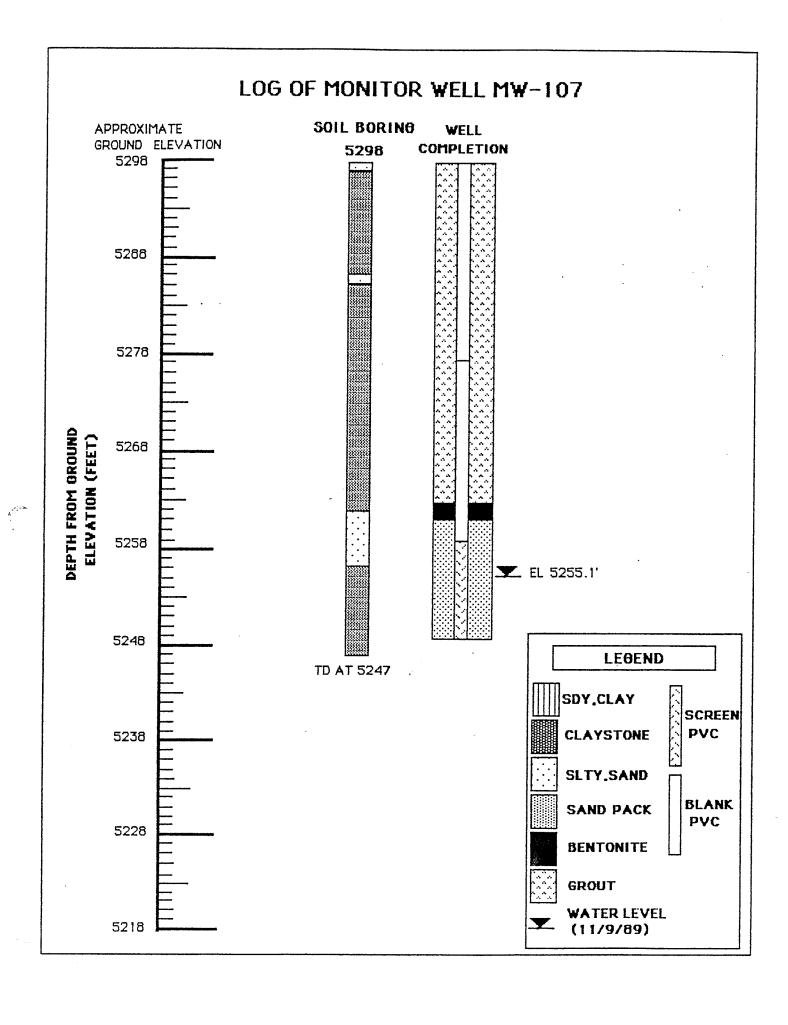


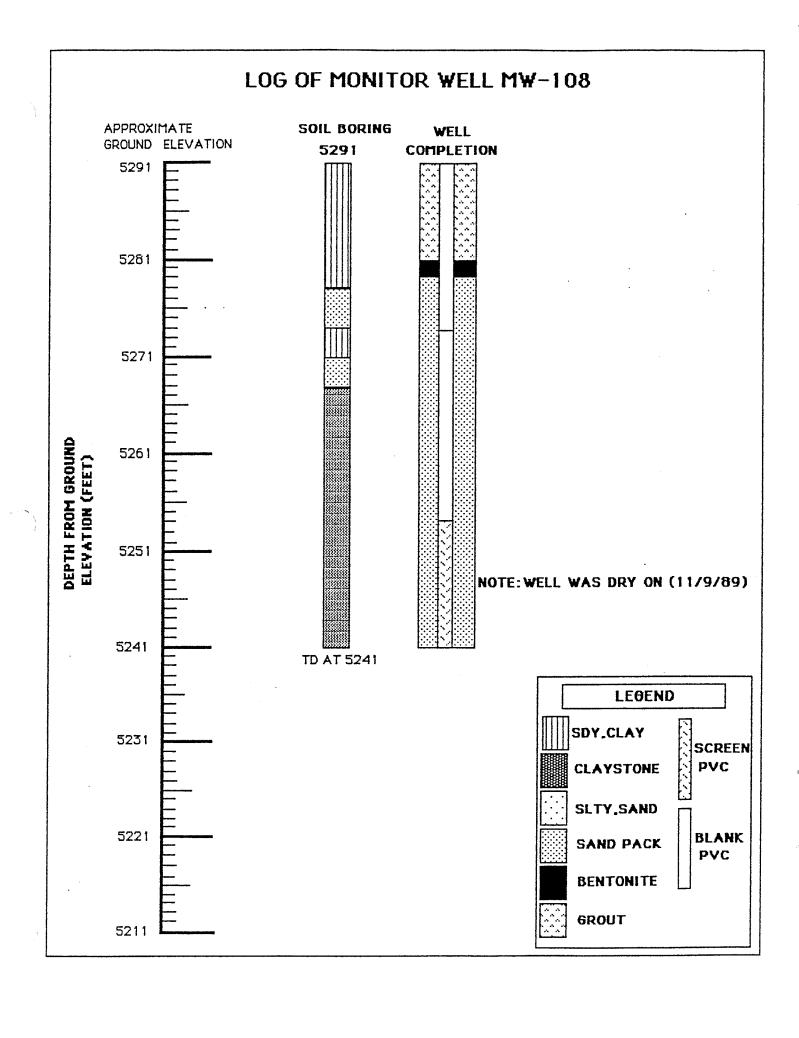


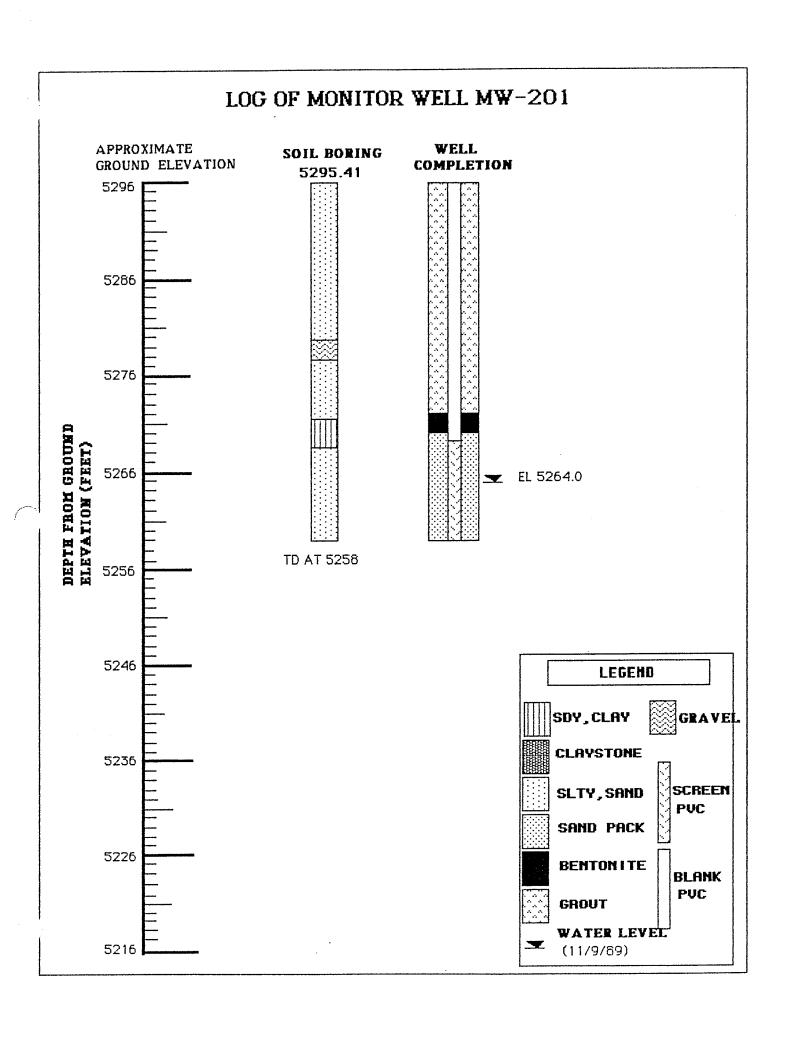




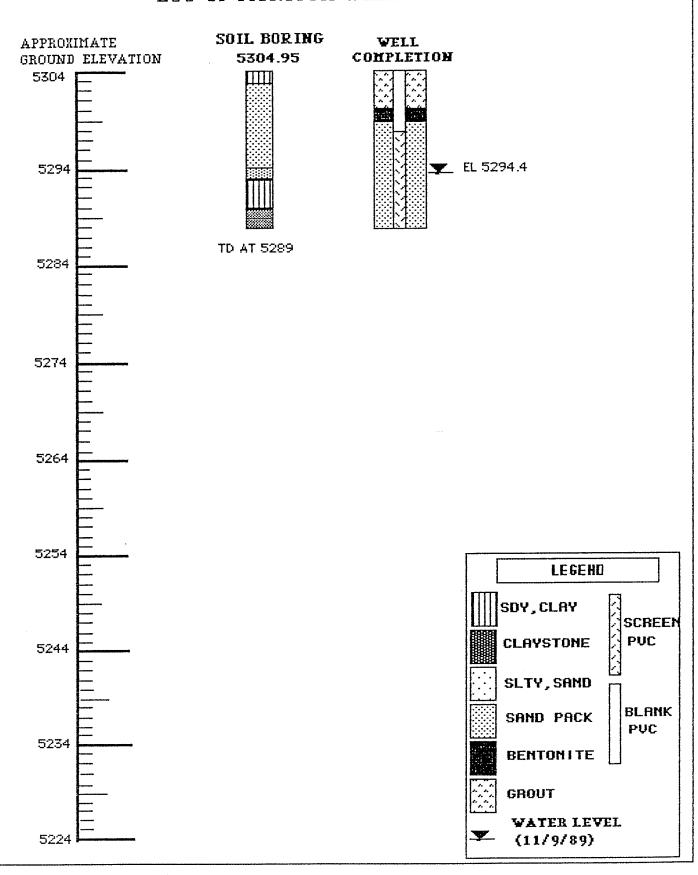


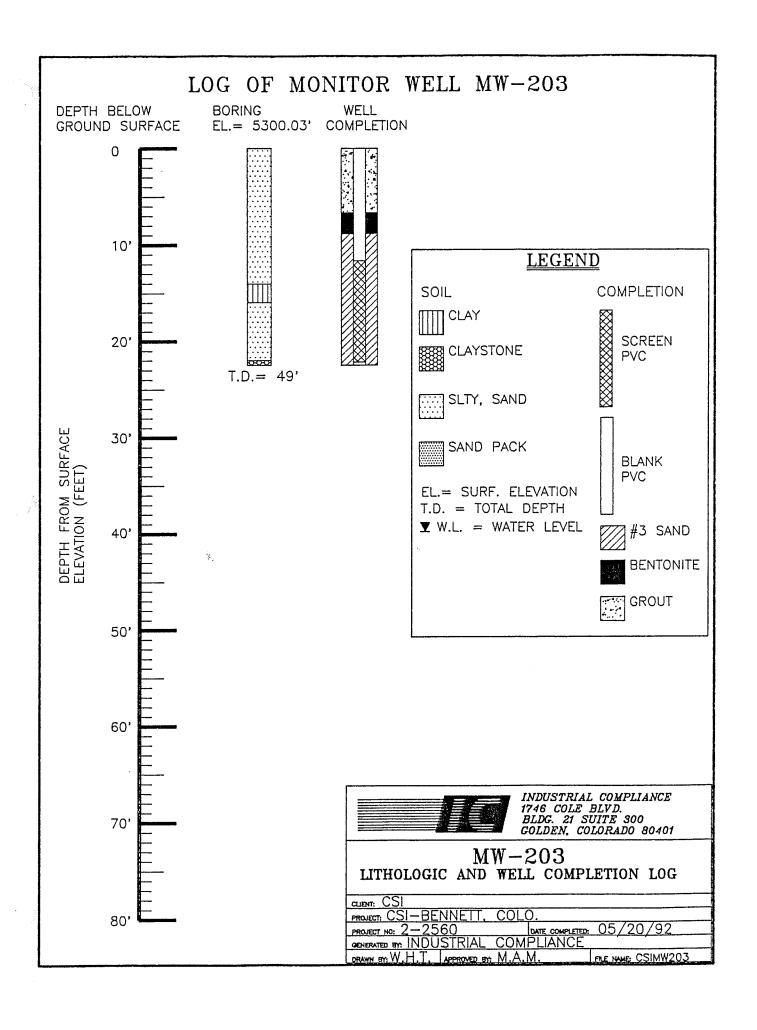


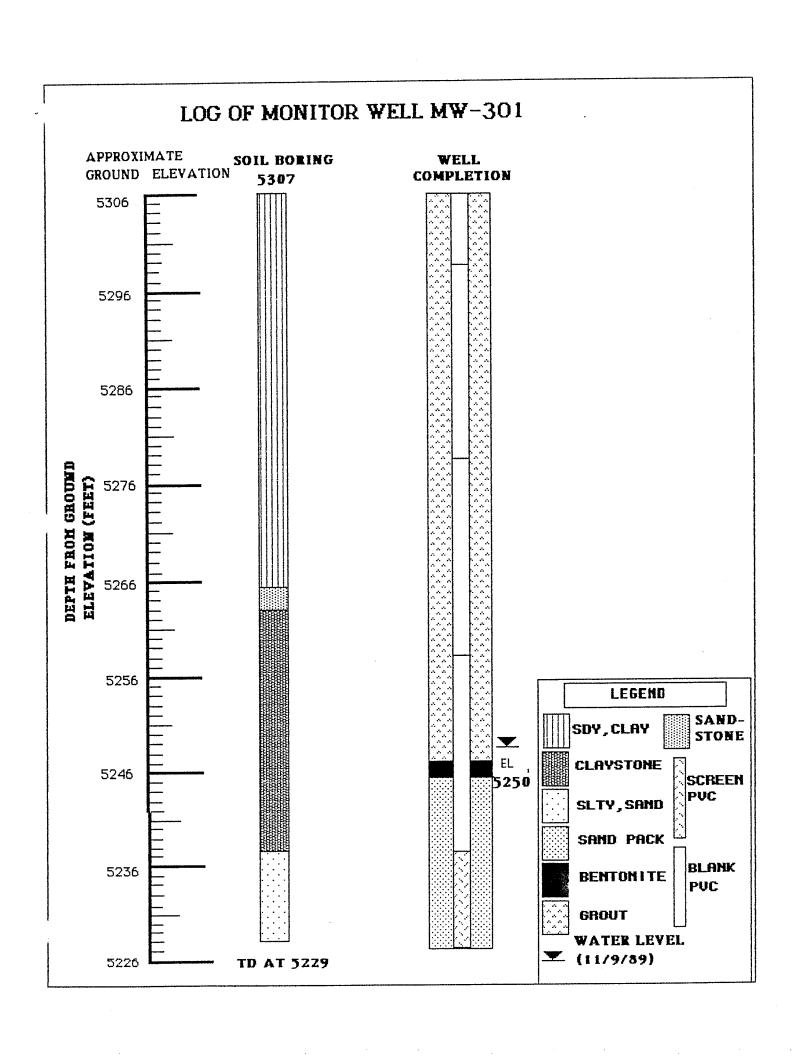


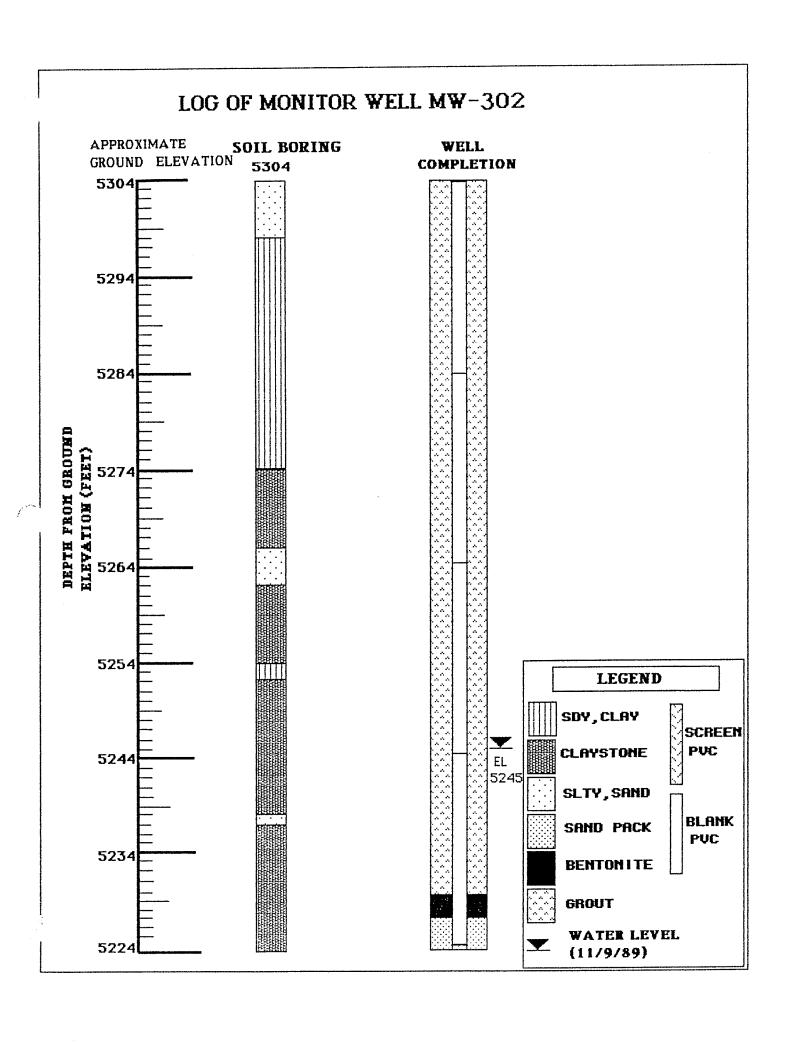


LOG OF MONITOR WELL MW-202

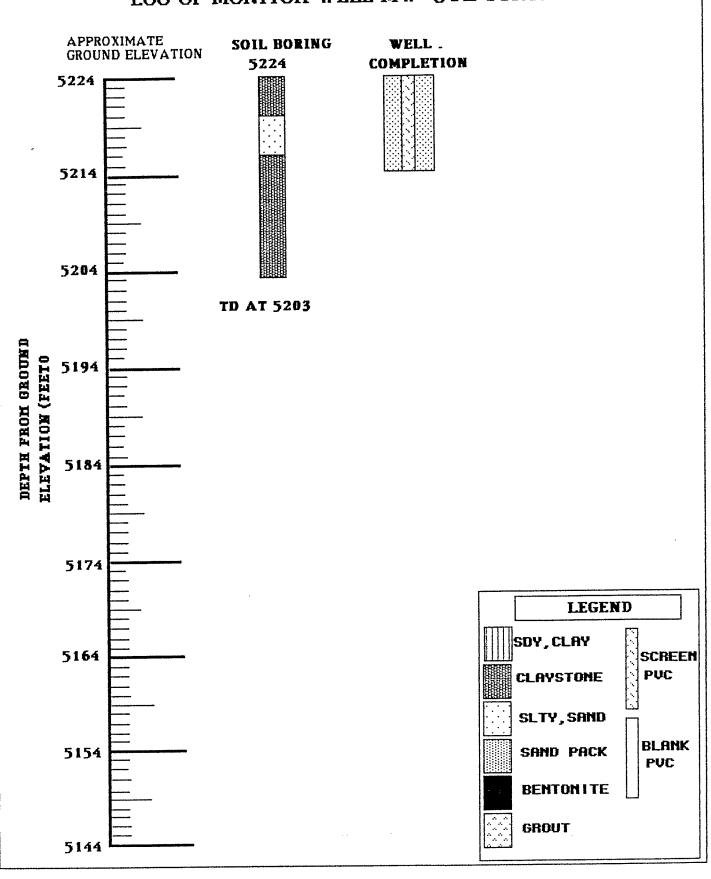


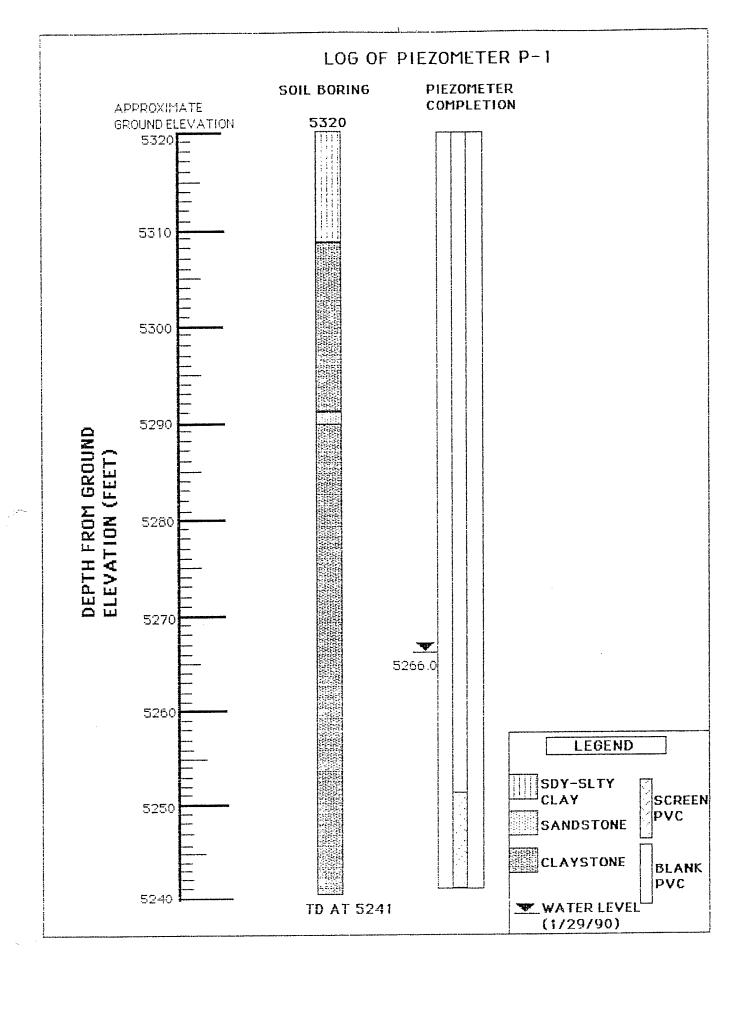


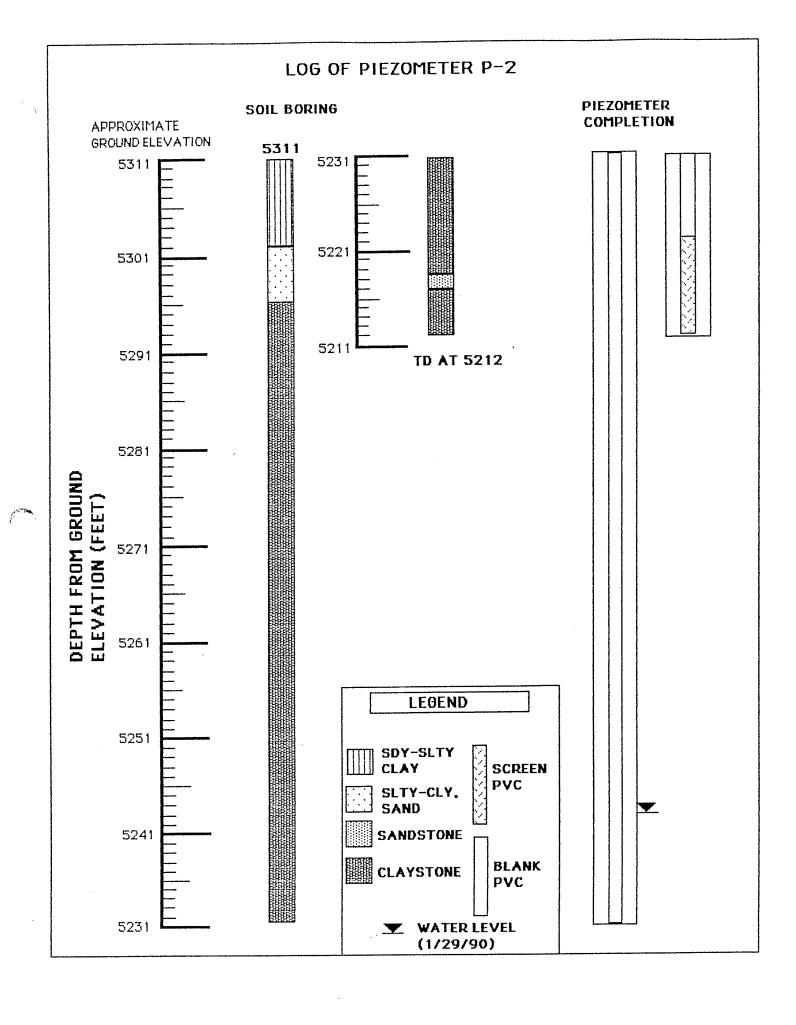


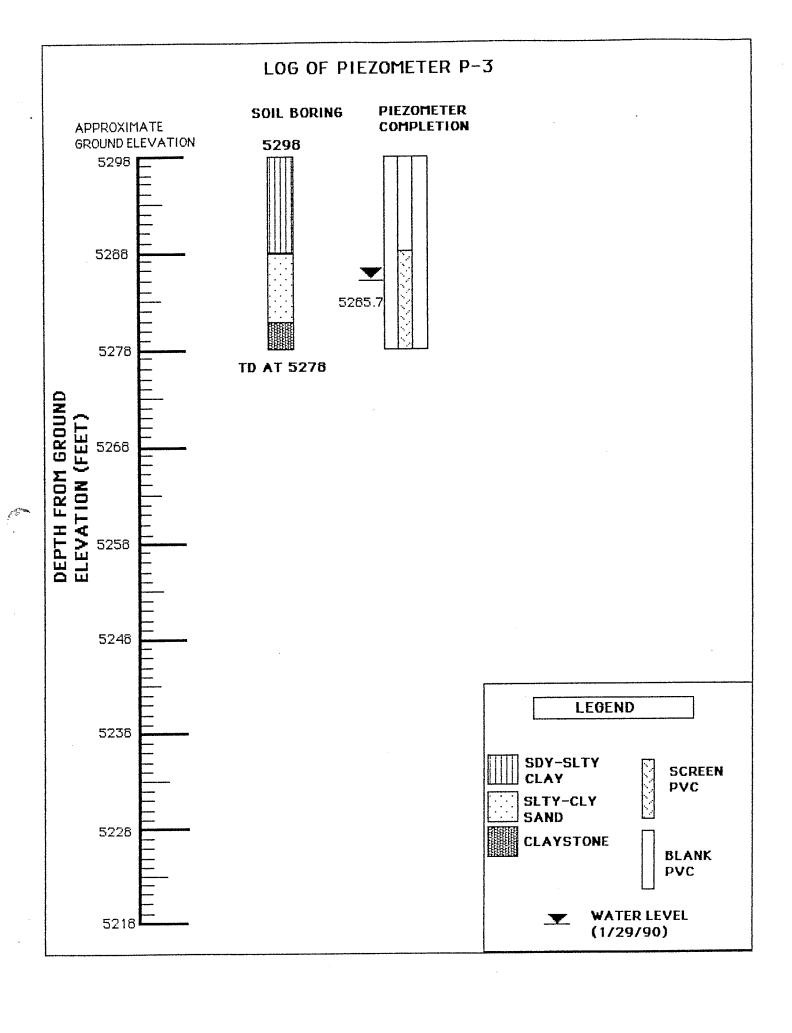


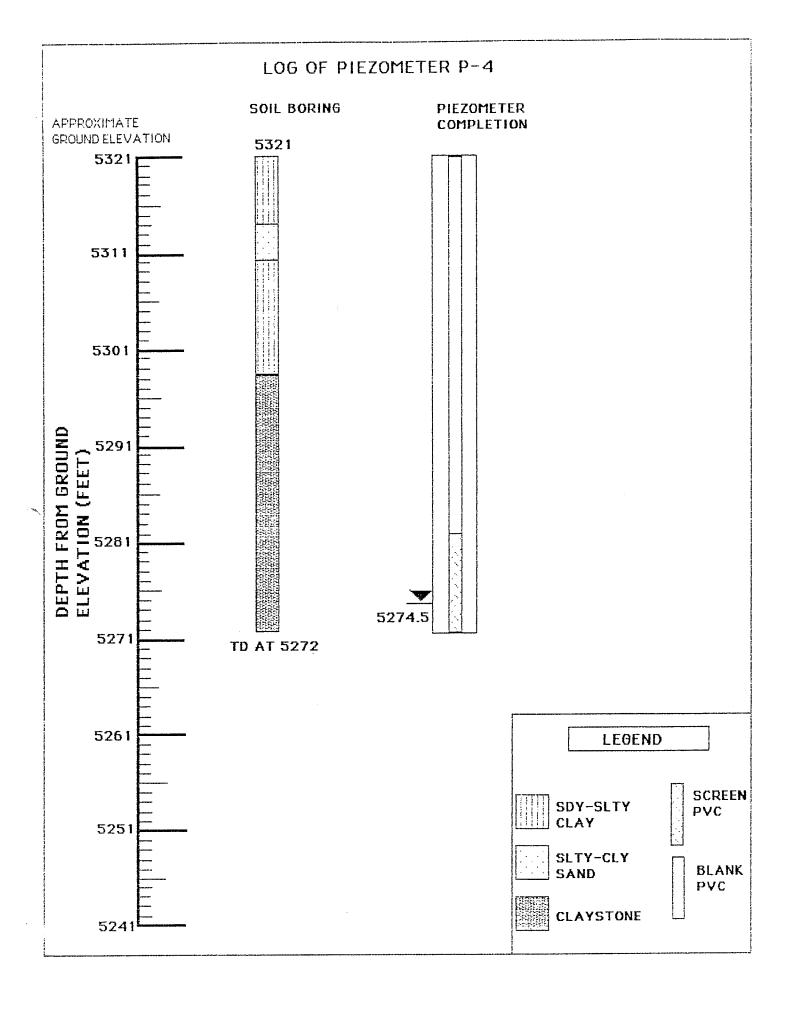
LOG OF MONITOR WELL MW-302 CONT.

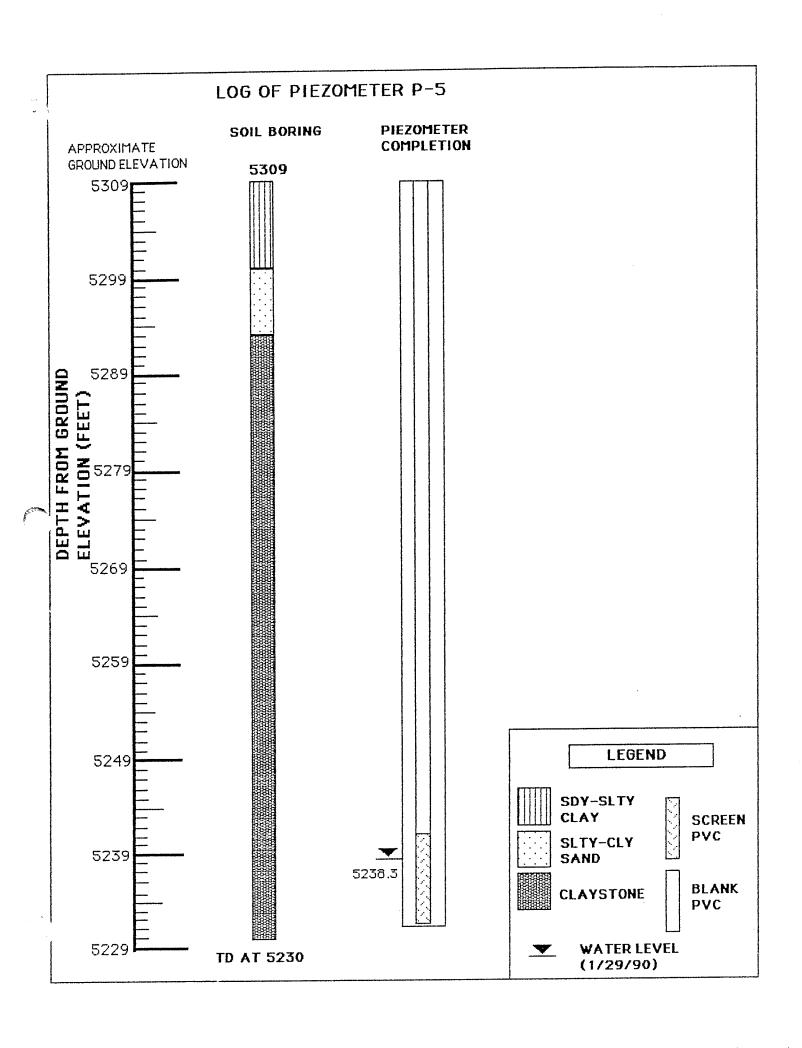


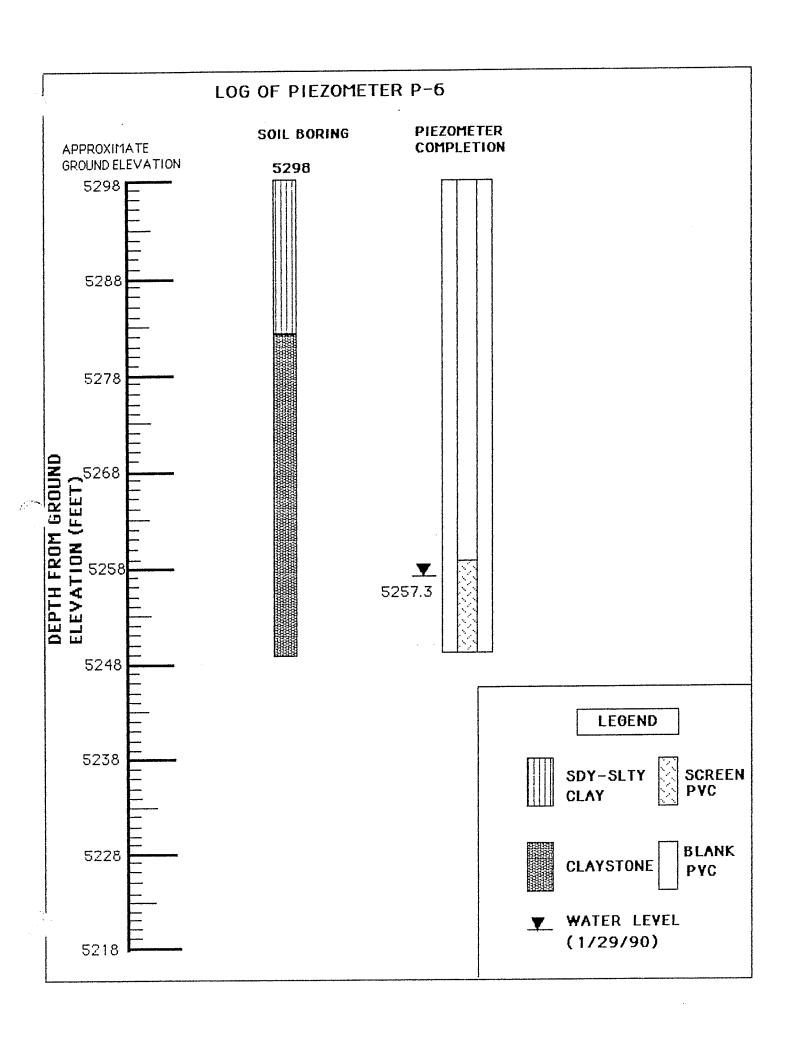


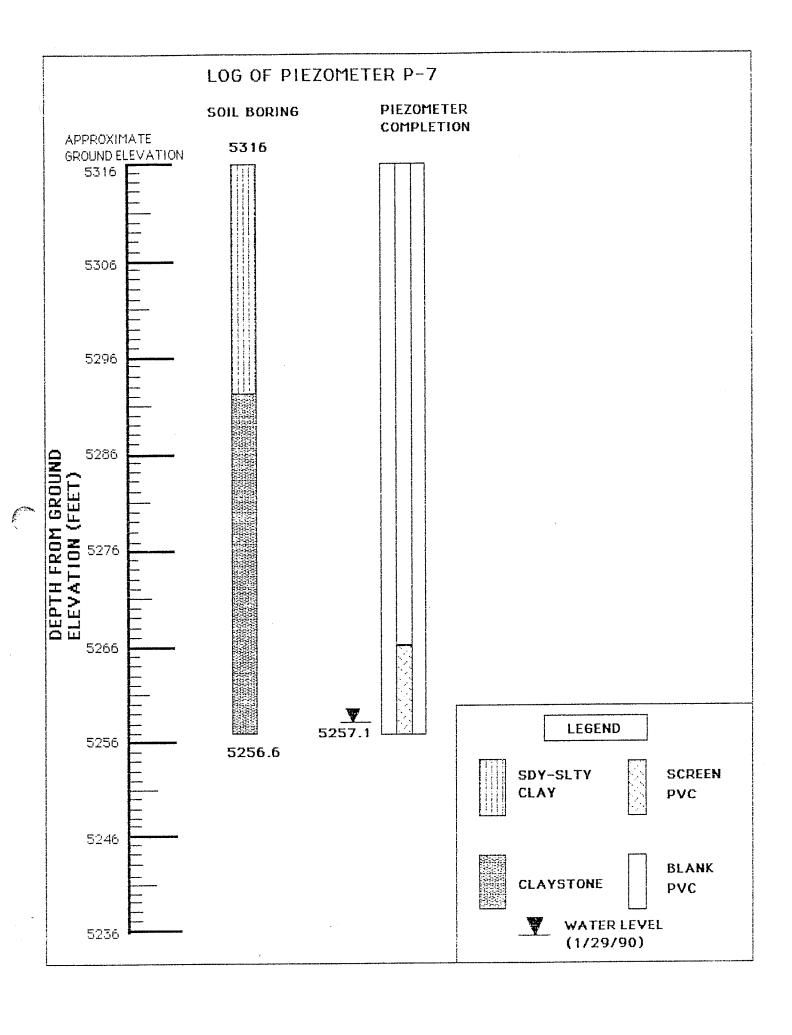


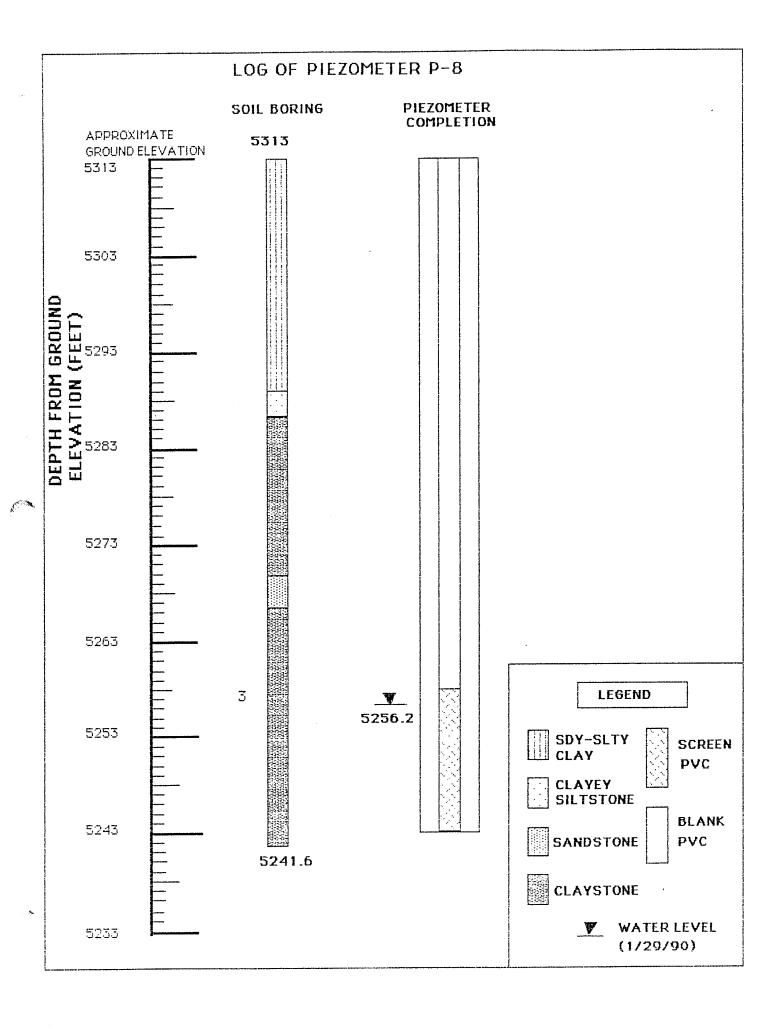


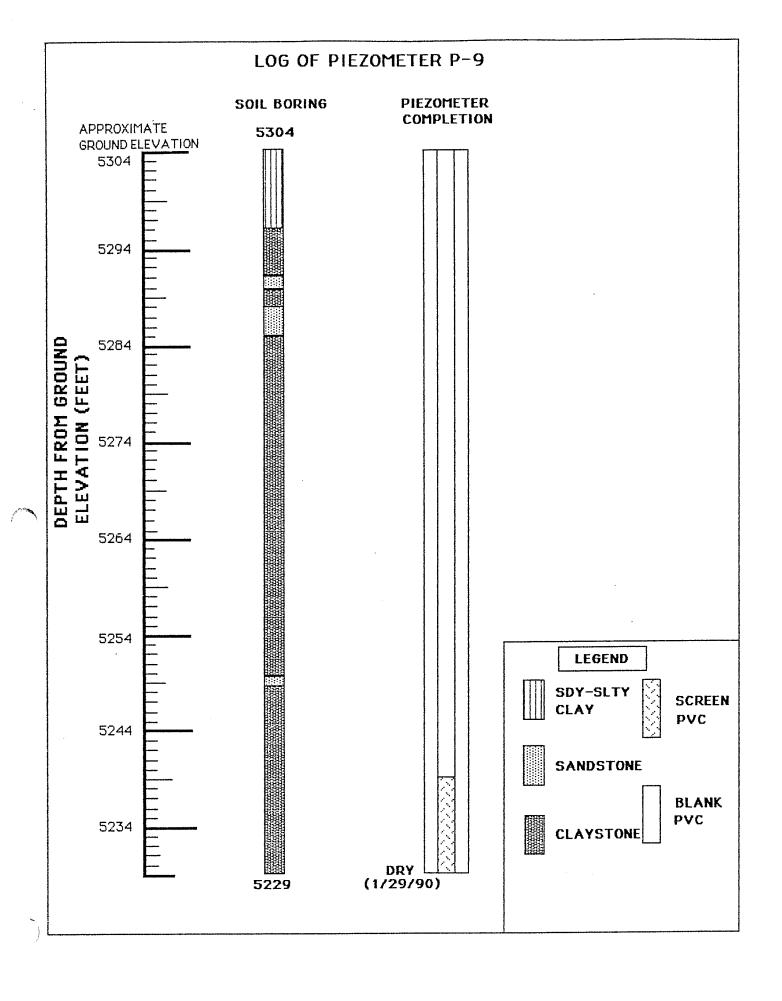


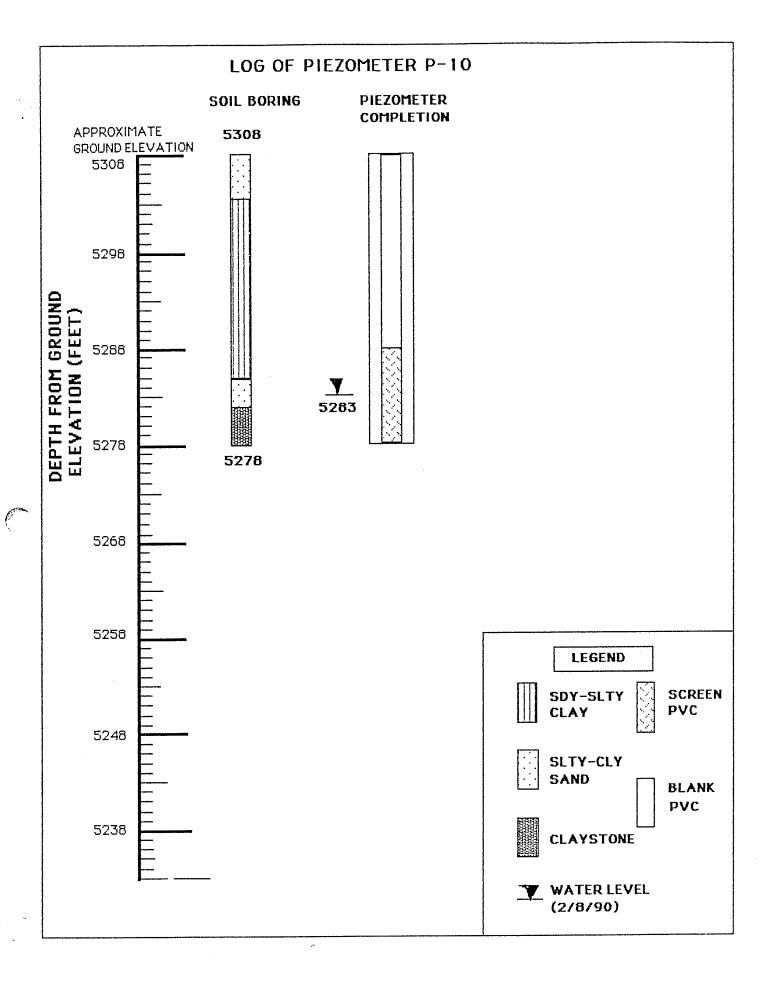


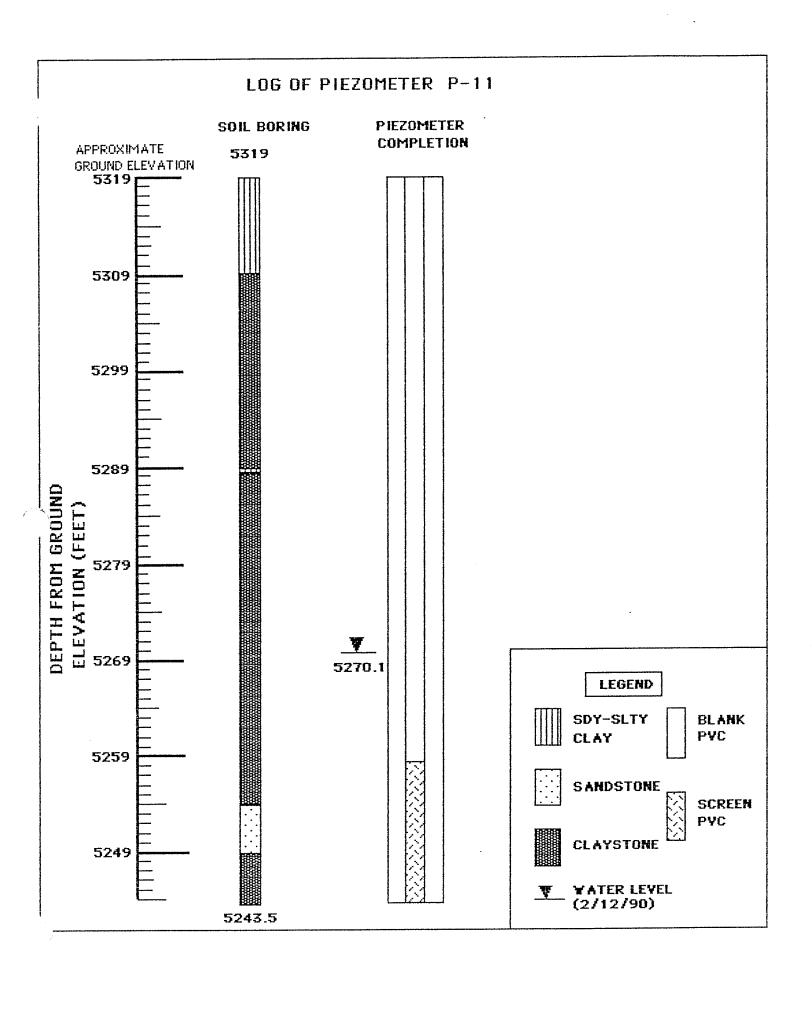


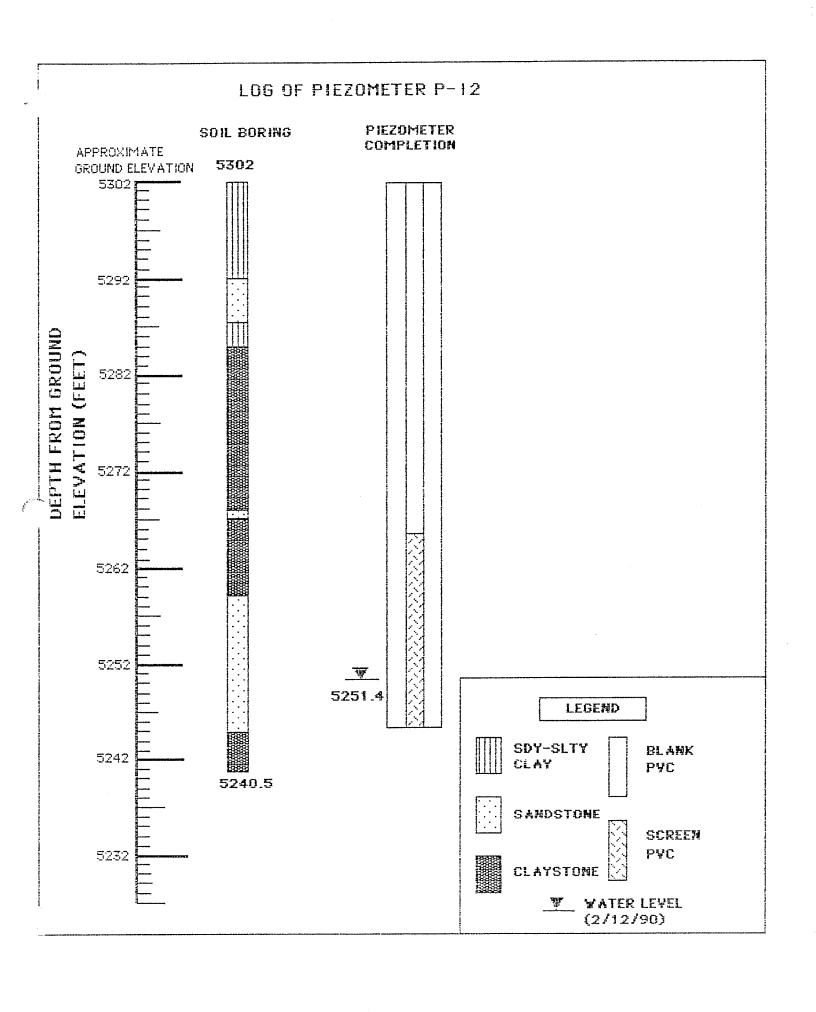


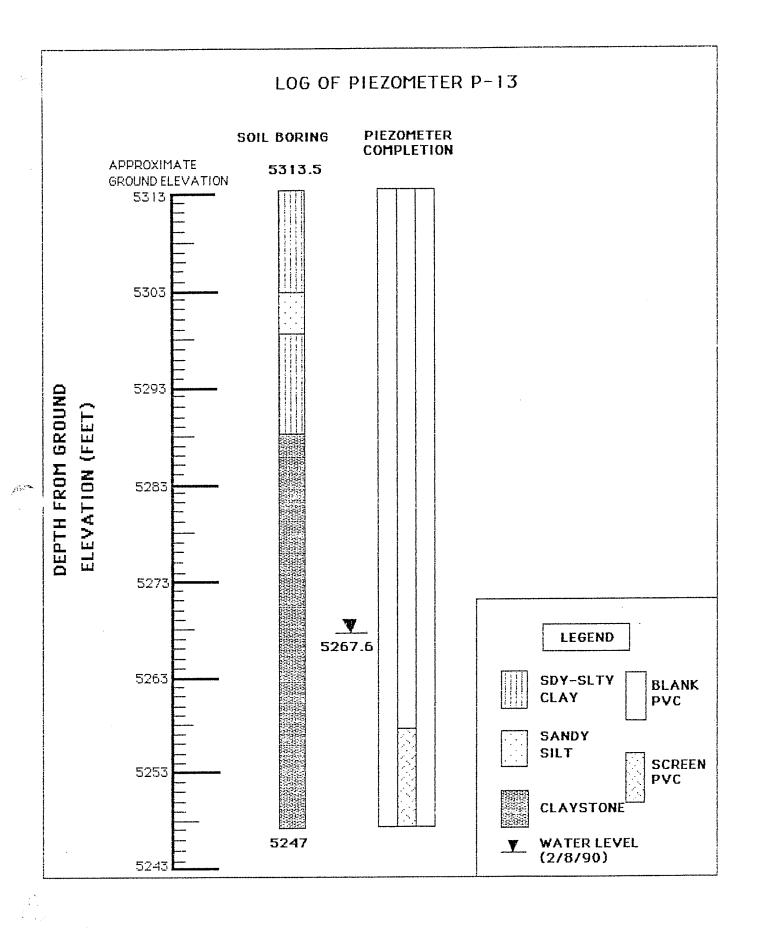


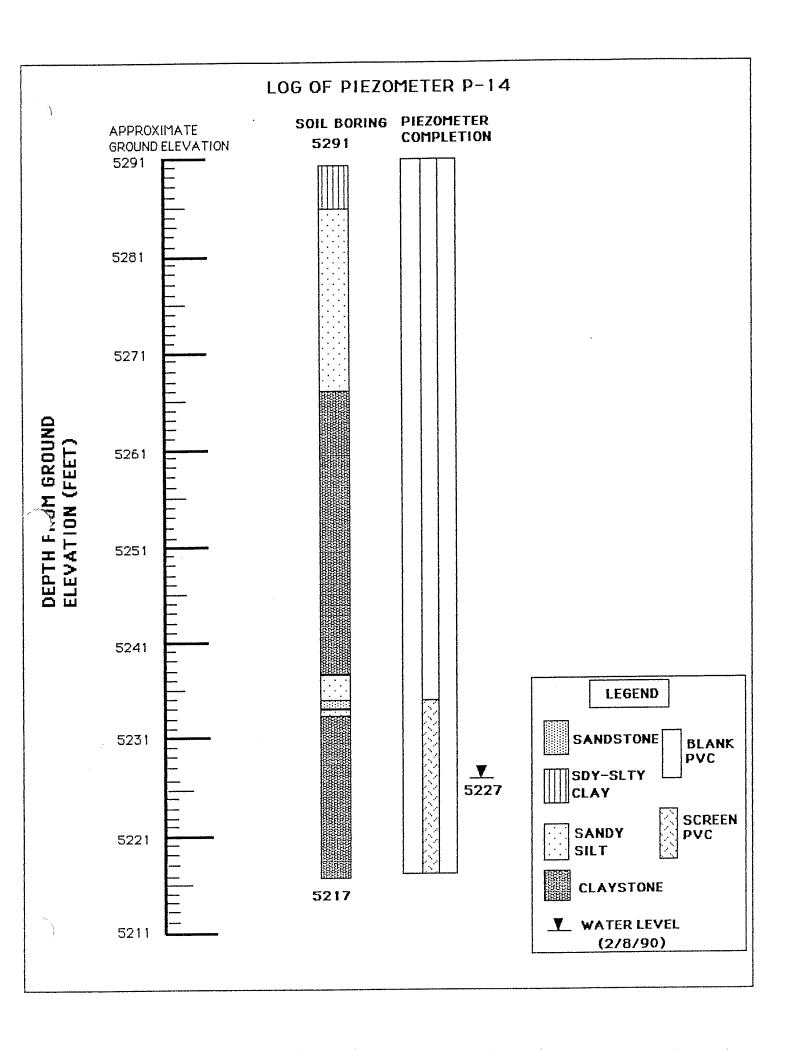


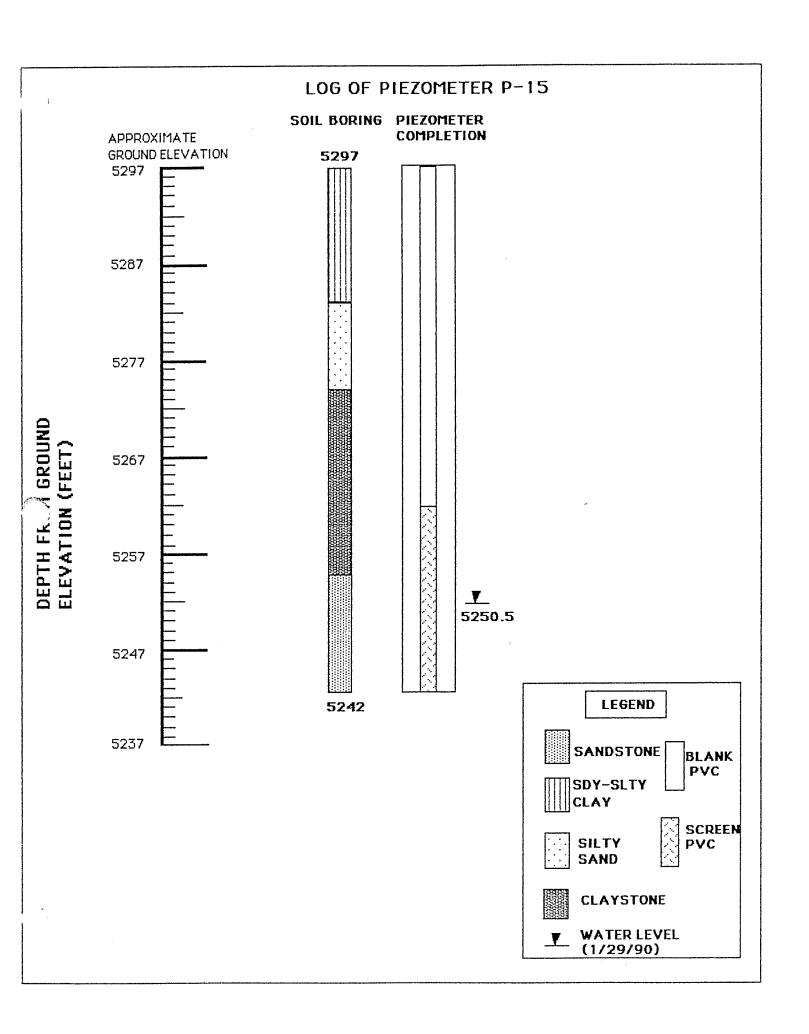


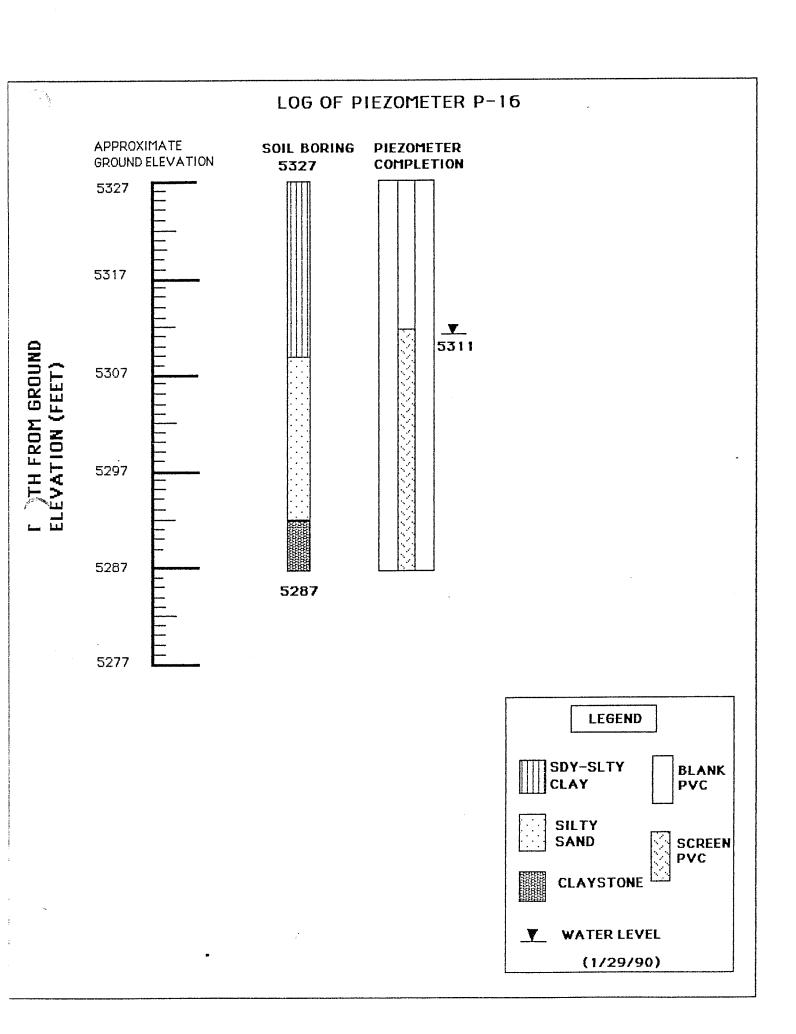


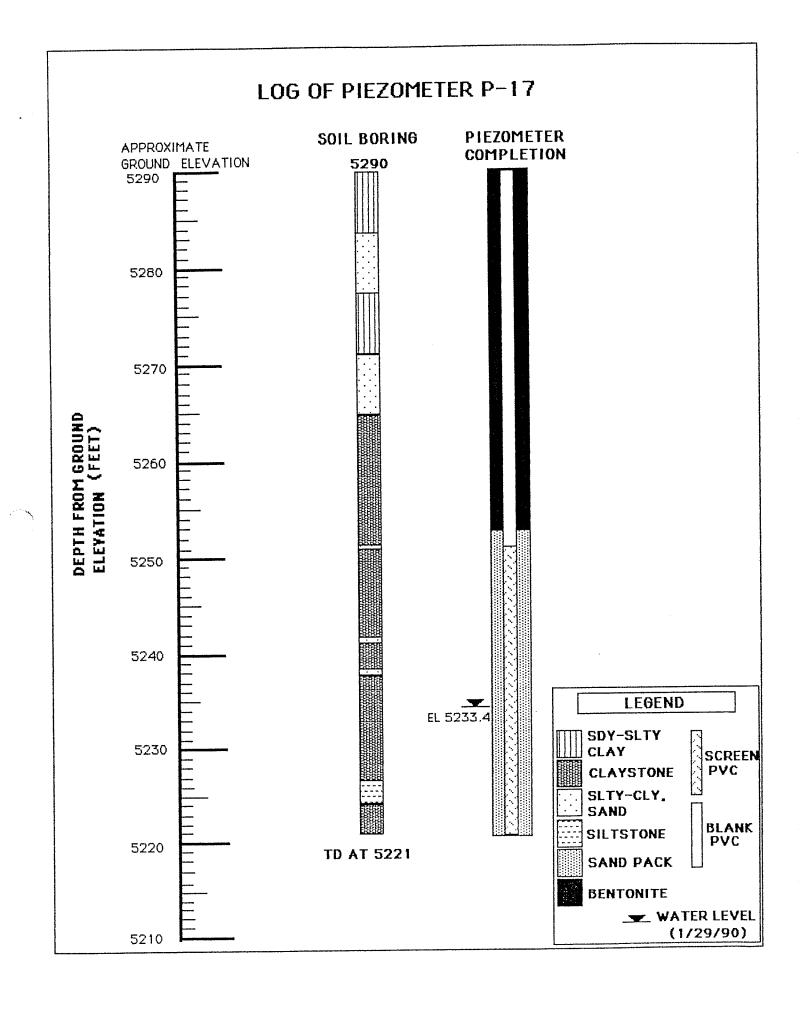


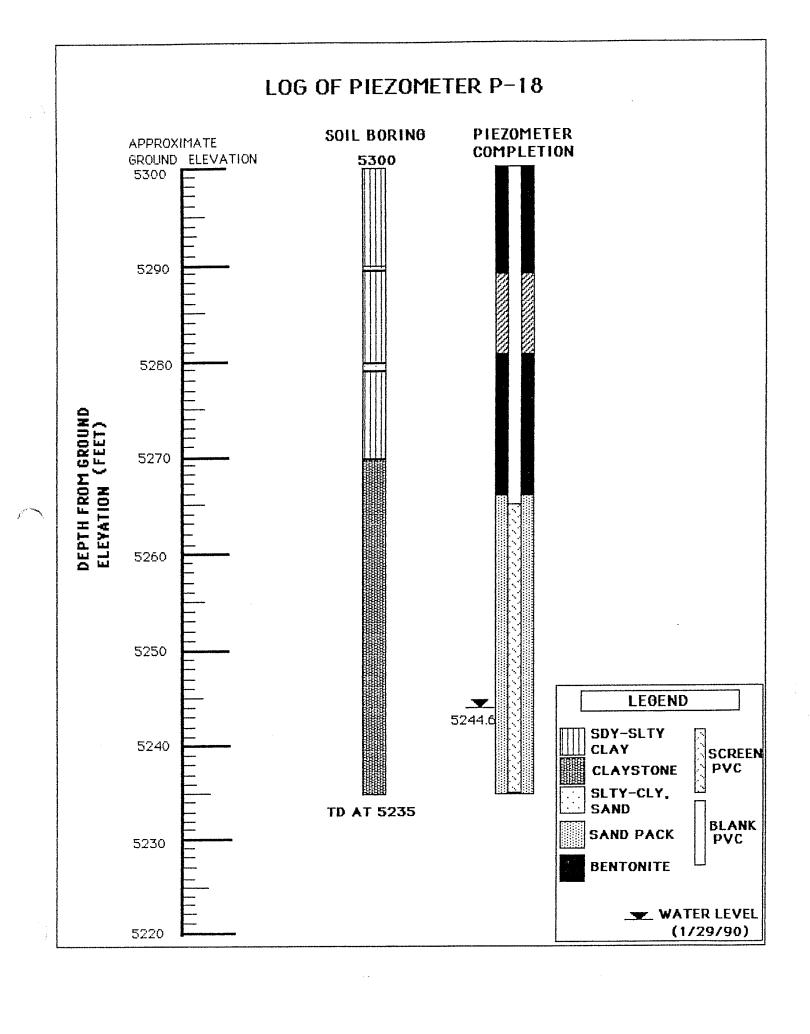


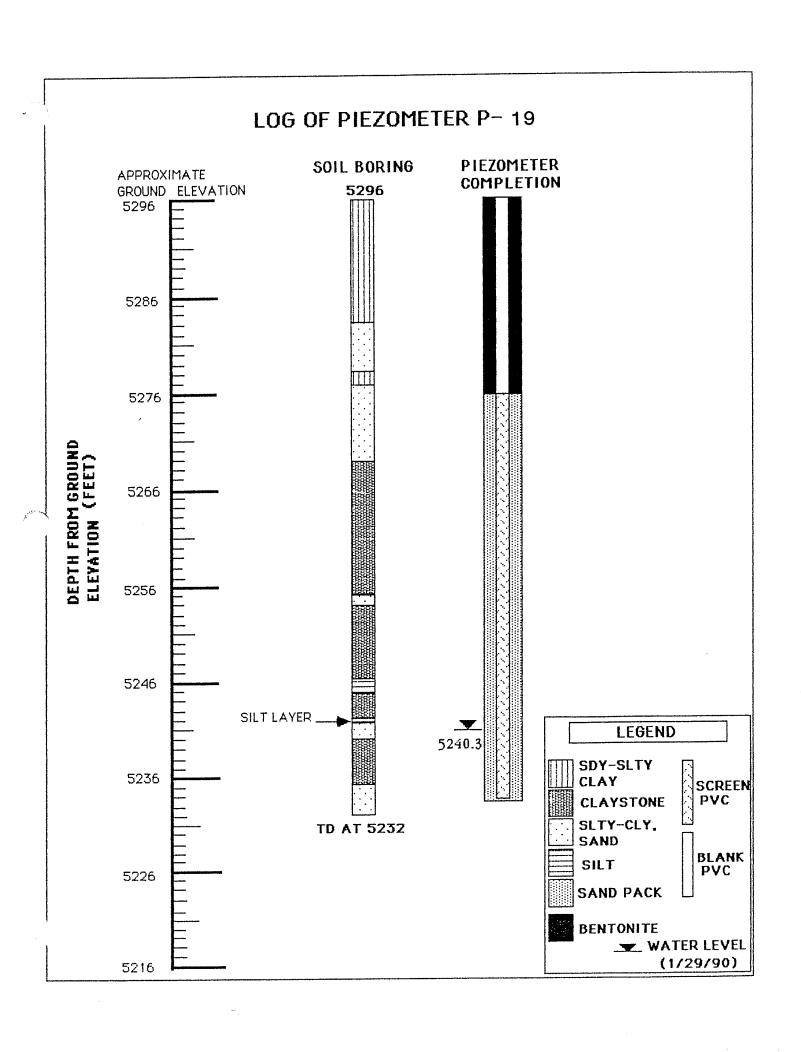


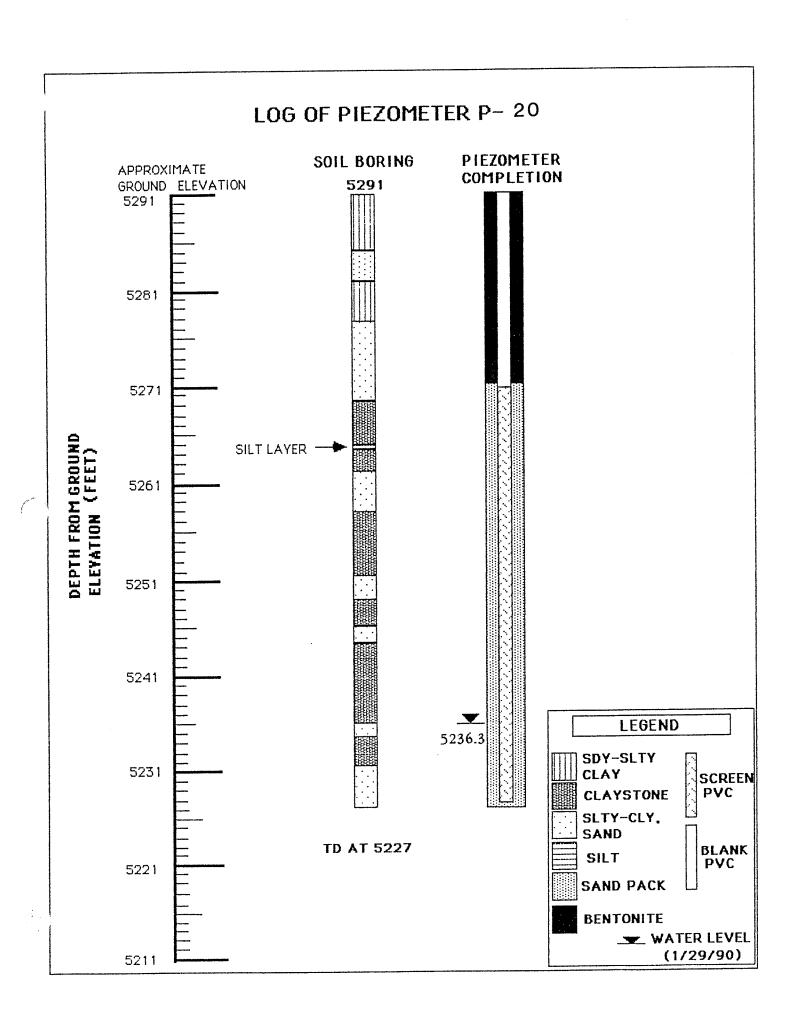


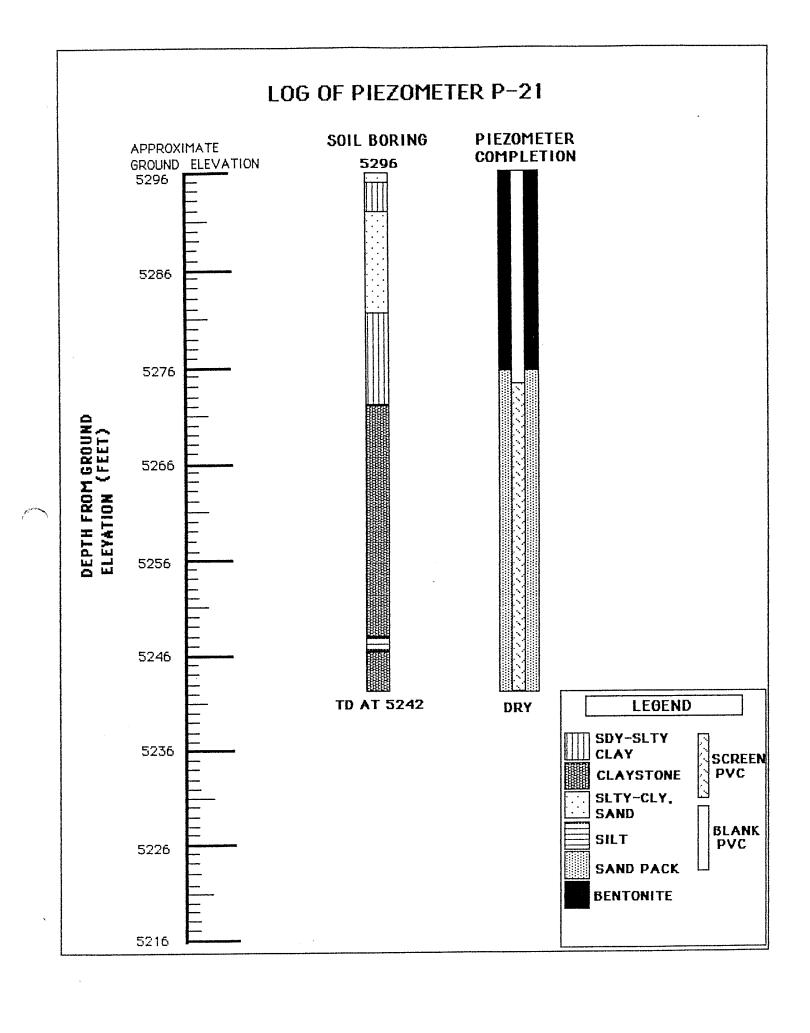


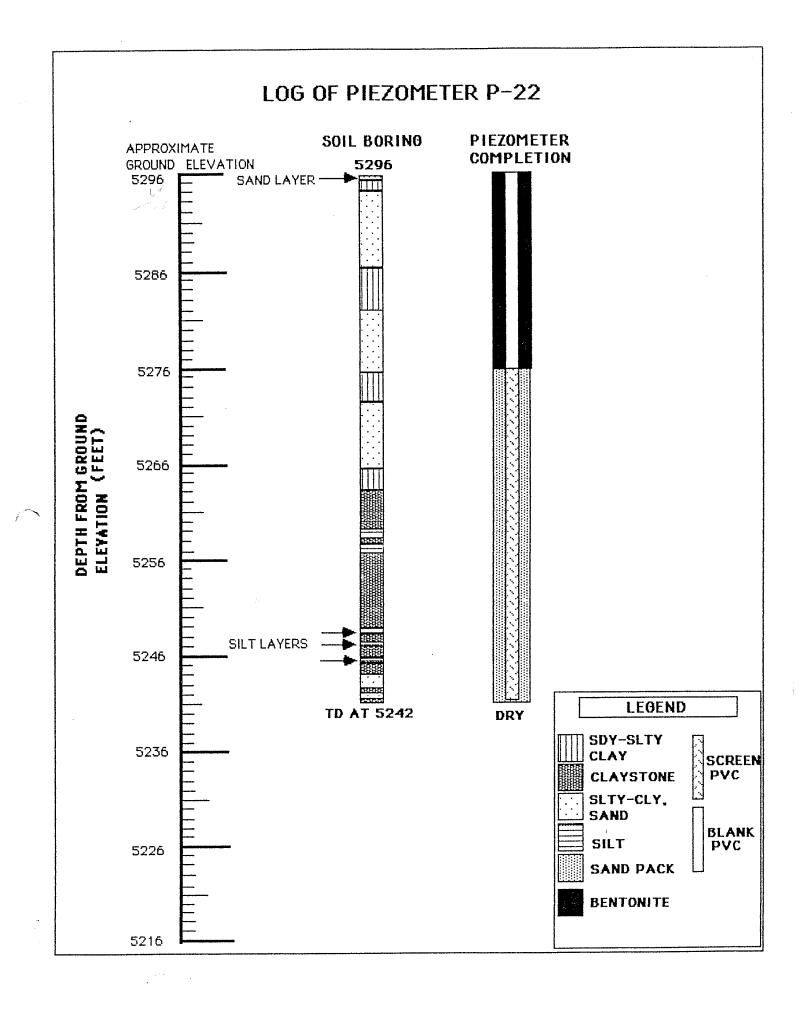


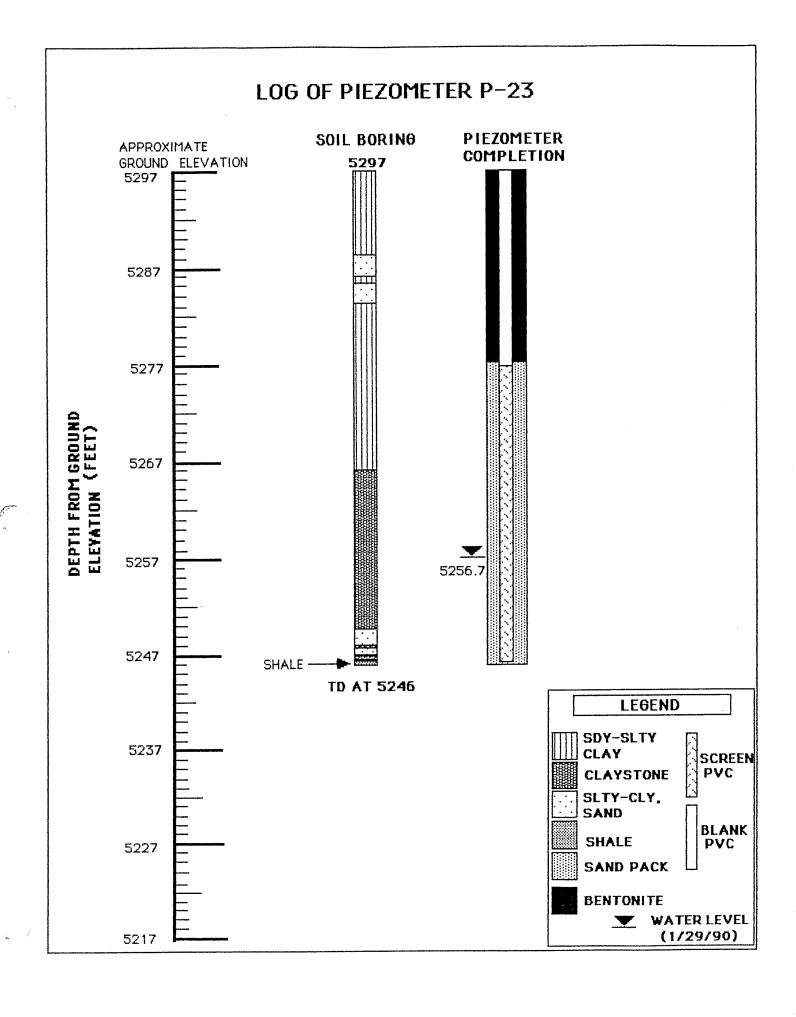


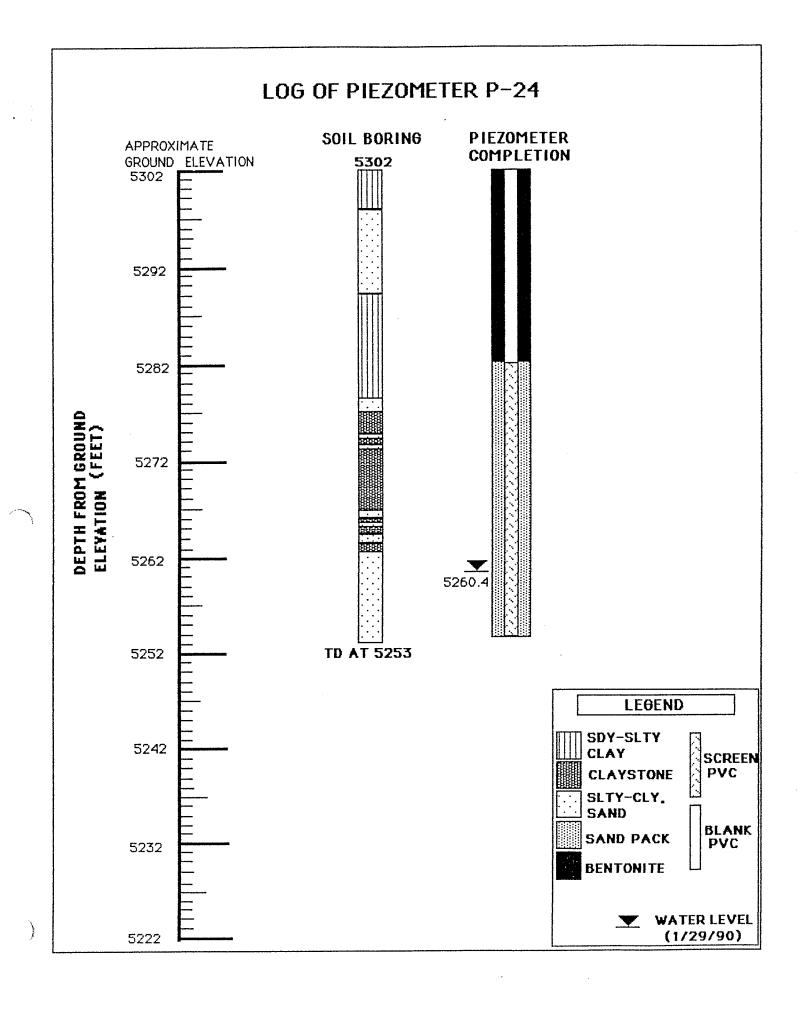


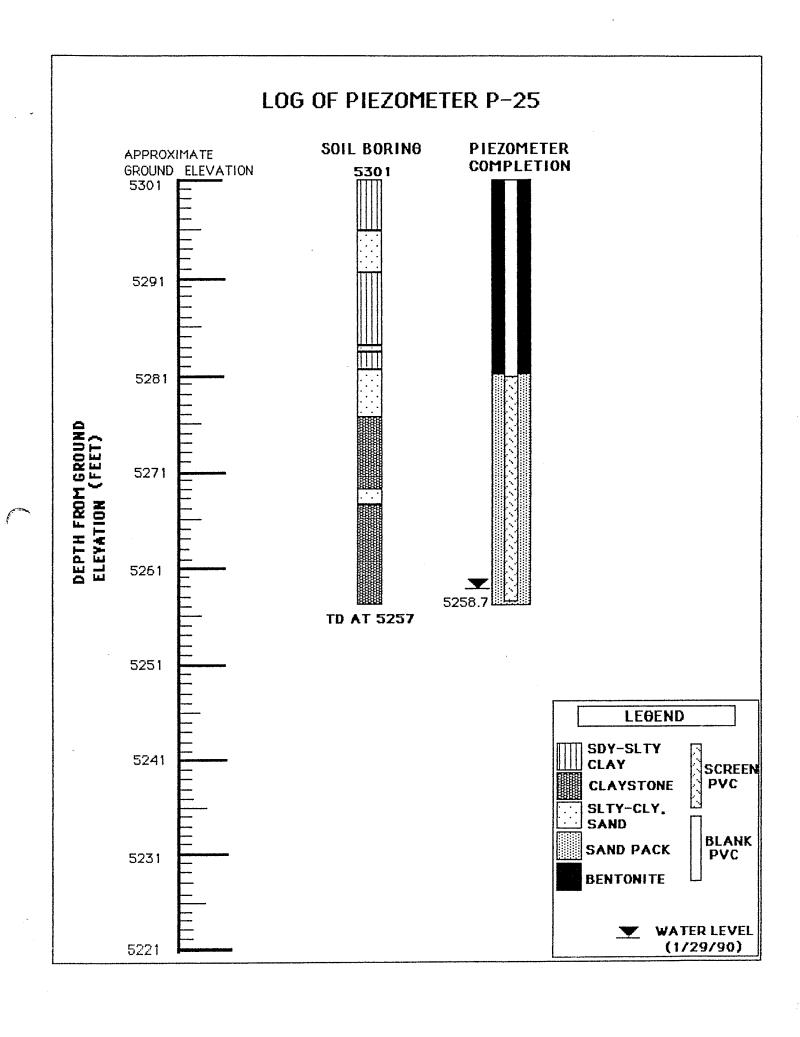


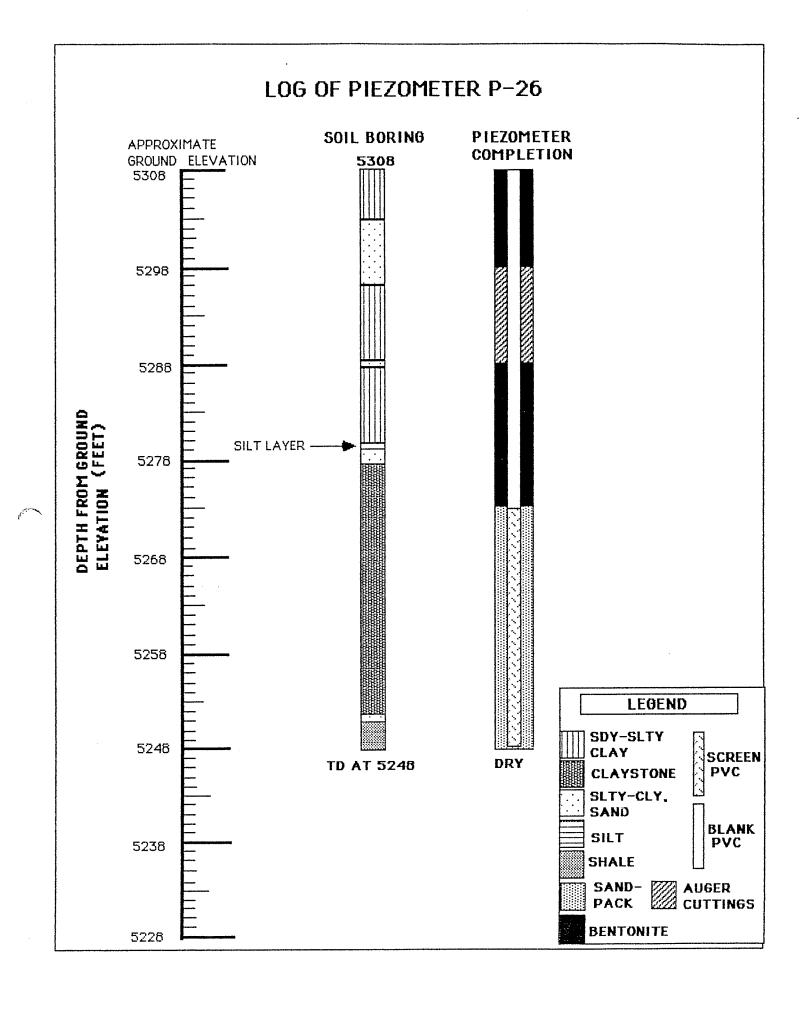


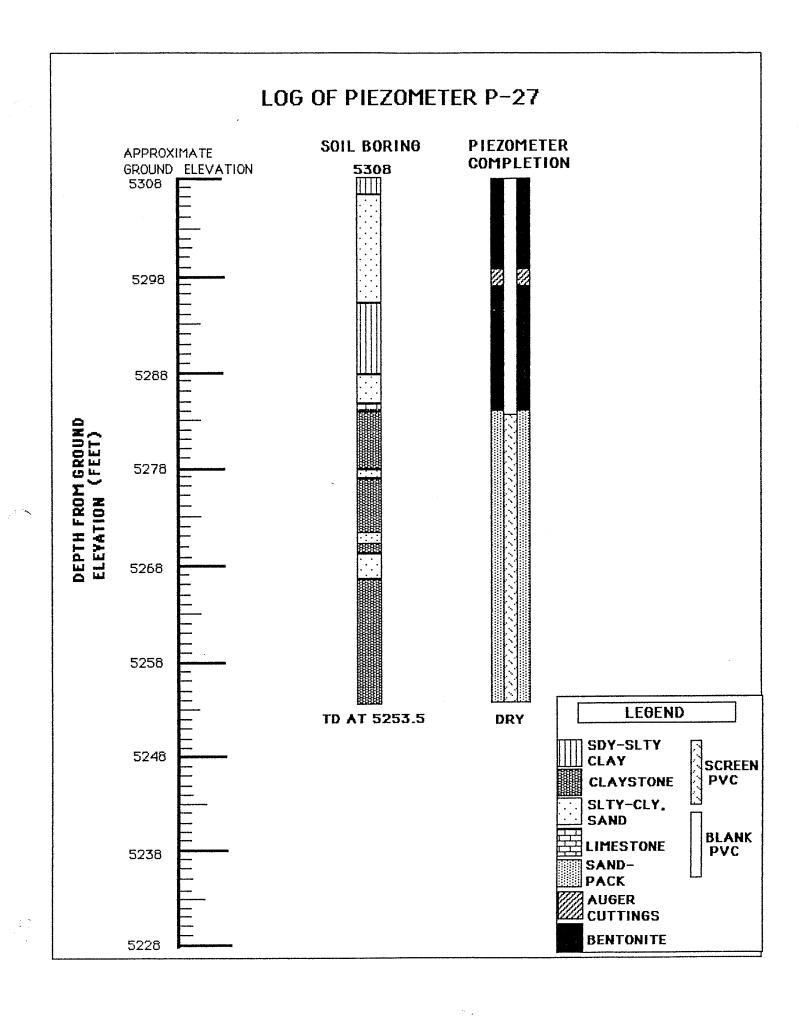


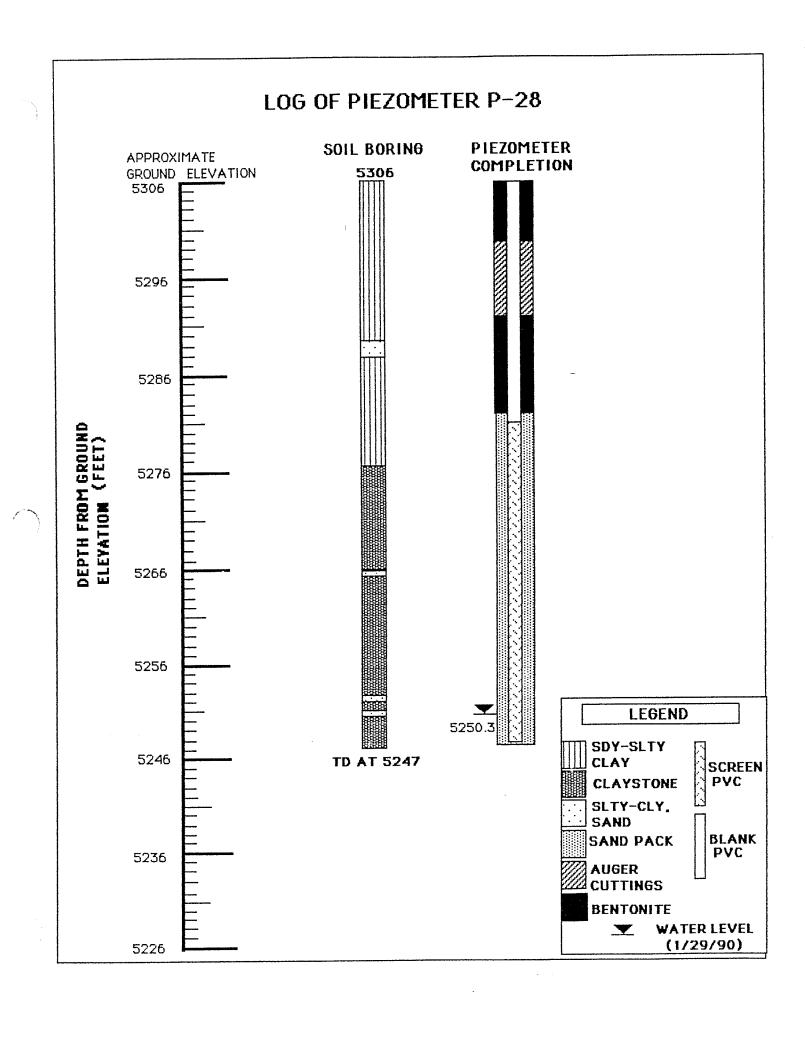


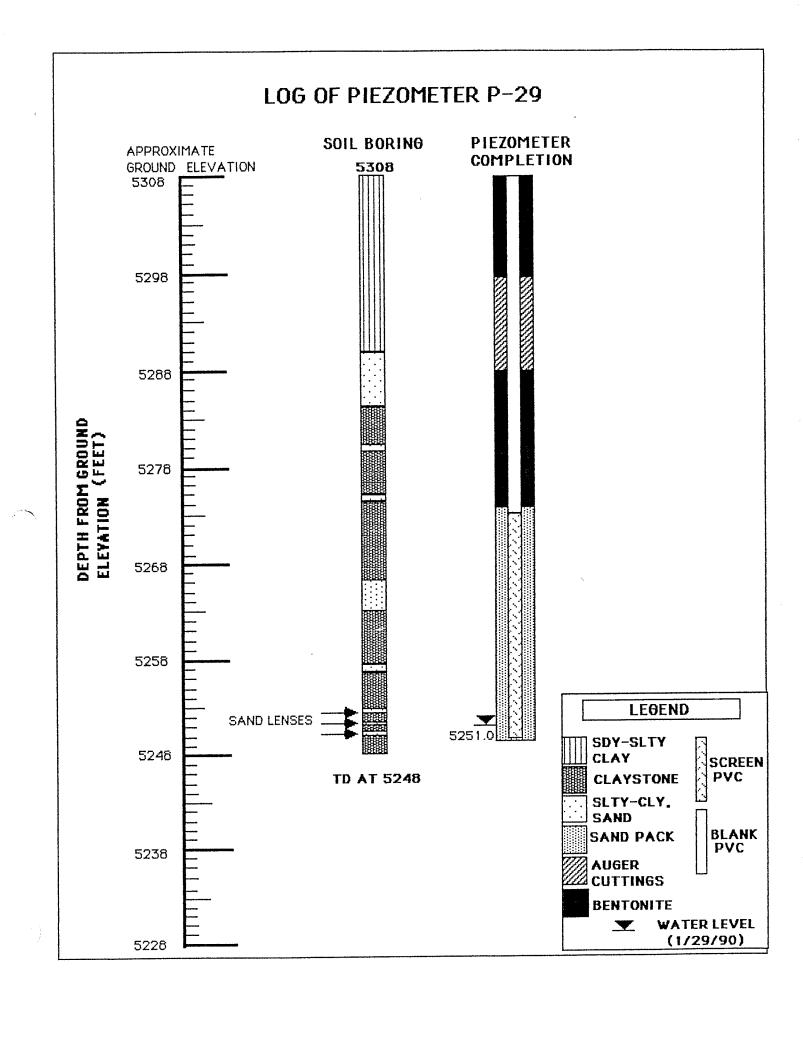


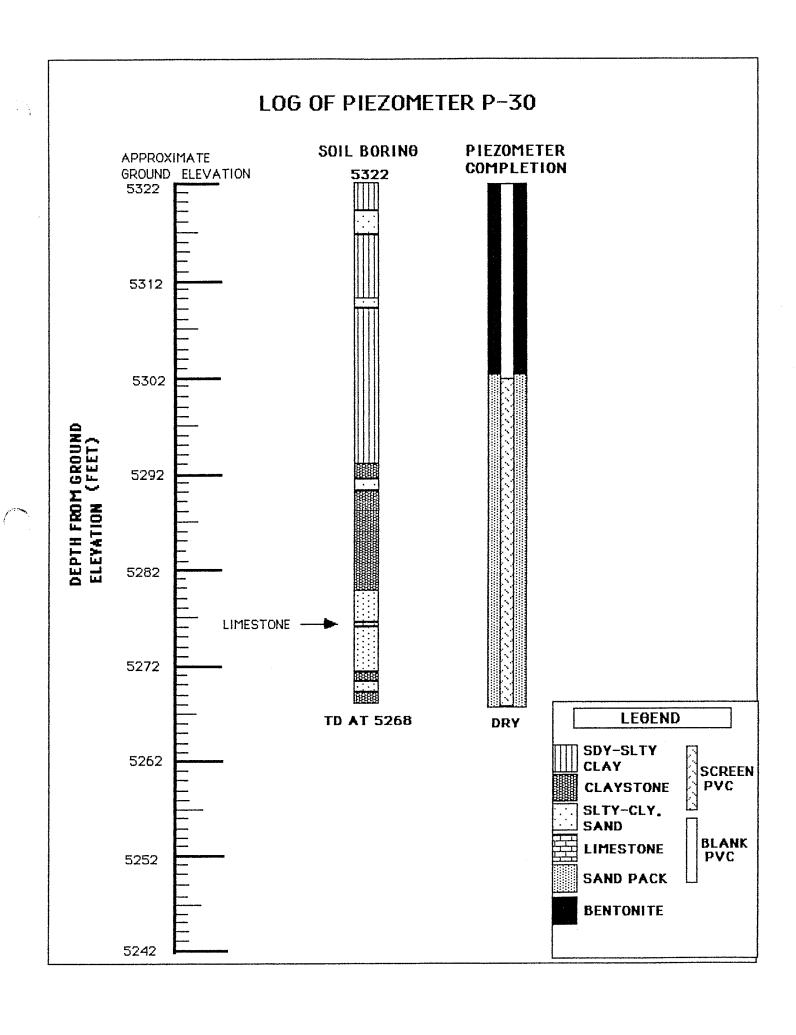


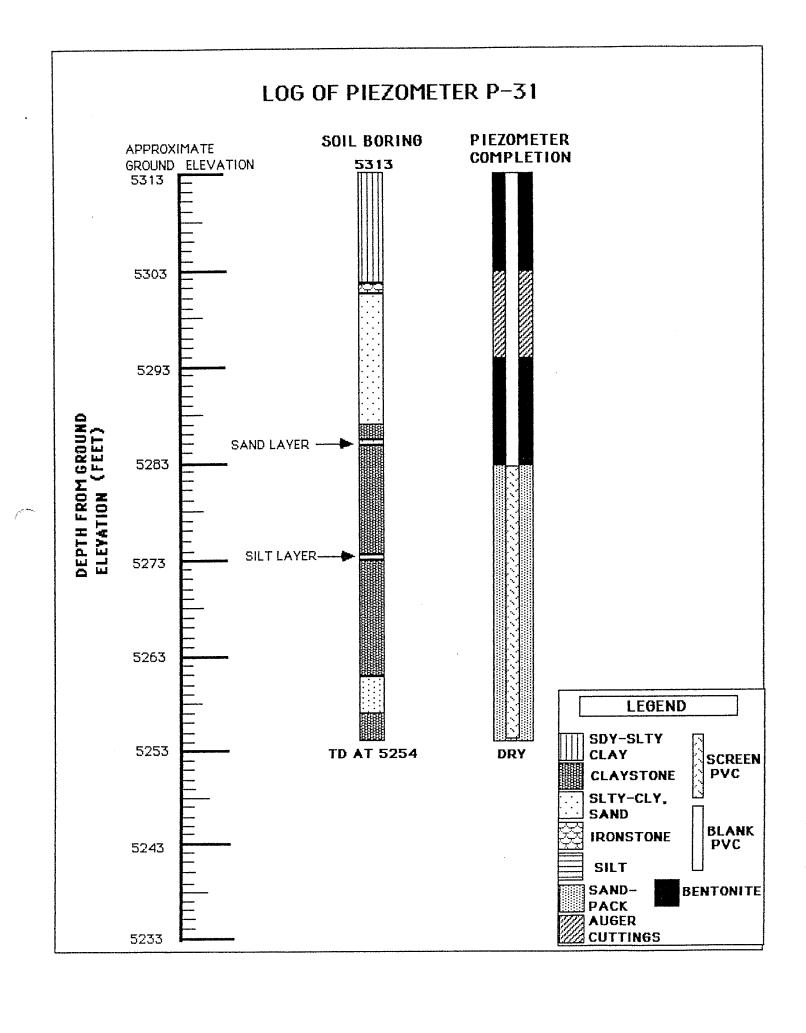


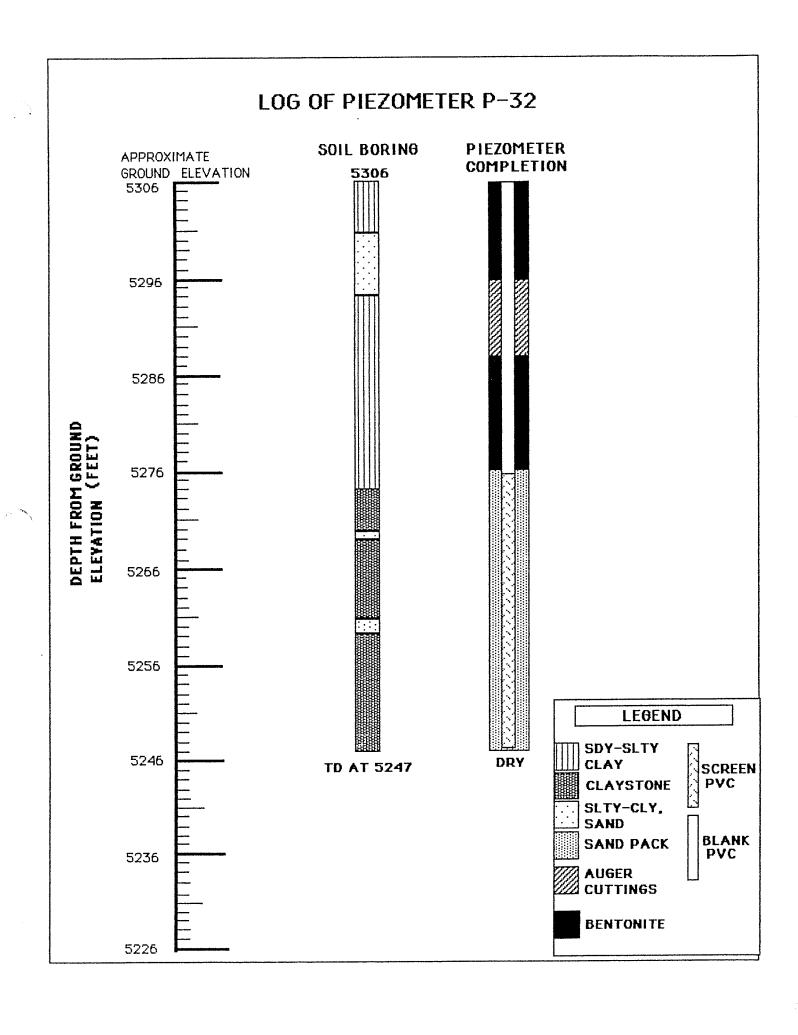


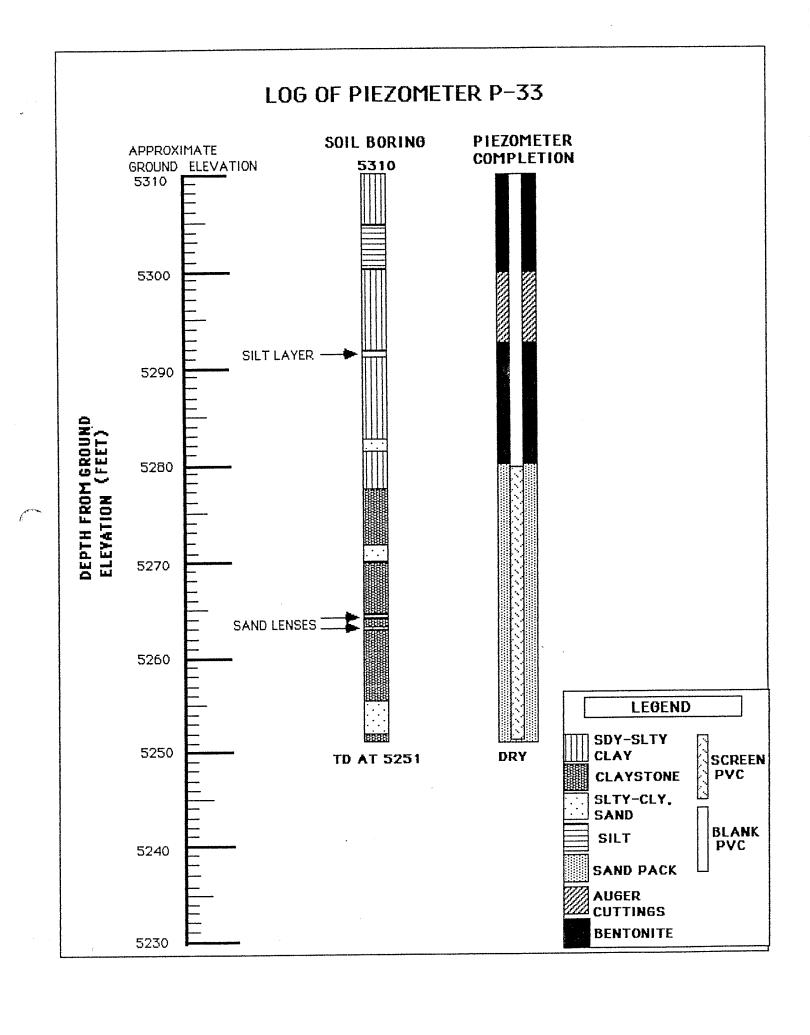


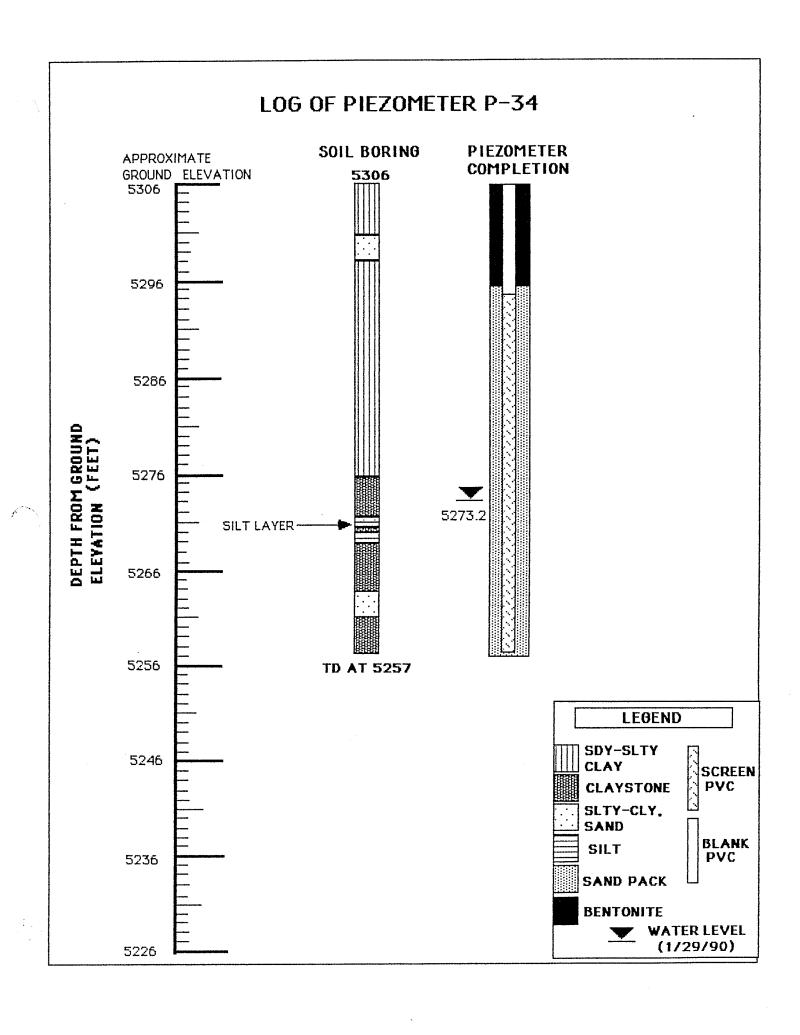


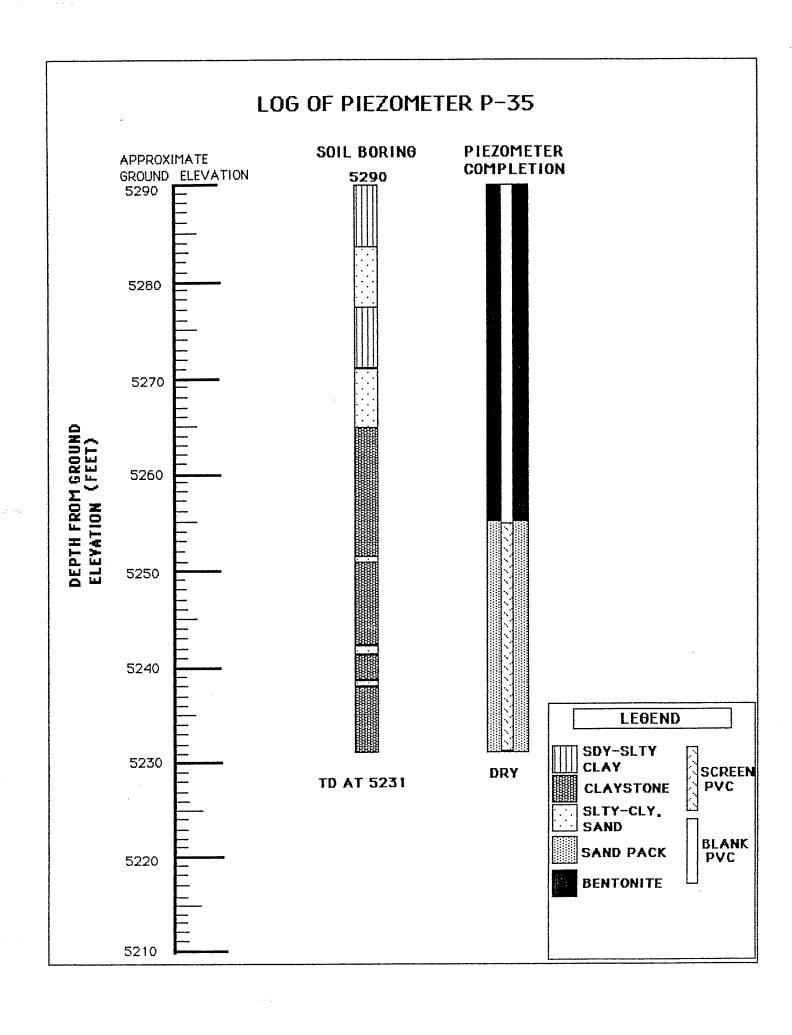


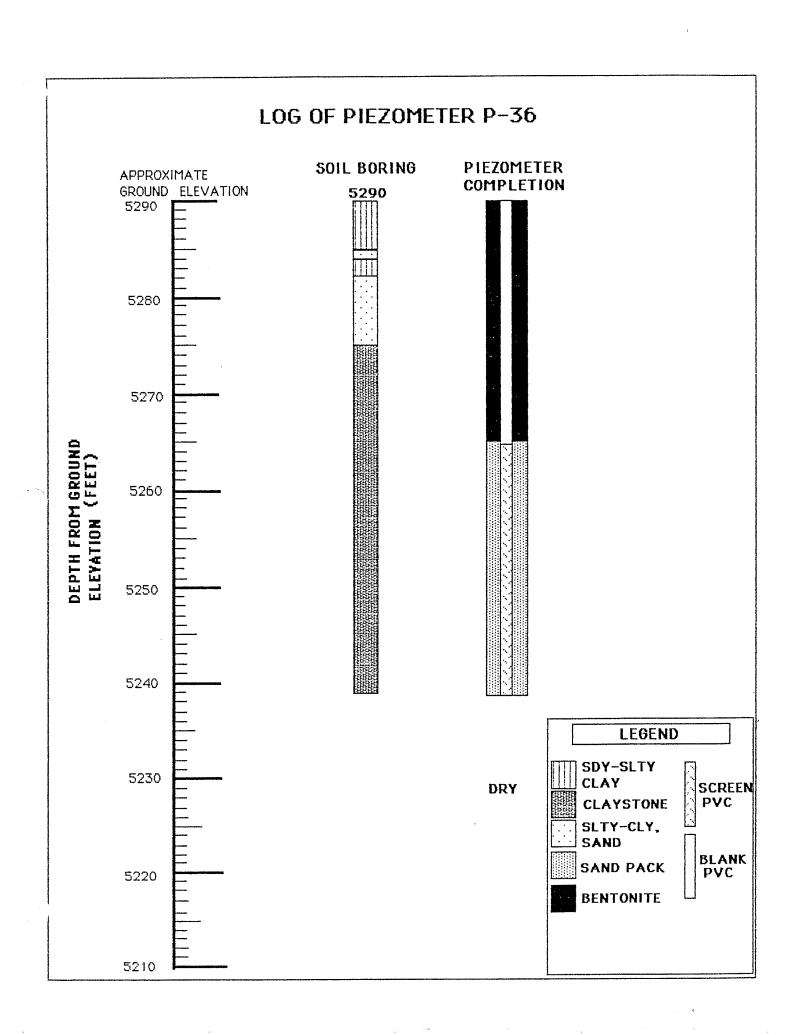












APPENDIX G - WATER QUALITY RESULTS



4036 Youngfield Wheat Ridge, Colorado 80033 (303) 425-6021 RECEIVED OCT 3 1988

September 29, 1988

Industrial Compliance 511 Orchard Rd. Golden, CO 80401

Attn: John Cocroft

Data Report: 88-09-584-5091

Dear Mr. Cocroft,

Attached are your analyses. If you have any questions concerning the reported information, please contact me.

The samples reported here will be disposed of 30 days after the above date unless other arrangements are made.

Thank you for using Evergreen Analytical.

Very truly yours,

John H. Barney

President

encl.

cc: file



4036 Youngfield Wheat Ridge, Colorado 80033 (303) 425-6021

SAMPLE CROSS REFERENCE LIST

Client	Industrial	Compliance,	Inc	
Client	Project #	1815	Lab	Project # <u>5091</u>
Date of	Report	September 2	29, 1988	
Evergree Sample 1		Client Sample N	<u>lo.</u>	<u>Analysis</u>
X5635 A		MW-1		Dissolved Metals, Ca, Fe
X5635 B X5635 C X5635 E	/D	MW-1 MW-1 MW-1		Mg, K, Na Pesticides TOC, TOX Cl, TDS, Spec. Cond., Alka, SO4 ² , NO3
X5636 A		MW-2		Dissolved Metals, Ca, Fe
X5636 B X5636 C X5636 E	/D	MW-2 MW-2 MW-2		Mg, K, Na Pesticides TOC, TOX Cl, TDS, Spec. Cond. Alka, SO42, NO3



4036 Youngfield Wheat Ridge, Colorado 80033 (303) 425-6021

Client <u>Indust</u>	rial Compliance, Ir	nc.
Client Project	# <u>1815</u> I	Lab Project #5091
Date of Report	September 29,	1988
Evergreen smpl	# X5635	x5636
Client sample #		<u>MW-2</u>
Alk 200.2	mg CaCO ₃ to pH 4.5	173.9 mg CaCO ₃ to pH 4.5
c1-	336 ppm	78 ppm
TDS	4675 mg/L	1515 mg/L
Nitrate	55.6 ppm	79 ppm
Sp. Cond	3900 micromos	1730 micromos
Sulfate	2463 ppm	716 ppm
TOC	41 mg/L	37 mg/L
TOX	157 ug/L	94 ug/L
Pesticides	none detected	none detected

Note: The TDS and Sp. Cond. values have been transposed for MW-1 and MW-2.



4036 Youngfield Wheat Ridge, Colorado 80033 (303) 425-6021

INORGANIC ANALYSIS DATA SHEET

Client <u>Industrial Compliance</u>	Project No. 5091
Client Sample No. MW-2	Date of Analysis 9/28-29/88
Lab Sample No. X5636A	
Elements Ident	tified and Measured*
Matrix: Water X Soil _	Other
ug/l or mg/Kg dry	y weight (Circle one)
1. Aluminum	13. Magnesium 194,000
2. Antimony	14. Manganese
3. Arsenic	15. Mercury
4. Barium	16. Nickel
5. Beryllium	17. Potassium 22,500
6. Cadmium_	18. Selenium
7. Calcium 468,000	19. Silver
8. Chromium_	20. Sodium 443,000
9. Cobalt	21. Thallium
10. Copper	22. Tin
11. Iron	23. Vanadium
12. Lead	24. Zinc
Cyanide	Percent Solids (%)
*All concentrations corrected f	for reagent blanks
Comments:	
comments:	/1
Inorganic Su	pervisor
Quality Assura	ance Officer

Form: EAIADS



4036 Youngfield Wheat Ridge, Colorado 80033 (303) 425-6021

INORGANIC ANALYSIS DATA SHEET

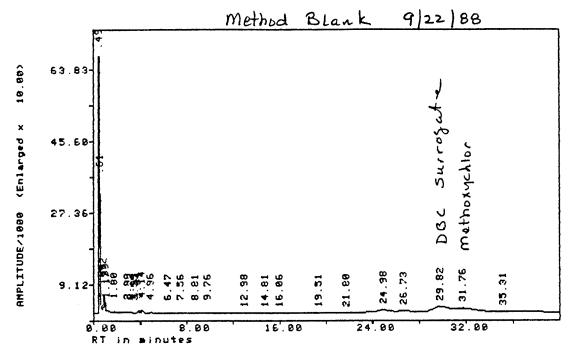
Client <u>Industrial Compliance</u>	e Project No. 5091
Client Sample No. MW-1	Date of Analysis 9/28-29/88
Lab Sample No. X5635A	•
Elements Iden	ntified and Measured*
Matrix: Water X Soil	Sludge Other
(ug/) or mg/Kg di	ry weight (Circle one)
1. Aluminum	13. Magnesium35,700
2. Antimony	14. Manganese
3. Arsenic	15. Mercury
4. Barium	16. Nickel
5. Beryllium	17. Potassium 43,100
6. Cadmium	18. Selenium
7. Calcium83,000	19. Silver
8. Chromium	20. Sodium274,000
9. Cobalt	21. Thallium
10. Copper	22. Tin
11. Iron	23. Vanadium
	24. Zinc
Cyanide	Percent Solids (%)
*All concentrations corrected	for reagent blanks
Comments:	
Comments:	
Inorganic Su	apervisorWKH
Quality Assura	ance Officer

Form: EAIADS

1D PESTICIDE ORGANICS ANALYSIS DATA SHEET

EPA SAMPLE NO.

Lab Name: EUERGREEN ANALUTICAL Contract:	1	METHOD BLANK WB9-ZZ-88
Lab Code: Case No.: SAS No.:	SDG 1	No.:
Matrix: (soil/water) WATER Lab	Sample ID:	WB9-22-88
Sample wt/vol: 1000 (g/mL) ML Lab	File ID:	ECD1005
Level: (low/med) Low Date	e Received:	Annie - Annie annie annie annie annie annie annie annie annie annie annie annie annie annie annie annie annie a
Moisture: not dec dec Date	Extracted:	9-22-88
Extraction: (SepF/Cont/Sonc) SEPF Date	Analyzed:	9-23-88
GPC Cleanup: (Y/N) N pH: Dilu	ition Factor	:: <u> </u>
	rion units: ng/Kg) <u>µg</u> /	<u>L</u> Q
319-84-6alpha-BHC	0.55	-
319-85-7beta-BHC		
319-86-8delta-BHC	0.05	
58-89-9gamma-BHC (Lindane)	0.05	•
76-44-8Heptachlor	0.05	· · · · · · · · · · · · · · · · · · ·
309-00-2Aldrin	0.05	:
1024-57-3Heptachlor epoxide	0.05	
959-98-8Endosulfan I	0.05	
60-57-1Dieldrin	0.05	
72-55-94,4'-DDE	0,10	 ; ;
72-20-8Endrin	0.10	
33213-65-9Endosulfan II	0.10	·
72-54-84,4'-DDD	0.10	· · · · · · · · · · · · · · · · · · ·
1031-07-8Endosulfan sulfate	0.10	
50-29-34,4'-DDT	0.10	
72-43-5Methoxychlor	0.10	
53494-70-5Endrin ketone	1.09	I_ <u>B</u> I
5103-71-9alpha-Chlordane	0.10	<u> </u>
5103-74-2gamma-Chlordane	0.50	!!
8001-35-2Toxaphene	0.50	<u> </u>
12674-11-2Aroclor-1016	_!	<u> </u>
11104-28-2Aroclor-1221	0.5	
11104-26-2Aroclor-1221	0.5	<u> </u>
53469-21-9Aroclor-1242	0.5	!
12672-29-6Aroclor-1248	0,5	<u> </u>
11097-69-1Aroclor-1254	0.5	
11097-69-1Aroclor-1254	1.0	<u> </u>
1 ************************************	1	<u> </u>

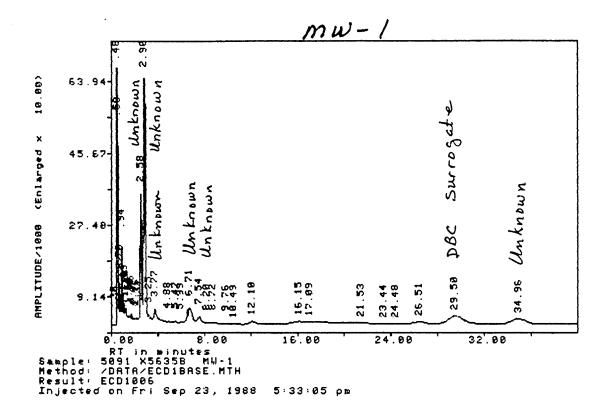


8.88 8.89
RT in minutes
Sample: UB9-22-88
Method: /DATA/ECD1BASE.MTH
Result: ECD1005
Injected on Fri Sep 23, 1988

1D PESTICIDE ORGANICS ANALYSIS DATA SHEET

EPA SAMPLE NO.

1	mw-/
9/i	
SDG No.:	
Sample ID: X	5635 B
File ID: E	CDIOOLe
Received: 9	-20-88
Extracted: 9	-22-88
ON HINTTS:	
/Kg) <u>ル</u> タ/ட	Q
0.05	
0.05	
1_0.05	1_4_1
0.05	1 4 1
0.05	<u> 4 </u>
	<u> </u>
	<u> </u>
·	<u> </u>
	-! <u>-4</u> -!
· · · · · · · · · · · · · · · · · · ·	<u> </u>
	<u> </u>
	<u> </u>
	<u> </u>
	<u>u</u>
	<u> </u>
	i u
	4
	<u>u</u>
	u
0.5	и
0,5	1 4
0.5	1_4_1
0.5	1 <u>u</u>
	1_4
1.0	<u> u </u>
1.0	! <u>-</u> 4_
	SDG No.: Sample ID: X File ID: E Received: Q Extracted: Q Analyzed: Q Analyzed: Q CON UNITS: (Kg)

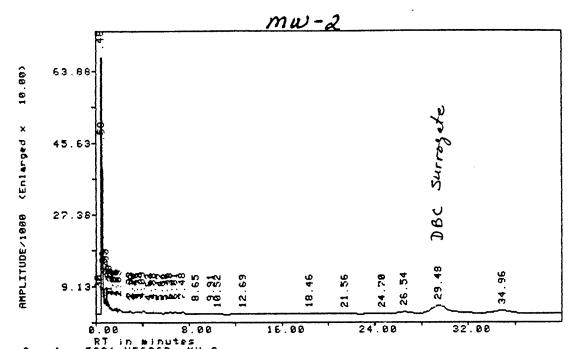


NO PESTICIDES OF PCB's Confirmed.

1D PESTICIDE ORGANICS ANALYSIS DATA SHEET

EPA SAMPLE NO.

•	m	W-2
Lab Name: EUERGREEN ANALYTICAL Contract:	509/	
Lab Code: Case No.: SAS No.:	SDG No.: _	
Matrix: (soil/water) WATER Las	Sample ID: 150	36B
Sample wt/vol: 900 (g/mL) ml Lal	File ID: EC.	D 1007
Level: (low/med) Low Date	te Received: 9-	20-88
% Moisture: not dec dec Dat	e Extracted: 9-2	2-88
Extraction: (SepF/Cont/Sonc) <u>SEPF</u> Dat	e Analyzed: 9-23	3-88
GPC Cleanup: $(Y/N) N$ pH: (L,D) Dil	ution Factor:	/
	TION UNITS:	
CAS NO. COMPOUND (ug/L or	ug/Kg) <u>ug/L</u>	Q
319-84-6alpha-BHC	0.05	u
319-85-7beta-BHC	0.05	u i
319-86-8delta-BHC	0.05	u
58-89-9gamma-BHC (Lindane)	0.05	<u> </u>
76-44-8Heptachlor	0.05	u
309-00-2Aldrin	0.05	u
1024-57-3Heptachlor epoxide	0.05	$\frac{1}{u}$
959-98-8Endosulfan I	0.05	u
60-57-1Dieldrin	0.70	u
72-55-94,4'-DDE	0.10	u
72-20-8Endrin	0.10	u
33213-65-9Endosulfan II	0.10	u
72-54-84,4'-DDD	0.10	u
1031-07-8Endosulfan sulfate	0.10	u
50-29-34,4'-DDT	0.10	U
72-43-5Methoxychlor	0.50	4
53494-70-5Endrin ketone	0,10	u
5103-71-9alpha-Chlordane	0.50	и
5103-74-2gamma-Chlordane	0.50	u
8001-35-2Toxaphene	1.0	<u>u</u>
12674-11-2Aroclor-1016	0.5	u l
11104-28-2Aroclor-1221	0.5	u
11141-16-5Aroclor-1232	0,5	U
53469-21-9Aroclor-1242	0.5	U
12672-29-6Aroclor-1248	0.5	$\frac{u}{u}$
11097-69-1Aroclor-1254	1.0	
11096-82-5Aroclor-1260	1.0	<u>u</u>
	- -	



8.00 16.00

RT in minutes
Sample: 5091 X5536B MW-2
Method: /DRTA/ECD1BASE.MTH
Result: ECD1007
Injected on Fri Sep 23, 1988 6:17:38 pm

A ·

APPENDIX H - PACKER TEST RESULTS



PACKER PERMEABILITY - DATA SHEET

Boring No. <u>58-29</u>		Job No. CSI - 2-1815	
Roring Radius (r)	(11)	Engineer JEC	
Depth of Boring (d) 60'	(f1)	Date9/2/88	
Depth to Static Water (A) 47.3	(ft)	Flow Meter No. 4/007027	
Depth to Center of Packer (B) 38'		Height of Gauge Above Ground Surface (C)	<u>(f1)</u>

	Flo	∠ Data			me to randing @ 574.5
Time 01234567891011813	Gallons	Cubic Feet	Flow Rate 609.7162 60182.8 637.62 637.62 637.62 637.62 637.62 637.62 637.62 637.62 648.2 6560.1 667.9	2 ps, For Rete 692.0 697.7 708.8 719.0 729.2 739.4 744.6	meter randing @ 574.5 Start @ = 1242 a coop. Packer sof at 7/psi Botton packer 40 Contar packer 38' Constant psi Congloted @ 1700
14 15 16 17 18 18 18			67/7 675.5 679.2 683.0 686.8 6903		
				Average Q =	ft ³ /min

 $K = \underbrace{0}_{2r} \ln \underbrace{1}_{r} : L \ge 10r$

K = 0 sinh-1 L: 10r>1≥r

CALCULATIONS

L = d-B

H = B + C + (L/2) + (P)(2.31)-S

where S = B + L/2 - A

Note: If no free water is encountered, S=0

BORING ER-29

L= d-B = 60'-38'= 22'

S= 8+4/2-A = 38'+11'-473' = 1.7'

C=0

for P=10: H= 38'+0'+11' + 10(2.51)-1.7' = 70.4'

Q: Ofor 0-5 min Flow rate = 631.8-609.0 /5 min = 22.89 min = 4.56get/min x 1ft3/450getsec = 2.2710-3

Sec

@ for 5-10min Flow rute = 652.5-631.8/5min = 4.17gal/min x 2.2x10-3 = 9.1x10-3ft/sec

3 for 10-15 min Flow rate = 671.7-6525/5min = 3.84gal/min x 2.2×10-3 = 8.4×10-3ft/sec

1 for 15-20min Flowrate = 690.3-671.7/smin = 3.72gel/min x 2.2x10-3 = 8.2x 10-3 fr3/sec

Qave = 2 a: /4 = 8.9 × 10-3 fr/sec

 $\Gamma = .17'$ L= Zz' SO $E = 2\pi LH \ln^{1}/r = \frac{22}{17} \ln^{1}/r = 4.9$ $E = \frac{8.9 \times 10^{-3} G^{3}/s}{2\pi (22)(704)} (4.9) = 4.5 \times 10^{-6} Gr/s \times 30.48 cm/r = 1.4 \times 10^{-4} cm/s$

L= 22'S= 1.7'c= 0H= 38' + 0' + 11' + 22(231) - 1.7' = 98.12'

Q: 1) 0-smin = 719-692/smin = 5.4 gol/min x 2.2 x10-3= .01ft 3/sec

2 5.10min= 744.6-719/5min= 5.12gel/min x 2.2x103= 101ft/s

 $Q_{ave} = .01ft^{3}/s$ $k = \frac{9}{2\pi L_{H}} \ln \frac{L}{r} = \frac{.01ft^{3}/s}{2\pi (22')(98,12')} \left(\frac{4.9}{9}\right) = 5.6 \times 10^{-6} \frac{\pi}{s} \times 3048 \, \text{cm/pc} = \frac{L1 \times 10^{-6} \text{cm}}{s}$

Kave = 125 x10-4 cm/c

11/2.

PACKER PERMEABILITY - DATA SHEET

Boring No. SB-30

Boring Radius (r) 2' (ft) Engineer JEC

Depth of Boring (d) 55 (ft) Date B/24/88

Depth to Static Water (A) 47.7 (ft) Flow Meter No. 4 1007029

Depth to Center of Packer (B) 28 (ft) Height of Gauge above

Gauge Pressure (P) 10 (psi) Ground Surface (C) 0 Atsurkee(ft)

	Flow Data		Longth of P 14'1" 10'1"	Notes
	Molarenday Cubic		Longth of P	·/LP
Time (Min.)	Gallons Feet	Flow Rate	10,1"	•
. 0	24 3.0		30'2"	•
2	25 5,8		35 &	
2 3 4	25 9.0 26 36			0 111/2 - 11/11
5	26 83		facker - Sing	Length of 33" placed u/ 50 ps;
	a7 2.8 a7 72		packer	placed u/ 50 ps;
g .	28 15		,	
T .	a 8 58			
1 /	29 0,2			•
12	29 8,3			
1 /-	30 2.3 30 6.2			
15	3/00	·		
16	31 · 39 31 · 77			
18	32 /4 32 52			ft ³ /min
20	32 BB		Average Q =	*t-/min
I .		CVICHI	オキカいた	

 $K = \frac{0}{2\pi LH} \sinh^{-1} \frac{L}{2r} : 1$

CALCULATIONS

L = d-B

H = B + C + (L/2) + (P)(2.31)-S

where S = B + L/2 - A

Note: If no free water is encountered, S = 0

PACKER PERMEABILITY - DATA SHEET

Boring No. SB-30

Boring Radius (r) 2" (f1) Engineer TEC

Depth of Boring (d) 35 (f1) Date 8/24/88

Depth to Static Water (A) 47.7 (f1) Flow Meter No. 41007029

Depth to Center of Packer (B) 28 (f1) Height of Gauge above

Gauge Pressure (P) 20 (psi) Ground Surface (C) 0 arsunface (f1)

	Flo	→ Data			Notes	
Time 0/2345478901123145	Gallons	Cubic Feet O	20 psi Flow Rate 33 60 34 43 35 9,2 36 7.1 38 5.7 39 0.6 39 5.5 40 0.8 40 5.7 40 9.3 41 3.2	30/31 42 5.0 43 0.1 43 5.5 44 6.6 45 9.6 45 9.6 46 7.2 47 7.0		10000 ps. 222620

Average Q = ft³/min

 $K = \frac{0}{2\pi 1 H} \ln \frac{L}{r} : L \ge 10r$

 $K = \underbrace{0}_{2rLH} \sinh^{-1} \underbrace{1}_{2r} : 10r > 1 \ge r$

CALCULATIONS L = d-B

H = B + C + (L/2) + (P)(2.31)-S

where S = B + L/2 - A

Note: If no free water is encountered, S = 0 BORING SB-30

BT = 10 d-8: L = 55-28 = 27' C = 0 $S = 0 + \frac{1}{2}A = 28 + 13.5' - 47.7 = -6.2'$ H = 28 + 0 + 15.5' + 10(2.31) + 6.2 = 70.8'

Q: () for 0-5 min = 26.83-24.3/5 min = .5/94/min x 2.2xxx = 1./x10-3 fr 3/5

(3) 5-10 min = 29.02-26.83/5min + .44gal/min x 2.2x0-3 = 9.6 x/0-4 ft3/5

(3) 0-15min = 31-29.02/5min = .40gal/minx 2.2×10-3= 8.7×10-4 f+3/5

(4) 15-20min = 32.38-31/5min . 38991/min x 2.2×10-3= 8.3 ×10-4 fi 1/s

Qave = 9.4x10-4 f+3/s

L=27' > 100 " K= 9/2 TLH In 1/2 In 1/2 In 1/2 = 51

K= 9.4×0-4 fr 3/2 /211(2+1)(70.81) (5.1) = 4×0-7 0/3 × 30.48 cm/2 = 1.2 ×10-5 cm/3

H= 28+0+13.5+20(2.31)+6.2 = 93.9'

Q: (1) 0-5min: 37.41-23.60/5min = .76 gel/min x 7. 2 × 10-3 = 1,7 × 10-3 fr3/s

(2) 5-10min = 40.08-37.41/smin + .53 gal/min x 2.2x10-3 = 1.2x10-3ft3/s

(3) 10-13-min = 41.32-40.08/3min = .4/gal/min x z.Zxxx= = 9.1 xxx=4 fr 3/s

 $Q_{\Delta Ve} = 1.3 \times 10^{-2} \text{ ft}^{3}/\text{s}$ $k = Q_{\Delta \pi LH}(5.1) = 1.3 \times 10^{-2} \text{ ft}^{3}/\text{s} / 2\pi(27)(939') = 4.2 \times 10^{-7} \text{ Pf/s} \times 30.48 \text{ cm/s} = 1.3 \times 10^{-5} \text{ cm/s}$

AI = 30 L= 27'

5=-6.2'

/+= 28+0+135+30(2.31)+6.2 = 117'

Q= 44.66-42.5/4min = .54gal/min x 2.2×0-3 = 1.2×10-3 ft3/s

K= Q/3ELH (51) = 1.2×10-3 ft3/5/2E(27)(117') (51) = 3.1×10-2 ft/5 × 50.48 cm/ft = 9.4×1000

kave = & ki/3 = 1.1 ×10-5cm/s

PACKER PERMEABILITY - DATA SHEET

Boring Radius (r) (11) Engineer Depth of Boring (d) (fi) Date. Depth to Static Water (A) 49.2 (ft) Flow Meter No. 4100 7029
Depth to Center of Packer (B) 57.5 (ft) Height of Gauge above (psi) Ground Surface (C) At surface o' Gauge Pressure (P) 12 mm + 23

	Fìo	w Data			Started to get again. 2 8:50 pm
Mm Time 01 2 3 4 5 6 7 8 9 10 11 12 13 14 15	Gallons	Cubic Feet	553.7 553.7	23 psi Elw Role 554,25 554.95 554.9 554.9 555.0 555.7 555.4 555.7 555.6 555.7	It be them of packer Q & 59.3, cander & packer @ 57.5' Packer tool @ 75.5; Initial maker reading 550, 2 Start time @ 2 9.25mm Stapped @ 2 9.50mm

ft3/min

In <u>L</u> : L≥10r

(**

K = 0 $sinh^{-1}$ L: 10r>L≥r CALCULATIONS

L = d-B

Average Q =

H = B + C + (L/2) + (P)(2.31)-S

where S = B + L/2 - A

Note: If no free water is encountered, S = 0

BORING 58-31

BT = 12 d = 75.5' B = 57.5' C = 0 A = 49.2' L = d - B = 18.0 S = 57.5' + 9.0 - 49.2' = 17.3' H = 57.5' + 0.1 + 12(2.31) - 17.3 = 76.9'

Q: 7 0-7min = 553.8-553.7 = 6 .019 Nmin x z. Zxxx=3 = 5.1x10-5 fr3/s

 $k = \frac{2\pi L + \ln \frac{L}{r}}{2\pi (18')(769')} = \frac{2\pi L + \ln \frac{L}{r}}{10^{-8} \text{ ft/sec} \times 30.48 \text{ cm/ft}} = \frac{5.2 \times 10^{-7} \text{ cm/s}}{10^{-8} \text{ ft/sec}}$

PSI =23

H= 57.5+0 r9 +23(2.31)-17.3 = 102.5

Q. Do-smin = 554.575-554.25/3 = .11901/min x 222 x 10-3 = 2.4 x 10-4 f+3/3

(2) 3-6min = 554.9-554.575/3 = . //ga//min = 2.4 x0-4 fr3/s

(3) 6-9min = 555.2-554.9/3 = . 10941/min x 2 2xp-3 = 2.2x10-4 ft 3/s

\$ 9-12min 5555-555-2/3 = 1941/min = 2.2x10-4ft3/s

(5) 12-15min 555.8-555.5/3 = . 1941/min = 2.2×10-4 ft/s

Que = \(\frac{\xi}{i=1}\) \(\Qi\)_5 = 2.3 \(\chi/0^{-4}\) fr \(\frac{1}{3}\)_5

K = 8/2RLH \$ (4.7) = 2.3 × 10-4 ft 3/5 /2R (18)(102.3) (4.7) = 9.3 × 10-8 ft/s × 30.48 cm/ft = 2.8 × 10-6 m/s

Kare = & ki / = (1.7 ×10-6 cm/s

PACKER PERMEABILITY - DATA SHEET

Job No. CSI Boring No._ Boring Radius (r)___ (ft) Engineer_ (ft) Date_ Depth of Boring (d) (ft) Flow Meter No. 4100 7029 (ft) Height of Gauge above Depth to Static Water (A) Depth to Center of Packer (E) 35 Gauge Pressure (P) 12 + 25+40(psi) Ground Surface (C)

				Gange	م <i>چ</i> ے جی ۔	
	Flo	₩ Data				Notes sofup @ = 1100 Start up @ = 1128
Time 01 23 45 47 89 10 11 13	Gallons	Cubic Feet	12 Flow Rate 569.9 570.1 570.1 570.1 570.1 570.1	35 573.0 573.8 574.2 574.45 574.55 574.55 574.60 574.65 574.67 574.70 574.70	\$75.60 575.65 575.70 575.75 575.80 575.85 575.85 575.95 575.95 576.00	Start up @ 2 1128 Starting rules 10 starting 555.4 Bottom pocker @ 37 Canta @ 35
14 15 16 17 18 19 19	ı			574.773 574.80 574.835 574.85 574.86 574.875 574.875	-	
				Average Q		ft ³ /min

1n <u>L</u> : L≥10r

CALCULATIONS

L = d-B

H = B + C + (L/2) + (P)(2.31)-S

where S = B + L/2 - A

Note: If no free water is encountered,

S = 0

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BORING SB. 34
```

151=12

1. 75' 8= 35' C=1,6' A= 67.4'

L= d-8= 40'

S= 35+20'-67.4= -12.4

H= 35'+1.6'+20'+ 12(2.31)+12.4 = 96.7'

Q= 570.1-569.1/7min = .03gal/min x 7.2x10-3 = 6.3x10-5fr3/s L= 20' + = 10(.17') K= 6.3×10-5 ft /211(26)(96.7') X /n /.17 = 2.5×10-8 ft/3 x 30.48 cm/4 = 7.6×10 cm

PSI = 25

H= 35'+16'+20'+ 25(2.31) + 12.4 = 126.8'

Q: 6 = 574,45 - 573.0/5min = . 29991/min

92 = 574,65-574,45/5min = .04gal/min

Q3 = 574.8-574.65/5min = 1039al/min Q3 = 574.9-574.8/5min = 1039al/min

Pave= & Qi = .1g=1/min x 2.2 x 10-3 = 2.1 x 10-4 ft 3/5

K = 2.1 × 10-4 fr 3/s / 2π(26) × 4.8 = 6.3 × 10-8 ft/sec × 30.48 cm/ft = 1.9 × 10-6 cm/s

PSI = 40

H= 35+ 1.6'+20+ 46(2.31) +12.4 = 161.4'

Q: Q = 575.8-575.6/y = 10594/min = x

Qz = 5760-575.8/4 = .05 qcl/min

Pave = . 05 gal/min 2.2 × 10-3 = 1.1 × 10-4 ft 3/5

K= 1,1×10-4 ft3/s/ 2π(20)(1,61,4') × 4,8= 2.6×10-8 ft/s × 30.48cm/et = 7.9×10-2cm/s

kave = E ki / = 1,2 ×10-6 cm/s

PACKER PERMEABILITY - DATA SHEET

(ft) Engineer Boring Radius (r) (f1) Date_ Depth of Boring (d)_ (f1) Flow Meter No.
(f1) Height of Gauge above Depth to Static Water (A) 54.8 Depth to Center of Packer (B) 38± (psi) Ground Surface (C) (f1) Gauge Pressure (P) 20

Sauge F	ressure Iri_			Brown Surface (C)	
	Fic	<u>⊸ Data</u>		Not	<u>es</u>
Time 012345678901123145	Gallons	<u>Feet</u>	Flow Rate 55 50 50 50 50 50 50 50 50 50 50 50 50	7.5 39'2%	Han of packer
				Average Q =	ft ³ /min

 $K = \underbrace{0}_{2r} \ln \underbrace{1}_{r} : L \ge 10r$

CALCULATIONS

L = d-B

H = B + C + (L/2) + (P)(2.31)-5

where S = B + L/2 - A

Note: If no free water is encountered,

BORING SE-35

MI = 20

 $d = 55' \quad B = 38.5' \quad C = 0' \quad A = 54.8' \quad r = .17'$ L = d - B = 16.5' S = 38.5' + 8.25' - 54.8' = -8.05' H = 38.5' + 0' + 8.25' + 20(2.31) + 8.05' = 101.0' $Q: \quad Q_1 = \frac{502.4 - 507.2}{5} = \frac{.32921/min}{6}$ $Q_2 = \frac{.3921/min}{6} \times \frac{.32210^{-3}}{.5720^{-4}} = \frac{.5720^{-4}}{3}$

L= 16.5' = (17)(10)

k= 5.7×10-4ft³/₂ / x /n 165/.17 = 2

7.6×10 cm

PACKER PERMEABILITY - DATA SHEET

Boring No. SB-35

Boring Radius (r) 2 (ft) Engineer JEE

Depth of Boring (d) 55 (ft) Date R/20/08

Depth to Static Nater (A) 54, 8 (ft) Flow Meter No.

Depth to Center of Packer (B) 38,5 (ft) Height of Gauge above

Gauge Pressure (P) 2 40 (psi) Ground Surface (C) (ft)

	Flo	w Data		. <u>Notes</u>
Time 01 2345 67890	Gallons	Cubic Feet	Flow Rate 512.0 518.7 514.8 515.8 516.75 517.6 518.3.5 519.0 519.8 521.2	a packer stocked 6 stops

Average $Q = ft^3/min$

 $K = \frac{Q}{2rLH} \ln \frac{L}{r} : L \ge 10r$

 $K = \frac{0}{2rLH} \sinh^{-1} \frac{L}{2r} : 10r > L \ge r$

CALCULATIONS I = d-B

H = B + C + (L/2) + (P)(2.31)-S

where S = B + L/2 - A

Note: If no free water is encountered, S = 0

BORING SE-35

PSI = 40 d= 55' D= 38.5' C= 0 A = 54.8' L= d-0 = 55'- 38.5' = 16.5' 5 = 32.5' + 8.25 - 54.8' = -8.05' H= 35.5' + 0' + 8.25' + 40(2.31) + 8.05 = 147.2'

Q' = 521.2-516.75/ = 1.2901/m

Qz = 521.2-516.75/ = .74901/min

Que = 1 gal/min x 2.2 x10-3 = 2.1 x10-3 fr3/3

L = 10(.17)

k = 2.1x10-3 ft 3/s /21 (16.5')(1472') x /n 165' - 4.6

1.9x10cm/s = (1.9x10cm/s) = (1.9x10cm/s)



PACKER PERMEABILITY - DATA SHEET

Boring Radius (r) (ft) Engineer_ Depth of Boring (d)_ (f1) Date_ Depth to Static Water (A) O (div) (ft) Flow Meter No. 41007027 (11) Height of Gauge bove Depth to Center of Packer (B) 35' (psi) Ground Surface (C) At Surface R 4 20 (ft) Gauge Pressure (P)_

	Flo	₩ Data			Notes note rading
Time 012345678901123145	Gzllons	Cubic Feet	Flow Rate 545.9 545.725 545.725 545.725 545.725	547.7 547.9 548.3	pro La Rest 520.05 70 psi on pac Ker Statel 0: 1519 Packer to Hom set at 37'
				Average 0 =	ft3/min

Average Q =

ft2/min

In L : L≥10r

CALCULATIONS

H = B + C + (L/2) + (P)(2.31)-S

where S = B + L/2 - A

Note: If no free water is encountered,

S = 0

BORING SB-37

PSI=12 J=60' B=35'A=40NA r=.17 c=0 L=J-B=25' S=0 H=35'+0't pas' + 12(2.31)-0 = 75.2'

Q: = .725-,70/5 = 5×10-3 gal/min ×2.2×10-3 = 1.1×10-5 A3/5

 $k = \frac{1.1 \times 10^{-5} \, \text{fi}^3}{2\pi (25)(75.2)} \times \frac{Ln^{\frac{26}{17} - 5}}{2\pi (25)(75.2)} = 4.7 \times 10^{-7} \, \text{ff/s} \times \frac{30.48 \, \text{cm/ft}}{2.14 \times 10^{-7} \, \text{cm/s}}$

PST = 20 H= 35'+0'+12.5'+ 20(2.31)-0=93.7'

Q: 550.1-5.47.1/10 =. 3gel/min x 2.2 ×10-3= 6.6 ×10-4 ft3/s

K= 6.6×10-4 fr3/s/ /21(25')(937') X5= 2.2×10-7 fr/s ×30.48 cm/g= 6.8×10-6 cm/s

kave = 3.5×10-6cm/s

APPENDIX I - RISING HEAD TEST RESULTS

Ring Head Test MW-1 CSI 2-1815

Using Formula presented for condition (c) in Cedergren (1967).

$$K = \frac{R^2}{2L(t_2-t_1)} ln\left(\frac{L}{R}\right) ln\left(\frac{h_1}{h_2}\right)$$

$$K = \frac{0.17^2}{2(5)(6-1.5)} ln \left(\frac{.92}{.80}\right)$$

Rising Head Test MW-1 CSI 2-1815

Static W.L. = 50.6 ft.

Time (min)	SH (W.L Static 2.)	4/40
0.1.1.2.2.3.3.4.5.6.7.8.9.1.2.1.3.5.6.1.8.0 0.1.1.2.2.3.3.4.5.6.7.8.9.1.2.1.3.5.6.1.7.8.0	52.58 - 50.6 = 1.98 52.56 - " = 1.96 52.42 - " = 1.78 52.38 - " = 1.78 52.38 - " = 1.75 52.33 - " = 1.69 52.29 - " = 1.65 52.29 - " = 1.65 52.19 - " = 1.59 52.19 - " = 1.59 52.19 - " = 1.57 52.17 - " = 1.46 52.00 - " = 1.46 51.98 - " = 1.30 51.98 - " = 1.23	10972008753009854107652 0999988888887777766652

Rising Head Test MW-1 O. 1_. Semi-Logarithmic 2 Cycles x 19 to the meh

Rising Head Test MW-Z CSI
2-1815

Using Formula presented for condition (C)
in Cedergren (1967). $K = \frac{R^2}{2L(t_2-t_1)} ln\left(\frac{L}{R}\right) ln\left(\frac{h_L}{h_Z}\right)$

 $K = \frac{0.17^{2}}{2(5)(12-1.5)} ln(\frac{5}{0.17}) ln(\frac{97}{67})$

 $K = 3.4 \times 10^{-4} \text{ ft/min}$ $K = 1.8 \times 10^{-4} \text{ cm/s}$

.

Rising Head Test CSI 2-1815

MW-Z Static W.L. = 20.0

	STANTE N	1.2 20.0
Time (min.)	SH (W.L Static L.)	4/40
0.75 1.5 2.25 3.5 4.17 56	21.95 - 20.0 = 1.93 21.90 - = 1.9 21.90 - = 1.9 21.85 - = 1.83 21.8 - = 1.8 21.8 - = 1.8 21.8 - = 1.8 21.65 - = 1.63	1.00 .97 .97 .95 .92 .92
7891011231447 1181417 180	21.5 - 21.45 - 21.45 - 21.35 - 21.35 - 21.2 - 21.2 - 21.2 - 21.1 - 21.1 - 21.1	77429777775655

Rising Head Test MW-Z 2-1815

 $t_{1} = \frac{1}{5}$ $t_{1} = \frac{1}{5}$ $t_{1} = \frac{1}{5}$ $t_{1} = \frac{1}{5}$ $t_{1} = \frac{1}{5}$ $t_{2} = \frac{1}{5}$ $t_{2} = \frac{1}{5}$ $t_{3} = \frac{1}{5}$ $t_{4} = \frac{1}{5}$ $t_{5} = \frac{1}{5}$ $t_{5} = \frac{1}{5}$ $t_{5} = \frac{1}{5}$ $t_{5} = \frac{1}{5}$ $t_{5} = \frac{1}{5}$ $t_{5} = \frac{1}{5}$ $t_{5} = \frac{1}{5}$

Semi-Logarithmic

0. ___

APPENDIX J - ESTIMATED TRAVEL TIME OF WATER

TRAVEL TIME CALCULATIONS

Calculations of the required travel time consists of the time required for water to flow from the base of the clay liner to the regional Denver Aguifer.

Basic assumptions:

- A. The Denver Formation in the vicinity of the site consists of approximately 90% claystone and 10% silty sand.
- B. Using an average elevation of the bottom of the clay liner of 5285 feet and an average elevation of the top of the Denver Aquifer of 5175 feet, the average vertical distance from the bottom of the clay liner and the top of the Denver Aquifer is 110 feet.
- C. There is approximately 11 feet of saturated silty sand and 99 feet of unsaturated claystone between the bottom of the liner and the top of the Denver aquifer.

Travel time calculation from the base of the clay liner to the top of the Denver aquifer (claystone and silty sand material):

1). Unsaturated flow conditions (McWhorter and Nelson, 1979)

$$T = z_0 (0) / [q_0^{1-1/n} (k_s^{1/n})]$$

Where:

T = travel time through unsaturated material

 z_0 = flow path length in unsaturated material = vertical distance between bottom of the clay liner and top of the Denver Aquifer (99 feet or 3018 centimeters).

@ = effective porosity = 0.1 (claystone) and 0.2 (silty sand).

 q_u = volume of infiltrated liquid water ; assuming a unit hydraulic gradient and an area of one square unit, q_u = unsaturated vertical hydraulic conductivity (k_u) . From field permeability tests, k_u is estimated to be 1.0 x 10 $^{-8}$ cm/s.

 k_{s} = saturated vertical hydraulic conductivity = 1.0 \times 10 $^{\text{-7}}$ cm/s.

n = Brooks Corey parameter = 4.0

$$T = (3018)(0.10)/[1 \times 10^{-8})^{3/4}(1 \times 10^{-7})^{1/4}] = 1.7 \times 10^{10}$$
 sec.

2). Saturated Flow Conditions: (McWhorter and Nelson, 1979).

$$T = z_0(\theta)/k_s$$

Where:

T = travel time through saturated material

 $\mathbf{z}_0 = \text{flow path length through saturated material} = 11$ feet or 335 centimeters

0 = 0.2

 $k_s = 1 \times 10^{-6} \text{ cm/s}$

 $T = (335)(0.2)/(1 \times 10^{-6}) = 8.4 \times 10^{7} \text{ sec.}$

Total travel time through the upper Denver Formation material: $1.7 \times 10^{10} + 8.4 \times 10^7 = 1.71 \times 10^{10} = 542 \text{ years.}$ APPENDIX K - ANALYTICAL LABORATORY PAPERWORK AND FORMS

TABLE 3

Equipment

CSI has the following equipment:

- 1. Pensky-Martens Closed Cup Flash Point Tester (Prescision Scientific No. 74537) with Electric Heater and Slo-Speed Stirrer.
- 2. Corning pH Meter 5 (no. 475001) with Temperature Comprensator and Calomel Combination Electrode Additional Electrodes; X-EL Flat Surface Combination Electrode, Ammonia Electrode.
- 3. Damon IEC HN-SII Centrifuge
- 4. Ohaus Moisture Determination Balance
- 5. Hach DR-3000 Spectrophotometer
- 6. Neotronics DigiFlam 850 Flammable Gas Monitor
- 7. Matheson-Kitagawa Tox Gas Analyzer
- 8. VRW Microdispenser Micro Pipet
- 9. VRW Hydrometer Set (No. 34630)
- 10. Gilmont Falling Ball Type Viscosimeter
- 11. Hach Digesdahl Digestion Apparatus
- 12. Alert Monitor 4 Radiation Meter
- 13. Hot Plates
- 14. Hood
- 15. Desicator
- 16. Balance
- 17. Burettes
- 18. Refrigerator
- 19. Glassware
- 20. Plasticware
- 21. Reagent Chemcials
- 22. Computer

INDUSTRIAL WASTE DISPOSAL MANIFEST

012218

	PART TO BE	I: COMPLETED BY SHIPP	PER/GENERATOF	3
COMPANY NAME				
BUSINESS ADDRE	ss			
ADDRESS OF SHIE	PMENT ORIGIN			
AUTHORIZED CON	NTACT		EMERO	GENCY PHONE
RECEIVER'S NAM	E			
BUSINESS ADDRE	SS			
SITE ADDRESS			PHONE	<u>E:</u>
QUANTITY	UNITS	WASTE DESCRIPTI	ON	CSI WASTE CODE NUMBER
The materials desc named below. I cer the best of my kno	rtify that the fore owledge.	e consigned to the Carrier going is true and correct to	SIGNATURE OF AUTHORIZED AGENT X TYPE OR PRINT ABOVE NAME:	
	PAR'		RIER/DRIVER	
CARRIER NAME				
BUSINESS ADDRI	ESS			
PHONE NO.		A-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1		
I certify that the meceived by me fo	naterials in the query shipment to the	uantity described above are e above destination.	SIGNATURE OF AUTHORIZED AGENT X	
		DATE:	TYPE OR PRINT ABOVE NAME:	
		T III E COMPLETED BY RECE	:IVER	
RECEIVER'S NAM	1E			
PHONE NO.				
SITE ADDRESS				
RECEIVER'S COM	MENTS			
week.				
I certify that the materials in the quantities described in Part I are received by me.		uantities described in Part I	SIGNATURE OF AUTHORIZED AGENT X	
		DATE	TYPE OR PRINT ABOVE NAME	
	WH	ITE - ORIGINAL YELLOW - TRANSPOR		ACILITY GOLD - GENERATOR

COMPUTER ENTRY DATA FORMS

Account Number: Lab Control No: Series:

Waste Code Number: EPA Generator ID: Date:

Company:
Business Address:
Contact:

Waste Description: Waste Process:

Anticipated Volume:
Anticipated Units:
Per:

Jutside Lab Report: Attached MSDS: Lab/MSDS Date:

> Sample Included: Taken By: , Sample Date:

> > Signed: Date Signed:

Account Number: Lab Control No: Series: 1

> LSR Date: Salesman:

> > Company: Contact:

Action Requested: CSI DISPOSAL Comments:

>> CHEMICAL ANALYSIS

Material Submitted:
Sample Date:
Frocess Involved:
Handling Precautions:
Analysis Requested:

Take to Outside Lab For: Outside Lab Used: Lab Signature:

>> LABORATORY REPLY

RCRA Characteristics: NONE
Possible RCRA Contaminants: NONE
Additional Testing Needed:
Additional Materials Needed:
Neutralization Requirements:
Waste Catagory:
Type of Treatment: SOLIDIFICATION
Reagent Type: KILN DUST
Waste/Water Ratio:
Waste/Reagent Ratio:
Volume Increase:
Direct Waste To:
Comments:

Waste Code Number: Date Signed: Signed: Account Number: Lab Control No: Series: 1

Company:

Location of Sampling: Waste Process:

> Date Sampled: Time Sampled:

Waste Description: Sample Equipment:

Composite Samples: Number of Sub-Samples: Volume of Sub-Samples:

Field Info/Comments:

Sampler: Sampler Title: Sampler Company:

Form: 2 Type: 3

Account Number: Lab Control No: Series: 1

Date Sampled: Time Sampled:

Waste Description: Waste Process:

Name of Composite Sample:

Samples Making Composite:

<u>Lab Control No</u>

Sample No.

Volume to Composite

1)
2)
3)
4)
5)
6)
7)
8)

Reason for Composite:

Composite will be used for:

Composited By: Date: Time:

LAB WORKSHEET

Account No: Lab Control No: Series: 1

Physical State:
Description:
Density:
Solubility:

Ignitability: IN DEGREES FAHRENHEIT

Closed Cup Flash Point: >150

Corrosivity Initial pH:

Reactivity: NONE

Sulfides Quantitative: NONE Cyanides Quantitative: NONE

Ammonia Quantitative: NONE

Chlorides Spot Test:

Radioactivity: NONE

Suspended Solids: NONE

Moisture: NONE

Viscosity: NONE

Kiln Dust Type:
Waste/Water Ratio:

Ratio:

Volume Increase:

Jalization Numbers:

Comments:

Analyst: Date: Manifest No: Class: A Date:

Company:
Business Address:
Address of Origin:
Contact:
Emergency Phone:

Facility:

Quantity: Units: Vaste Description: Vaste Code Number:

Generator Agent:

Carrier Name: Carrier Address: Carrier Phone: Carrier Agent:

Receiver Name: CONSERVATION SERVICES, INC.

ciever Phone: 303-426-8158

Site Address: 777 W. 62ND AVENUE DENVER, CO 80216

Receiver Comments:

Receiver Agent:

WASTE CHARACTERIZATION FORMS



EPA Generator	1.D.	No.	(If	Any])

CSI Waste Code No. .

I Directions: In order for us to determine whether we can lawfully and safely transport, treat, and dispose of your waste material, we must obtain certain informatiout the chemical and physical properties of the waste and its chemical composition. Please be complete in your answers; if your response is 'none' or 'not available', so indicate. Answers must be printed in ink or typewritten and the completed form must be signed. Please make a copy of this form for your records.

1) Generator Name		Date
2) Generating Facility Complete Add	dress	
Technical	Title	PhonePhonePhone
4) General Waste Description		
5) Process Generating Waste		
Per: [] Day [] Month [] 7) Is this Waste a "Hazardous Waste"	Year or [] One Time [] Other	[] Drums [] Other
g) Density h) Odor: [] Strong [] Mild i) Complete waste composition (A organic	[] °F [] °C (closed cup only) Bilayered [] Multilayered olid [] Liquid [] Semi-Solid [] Other_ C)	
9) Is the information provided in	Section 8 above based upon Laboratory and f most recent analysis	lysis or Material Safety Data Sheets (MSDS) of the waste
	siologic Materials, Pathogens, or Etiological A	gents?lf yes, specify type and
1) Does this waste stream contain an	y Polychlorinated Biphenyls (PCB)?	If yes. specify concentration
2) Sample included []	aken By	Date
3) Required personnel protective equ	sipment and procedures	
willful omissions of composition or pro Generator's Authorized Signatory Signature X Print or Type Confidentiality Agreement: As undersigned agrees to treat suc	consideration for the Generator's release of the al	f my knowledge and ability to determine, that no deliberate or ds have been disclosed. Date Title Dove information and any other supplemental data, the disclose such information to others except as is required
·	es only after first giving notice to the Generator.	
ByName CS! Representative		Title

CONSERVATION 7777 West 62nd Denver, Colorado 80216 SERVICES, INC. (303) 426-8158

CHEMICAL WASTE REPRESENTATIVE SAMPLE DATA FORM

Generator:	Company	's Name		-		
	•	's Address				
Location of	r Sampli	unit, l	Pond, Pit	Tank, Etc	.	
Process Pro	oducing	Waste:				
Date Sample	ed:		Time	Sampled:		AM/PM
Type of Wa	ste:	ıdge, Waster	water, So	lid, Mix,	Etc S	Specify
Volume of S	Sample Co	ollected:				
Type of Sa	mple: Cir	cle				
Coliwasa	Grain	Trier	Scoop	Arger	Pond	Weighted Bottle
Thief	Other	Specify				
Composite S Number o Volume o	f Sample:					
Field Info	rmation:	Comments				
I certify managed. Co		ample is r	epresent	ative of	the was	ste to be
Print Name			Com	pany		
Signature			Tit	le		
Phone Numb	er					



Generator:	
Manifest Number:	
Lab Control Number	

DRUM SAMPLING SHEET

le. Number	Drum	Content		Consistancy D-Difficult	Drum
Number	Number	Amount	Description of Contents	E–Easy	Condition
			·		
			·		
1					
1					
					<u>.</u>
. 3					
<u> </u>					
	 				

GENERATOR TYPE OF WASTE PROCESS PRODUCING WASTE NAME OF COMPOSITE SAMPLE (NEW)	LAB	CONTROL	. NO	•
PROCESS PRODUCING WASTE				
				-
NAME OF COMPOSITE SAMPLE (NEW)				
	•			
NAME OF SAMPLES MAKING COMPOSITE (OLD) LAB CONTROL NO. SAMPLE NO.	•	VOLUME	TO	COMPOSITE
1) 2) 3) 4) 5) 6) 7) 8) 9)				
THE REASON FOR MAKING THE COMPOSITE SAM	1PLE	•		
THE COMPOSITE SAMPLE WILL BE USED FOR.				
ANALYSTTIME				

CONSERVATION SERVICES, INC. LABORATORY WORKSHEET

CHSTOMER:	CONTACT:
WASTE CODE: CO-	01 MANIFEST NO.:
RECIEVED: DATE	O1- TIME VOLUME
PHYSICAL STATEOIL	(top to bottom 1 2 3 4) LIQUID % SOLID % SLUDGE % OTHER
DESCRIPTION C	COLORODORLOOK
DENSITY	() lb/cu.yd () lb/gal () g/cc
MISCIBILITY	Miscible in water? () yes () no
SOLUBILITY Sol	luble in water? () very () slight () none
	%LELatcF
CLOSI	ED CUP FLASH POINT °F
CORROSIVITY	INITIAL pH((
REACTIVITY Any Describe:	reactions with () AIR() WATER() KILN DUST
	ACID ADJUST: 20 ml of a
	BASE ADJUST: 20 ml of a
SULFIDES Spot	test ()positive ()negative QUANTITATIVE
CYANIDES QUAN	TITATIVE
AMMONIA QUAN	TITATIVE
CHLORIDES Spo	t test()very()slight()none
RADIOACTIVITY	Greater() or Less() than background
SUSPENDED SOLI	DS After Centrifuge% of total volume
MOISTURE	% MOISTURE by evaporation
VISCOSITY	cp (centipoises)
ANALYST	DATE

CUSTOMER: CO-01-	DA'	TE:		
WASTE CODE: CO-01-				
SOLIDIFICATION REQUIREME	ENTS			
		OF HACTE	100MI S-	
KILN DUST TYPE				
WASTE/WATER RATIO RATIO FIGURES START: FINISH: TOTAL:	_% WASTE	% W.	ATER	
FINISH:	MLS KD=	G		
TOTAL: _	MLS KD=	G		
$\frac{200 \text{ MLS KD}}{x} = \frac{154.}{x}$	O G KD	x =		
KILN DUST TYPE	WEIGHT	OF WAST	E 100MLS=	
WASTE/WATER RATIO	_% WASTE	% W	ATER	
RATIO FIGURES START:	MLS KD= MLS KD=	G		
WASTE/WATER RATIO RATIO FIGURES START: FINISH: TOTAL:	MLS KD=	G		
$\frac{200 \text{ MLS KD}}{x} = \frac{154}{}$				
RATIO	VOLUME INCRE	CASE		
COMMENTS				,
NEUTRALIZATION REQUIREM				
TITRANT USED:				FINAL PH
INDICATOR TYPE				
m o nu of	pproximately	ga	llons of	
To get a pH of a is needed for each		of w	aste.	
COMMENTS				
ANALYST	DATE			



DATE	CAI	ESMAN		LAB CONT	ROL		LSR SERIES
DATE	JAL	ESIVIAIN		NUMBER	noc		NUMBER
GENERATOR NAME					COMPANY		
GENERATOR					PHONE		
ADDRESS ATTACHED							
FORMS I	□ WCD □ ANALYTICA				DOY D CON	PRIORI	□ OTHER
VOLUME/ FREQUENCY		ACTION REQUES	TED DISPOSA	r _ (COMMENTS)	PRIORI	1 [
OTHER COMMENTS							
GENERAL WA						DATE SAMPL	ED.
PHYSICAL	(COLOR		······································	ODOR	
STATE PROCESS GE	NERATING		I]	
WASTE	NEI/AIII/O						
HANDLING PRECAUTION:	S						
INFORMATION SAMPLE BOT							
ANALYSIS			mm.om	0.01/5::07::5::			
REQUESTED	☐ RCRA CHARACTERISTIC		REASE □ GC/M POLLUTANTS □ GC/M	S PHENOL/CN S VOA	□ PCB □ PHENOL		□ FLASH POINT □ pH
	☐ F001-F005 SOLVENTS	□ TOTAL RCI			D SULFIDES		□ TCLP
	☐ EP TOXICITY PESTICIDI			S PEST/PCB	□ CHLORIDES		
	☐ TOTAL ORGANIC HALID	ES (TOX) 🗆 BTU	□ GC/M	S METAL	□ ORGANIC SC	AN FOR .	
	O OTHER						
OUTSIDE							
LAB USED				LAB SIG	GNATURE		
		ΙΔΕ	BORATORY	REPLY			
WASTE HAS F	ICPA	SPECIFY	JONATONI				
CHARACTERIS	STICS YES NO	RCRA CHARACTERIS	TIC				
POSSIBLE RC LISTED CONT							
ADDITIONAL TESTING NEE	DED						
ADDITIONAL			DATE		DATE		
MATERIALS N	EEDED		DATE		DATE DATE		
MATERIALS N					RECEIVED		
NEUTRALIZAT REQUIREMEN							
WASTE CATEGORY							
TYPE OF	D COLIDIFICATION	□ DIRECT BURIA	N D NEUTDALIZ	ATION THEN S	OL IDIEICATION		SEE COMMENTS
TREATMENT REAGENT	SOLIDIFICATION	D DIRECT BORD	AL D NEOTRALIZ		TY EQUIPMENT		SEE COMMENTS
TYPE RATIO	KILN DUST D FLY AS	H □ OTHER RATIO		REQ!	JIRED IMF		
WASTE/WATE		WASTE/RE	AGENT		EASE		Allega A. C.
DIRECT WAST		□ BASIN TWO □ □	DISPOSAL CELL D	OTHER			
COMMENTS							
				, where which is a summer of the second			
-				···			
<u> </u>							
WASTE CODE	•						

_ SIGNED __

DATE: ___

APPENDIX L - CSI PERSONNEL JOB DESCRIPTIONS

JOB DESCRIPTIONS

FACILITY MANAGER

T. RESPONSIBILITIES

- A. Schedule site operations
 - Supervise and direct equipment operations on the site
 - Supervise tank farm and solidification activities on the site
 - 3. Supervise the scheduling of reagent deliveries for the solidification process
 - 4. The Facility Manager is designated as the emergency response coordinator for the site
- B. Ensure that site operations adhere to all safety rules and regulations
 - 1. Perform daily inspections of the site
 - 2. Assist the technical department or sub-contractors in monitoring activities and operations
 - 3. Supervise sub-contractors with regard to safety procedures
 - 4. Ensure that the solidification operators adhere to all the safety rules and regulations

C. Maintain Site

Supervise road and access maintenance

- 2. Supervise grounds maintenance
- 3. Supervise general site housekeeping (i.e. maintenance, snow removal, etc.)

D. Personnel

- Ensure that all employees meet CSI and
 D.O.T. hiring requirements, where applicable
 - 2. Ensure that all employees are trained in the use of their assigned equipment, and that they understand and are able to perform their assigned tasks efficiently
 - 3. Ensure that all employees are trained in material handling and emergency response procedures
 - 4. Monitor drivers daily logs
 - 5. Keep up with local, state, and federal laws regarding the handling of waste materials

E. Equipment

- Institute and maintain an equipment inspection and prevention maintenance program
- Institute and maintain a truck and tank testing and inspection program
- F. Marketing and Employee Relations
 - Assist CSI Sales personnel with any scheduling or waste handling difficulties that might arise
 - Set up and manage a comprehensive safety program at the site
 - a. Hold regular district safety meetings at least monthly, or as necessary.
 - 1. These meetings should include a

safety or health topic

- 2. Discuss any safety problems the operation is experiencing
- 3. Commend people who are doing a good job as far as safety is concerned
- 4. Discuss improvements or ideas to improve the company's safety program
- b. Attend organized safety or training meetings and/or seminars and prepare a summary of information gathered at such meetings
- c. Help establish and monitor a disciplinary policy for the company

3. Security Program

- a. Prepare a manual for security monitoring at the site. This will include:
 - Setting up schedules for building and machinery safety equipment inspections, including, but not limited, to fire extinguishers, eyewashes, escape routes, etc.
 - 2. Setting up schedules for daily inspections of perimeter fencing, lighting, general housekeeping, inclimate weather preparation, etc. and making sure written daily logs are kept to verify such inspections
 - 3. If guards are employed, he will be

responsible for their training and schedules

- b. Prepare a program for vehicle inspection and verification of condition
 - 1. See that all vehicles engaged in the transportation of waste or reagent products are inspected for:
 - a. Safety equipment
 - b. Lights, brakes, tires, etc.
 - c. Proper paperwork
 - d. That the truck is clean (if leaving site)
 - e. General appearance
 - 2. Supervise site security inspection to reduce or eliminate theft of company property
- G. Hiring and Training Procedures
 - 1. Hiring
 - a. Must approve all hourly personnel, office and/or clerical people hired for the site
 - b. Be responsible for checking references, driving records, and to be sure all employees have complete physicals (a) before they are hired, and (b) at least once per year thereafter
 - c. Be sure all new employees receive, understand, and acknowledge receipt of all orientation literature including, but not limited to,

safety material, training manuals, job descriptions, etc.

2. Training

- a. Be responsible for designing procedures to help train new employees.
- b. Be responsible for instituting and maintaining formal written training procedures.

H. Record Keeping and Follow-up Procedures

1. Record Keeping

- a. Prepare and review any accident reports, and be sure all necessary reports and/or correspondence is completed and submitted in a timely fashion.
- b. Be responsible for completing the necessary OSHA forms and/or reports.
- c. Be responsible for completing workmen's compensation and vehicle accident reports.
- d. Also keep a permanent file of:
 - 1. Employee commendations or reprimands
 - Monthly safety meetings
 - 3. Safety committee meetings
- e. Maintain building and machinery safety equipment inspection file
- f. Maintain security guard logs and inspection file, if necessary
- g. Keep equipment operation & maintenance logs
- h. Keep site inspection records

- i. Reagent deliveries
- j. Verify waste inventories
- k. Record all treatment & solidification operations
- 1. Keep equipment utilization & maintenance records

2. Follow-up Procedures

- a. Travel to jobs-in-progress, as necessary to observe working techniques and safety procedures of employees and supervisors, on a spot check basis
- b. Institute on-site safety and housekeeping inspections. These inspections are to be confirmed in writing and held at least once per week

II. AUTHORITY

- A. Supervise site operators & helpers
- B. Site emergency response coordinator
- C. Off-site emergency response team leader
- D. Has disciplinary control over all hourly employees
- E. May make disciplinary action recommendations for safety violations involving salaried personnel to the Vice President of Operations

III. COMPENSATION

- A. This position will be salaried
- B. Other Compensations
 - 1. Expense account
 - Vehicle: pickup truck or equivalent with insurance,
 maintenance, and fuel will be provided
 - 3. Incentive bonuses may be provided

IV. OBJECTIVES

- A. Insure effective, safe, and economical operations
- B. Set-up and maintain and prescribe safety and training program
- C. Help establish and maintain a budget for the site
- D. Set-up and maintain an effective respiratory and safety equipment training program
- E. Insure that all drivers attend Defensive Driving Courses
- F. Get involved in community activities
- G. Establish a good repor with local and state law enforcement officials
- H. Help maintain a good CSI image in the community and with regulatory officials

V. QUALIFICATIONS

- A. High school diploma (minimum)
- B. Familiar with heavy equipment operation & maintenance
- C. Safety training and emergency response experience
- D. Preventative maintenance of heavy equipment experience
- E. Some business administration training

SERVICE SUPERVISOR.

I. RESPONSIBILITIES

- A. Customer job site project supervisor. He is to coordinate trucks, drivers/operators, laborers, sub-contractors to meet job specifications toward a successful project completion
- B. He is accountable for all CSI and CSI sub-contracted employees on the customer's location
- C. Responsible for enforcement of safety policies and procedures at any and all work sites. He must see that there is no deviation from established procedures or safety rules. He is to see that all violations are brought to the attention of the appropriate persons, and is empowered to make such field changes as are necessary to ensure compliance with all regulations
- D. He is responsible for reporting and recording all events on a job including safety violations, accident reports, daily work performed, and any other reports required by CSI Management
- E. He is responsible for on-site customer contact relevant to the job and for maintaining good customer relations
- F. He is responsible for ensuring that all employees are

trained in the use of their assigned equipment and for monitoring daily job logs

II. AUTHORITY

- A. Reports to Facility Manager
- B. Has disciplinary control over all CSI truck drivers/operators, laborers, and supervisory authority over all CSI sub-contracted employees
- C. Customer job site and facility off-site emergency response coordinator

III. COMPENSATION

- A. Will be a salaried position
- B. Provided with pickup truck, fuel, maintenance, and insurance
- C. Incentive bonus may be provided

CHEMIST

I. RESPONSIBILITIES

- A. Set-up and coordinate health and technical program for the District
 - 1. Health Program
 - a. Prepare and supervise a written respiratory program, according to OSHA standards and guidelines
 - b. Prepare, coordinate, and circulate safety data information sheets on waste streams handled at this site to the appropriate personnel
 - c. Make sure that all appropriate employees are informed of the proper chemical handling procedures for waste streams handled at the site
 - d. Supervise training sessions for employees on respirator, health, and chemical handling procedures
 - e. To make regular, at least once a week, inspections of facilities, including but not limited to first aid stations, safety equipment storage and condition, safety showers and

- eyewashes, employee locker rooms, etc. He also shall maintain a permanent written record of all such inspections
- f. Monitor facility laboratory for proper safety and health techniques
- g. Coordinate with Facility Manager as to the proper safety gear required for work on-site or in customer's plant

2. Technical Program

- a. Coordinate with the Sales Engineers on the following for each waste stream
 - 1. Waste Characterization Data Sheet
 - 2. Sample of the waste to be handled
 - Treatment and disposal recommendation for waste stream
- b. Be sure each waste stream has been approved by Facility Manager for disposal and/or treatment. On any special waste, coordinate with the proper EPA or Health Department officials
- c. Communicate with customer's Technical Staff to help with problems and/or questions regarding treatment or disposal at the site
- d. Give approval for disposal, transfer, or shipping of waste to/from the site after the following criteria are met
 - 1. Proper paperwork accompanies the waste
 - 2. Analysis of waste to assure identity
 - 3. Designation of proper safety gear and

procedures

- 4. Filling out a waste disposal or shipping authorization sheet
- B. Record keeping and follow-up procedures
 - 1. Record Keeping
 - a. Set up monthly reporting to the State Health Department to include:
 - 1. Waste received
 - 2. Waste code numbers
 - 3. Volumes
 - 4. Dates
 - 5. Treatment and disposal method
 - 6. Disposal site used
 - b. Report of analysis on monitoring wells, leachate detection wells, and air quality monitoring
 - c. CSI internal weekly reports of waste handled, volumes, treatment, and disposal method
 - 2. Follow-up Procedures
 - a. Travel to and communicate with customer's technical staff about questions and/or problems regarding waste treatment at the site
 - b. Help Sales Engineers and other CSI personnel with technical problems concerning waste registration, treatment, or disposal
- C. Hiring and Training
 - 1. Hiring
 - a. Interview and select possible candidates for

technician positions

b. Will select, with the approval of the Facility

Manager, people to fill these positions

2. Training

- a. Supervise the training of all appropriate personnel with regard to health, technical, and chemical handling procedures
- b. Will assist in the program selection for guest speakers at training and safety meetings
- c. Will prepare chemical handling demonstration for the monthly safety meetings
- d. Be responsible for training technicians as to proper laboratory techniques and CSI policies

II. AUTHORITY AND QUALIFICATIONS

- A. Reporting and supervision of technical personnel
 - 1. Reporting
 - a. Facility Manager: for day-to-day activities and site specific duties and for adhering to company procedures, etc.
 - b. Also reports to CSI Vice President on policy matters
 - c. Both the Facility Manager and Vice President

 must agree before any terminations or merit

 increases are instituted

2. Supervision

- a. Supervise technicians
- b. Supervise waste handling procedures and personnel on site

c. Recommend to the Facility Manager disciplinary action for people that do not adhere to these policies and procedures

B. Qualifications

- 1. Must have a degree in Chemistry
- At least one year experience in proper laboratory procedures
- 3. One year experience in chemical waste handling is desired
- 4. Familiar with RCRA, U.S.E.P.A. and Colorado Solid Waste Regulations and guidelines
- 5. Must complete advanced first aid training within six months after hiring

III. COMPENSATION

A. This position will be salaried

NOTE: The hiring, firing, and any raises must be approved by both the Facility Manager and the Vice President of CSI

- B. Other Compensations
 - 1. Vehicle allowance
 - 2. Expense account
 - 3. Incentive bonus may be provided

IV. OBJECTIVES

- A. Establish and maintain a good repor with customer's technical staff
- B. Be willing to help with any technical problems and/or questions on or off the site
- C. Set-up complete records, logs, and reports to proper

- agencies
- D. Make sure Federal, State, and CSI rules and regulations are followed
- E. Set-up and make sure the health program meets or exceeds industry and OSHA standards
- F. Supervise waste handling procedures on and off site to protect people and the environment
- G. Attend appropriate courses and seminars to ensure our programs are kept up-to-date
- H. Get involved with local and state chemical associations and with environmental community activities
- I. Establish and maintain a good repor with Federal,
 State, and Local Health Department officials
- J. Help maintain a good CSI image in the community

LAB TECHNICIAN

I. RESPONSIBILITIES

- A. Approval of waste streams from customer to be brought on-site for treatment/solidification
- B. Approval for disposal, transfer, or shipping of waste to/from the site after the following criteria are met:
 - 1. Proper paperwork accompanies the waste
 - 2. Sampling and analysis of waste to assure identity
 - Designation of proper safety gear and procedures to be followed
 - 4. Filling out a Waste Disposal Transportation
 Authorization Sheet
- C. Supervise unloading of waste
 - 1. Ensure that wastes are discharged in the proper locations
 - 2. Ensure compliance with safety rules
- D. Help Chemist with inspections of facilities including but not limited to, first aid station, safety equipment storage and condition, safety showers and eyewashes, employee locker room, etc.
- E. Help Chemist with training sessions for employees on respirators, health, and chemical handling procedures

- F. Dispense first aid on-site
- G. Supervise weekly tank guaging and daily cleanup of tank farm
- H. Cleaning of respirators
- I. Sample all monitoring facilities
- J. Perform other routine duties as assigned by the Chemist II. RECORD KEEPING
 - A. Updating Waste Characterization reference books
 - B. Log Lab Recommendations and Service Requests
 - C. Daily logging of activities on-site
 - D. Daily logging of waste movement

III. QUALIFICATIONS

- A. Must have a high school education
- B. At least one year college Chemistry or one year experience in laboratory and in waste handling procedures
- C. Will be required to have advanced first aid training within 6 months after hiring

IV. AUTHORITY

- A. This position reports to Chemist
- B. Supervision of truck drivers during loading/unloading to ensure proper discharge of waste and compliance with safety rules

V. COMPENSATION

A. This is an hourly position

DRIVER/OPERATOR

I. RESPONSIBILITIES

- A. Be familiar with all equipment to be used
- B. Inspection: It is the responsibility of each driver to inspect his vehicle and report maintenance needs before operation
 - Drivers' daily vehicle inspection report will be completed and signed prior to leaving your terminal
 - a. Pay special attention to all tank valves, flanges, hatches, dust caps and gaskets for leaks, loose fittings, etc.
 - 2. Do not operate a piece of defective equipment that could cause an accident or a spill. The Service Supervisor or Facility Manager, as appropriate, must be notified
- C. Loading and Unloading of Materials: It shall be the responsibility of each driver to observe all safety rules and wear all required safety equipment during loading or unloading operations
- D. Transportation:
 - 1. Follow all federal, state and local laws regarding

- the safe operation of a motor vehicle
- 2. Follow the designated route to your destination
- 3. Always maintain a professional attitude
- 4. Frequently check your trailer through the use of your mirrors. Look for leaks, seepage, vapors or puddles when stopped for various traffic conditions or driving breaks
- 5. There will be <u>no smoking</u> or <u>open flames</u> within 50 feet of the vehicle. Smoking is allowed in the cab of the vehicle, and extinguish all smoking material in the ashtray
- E. Observe all plant and/or CSI safety rules and regulations

II. RECORD KEEPING

- A. Maintain proper documentation and paperwork
- B. Maintain daily logs and vehicle maintenance reports

III. AUTHORITY

- A. Reports to Service Supervisor and/or Facility Manager
- IV. COMPENSATION
 - A. This is an hourly paid position

LABORER

I. RESPONSIBILITIES

Will assist drivers on in-plant work and also any jobs that may occur on CSI's facilities

II. AUTHORITY

This position reports to Chemist and Facility Manager

III. COMPENSATION

This is an hourly paid position

APPENDIX M - CLAY LINER EFFICIENCY CALCULATIONS

INTRODUCTION

A clay liner and drainage system has been designed. Conservative assumptions and calculations were made in all cases to provide a large safety margin. The design calculations take into account effects of capillary pressure in the partially saturated zone beneath the liner. Capillary pressure increases the gradient across the liner thus increasing the flow. A sand layer beneath the clay liner has been included to minimize capillary effects. To increase flow to the drain pipe, a gravel layer was placed on top of the liner.

ASSUMPTIONS AND METHOD

Seepage calculations were made using the method developed by McWhorter and Nelson (1978). The following equations determine whether or not the foundation material, sand beneath the liner, is saturated or unsaturated:

$$h_f = y + D_g + D_1 - K_f (\underline{D}_g + \underline{D}_1 _{K_1})$$

hf < hd, unsaturated flow beneath liner

 $h_f \geq h_d$, saturated flow beneath liner

where

y = depth of water standing on surface of cell

 D_g = saturated depth in gravel above the liner

 D_1 = saturated depth of liner

 K_f = hydraulic conductivity of foundation material

 K_g = hydraulic conductivity of gravel above the liner

K₁ = hydraulic conductivity of liner

ha = displacement pressure of foundation material

h_f = pore water pressure head at the liner foundation
 interface

Darcy velocity through the liner was then calculated using the following form of Darcy's equation:

$$q = \underbrace{y + D_g + D_1 - h_d(q/K_f)^{-1/(2 + 3)}}_{K_g} + \underbrace{D_1}_{K_1}$$

where

q = Darcy velocity

= pore size distribution.

Darcy's equation was also used to compute flow to the drain pipes through the gravel layer. Displacement pressure was estimated using the following equation (McWorter and Nelson, missprint $k = -9.66(K_f/n - r) - .401$

1978):

where

n = porosity of foundation material
r = residual volumetric water content of foundation
material.

Values for porosity, initial volumetric water content, hydraulic conductivity, and residual volumetric water content, were assumed based on field observations and selected values from McWhorter and Sunada (1977). Transit times were calculated assuming that the liner is saturated, which is almost equivalent to taking the U.S. EPA modified transit time equation (1986). The McWhorter method is more conservative since the pore water pressure estimate at the liner foundation interface is more negative than the displacement pressure used in the modified transit time equation.

DESCRIPTION OF CASES

Since the percentage of water flowing into the drains versus water which could flow through the liner is a function of the boundary conditions and time, three cases were considered. In the first case no flow to the drain is assumed since the flow rate could be so low that the negative pore water pressure is the dominant driving mechanism. In the second and third cases, it was assumed that sufficient head has developed on top of the liner to drive water into the drain pipe. In the second case the head on top of the liner is zero and in the third it is 0.5 feet. In all cases it was assumed that the liner was initially saturated. If the liner is compacted to near standard procter density and optimum moisture content, the saturated approximation for the liner will be sufficiently accurate.

Since laboratory hydraulic conductivities are sometimes difficult to obtain in the field, each case was calculated for two liner hydraulic conductivities. The laboratory determined value of 1.0E-7 cm/sec and a potential real field value of 1.0E-6 cm/sec. With good fill control, a value of 1.0E-7 cm/sec is expected.

Materials exhibiting different hydraulic conductivities were considered for placement above and below the liner with respect to facilitating drainage, decreasing the gradient across the liner, and cost effectiveness.

RESULTS AND CONCLUSIONS

Under normal operating conditions, no flow through the waste materials to the liner is expected. The U.S. EPA water balance method (Fenn et al., 1975) indicates that net movement of water resulting from precipitation in the cell cap is upward. Free water left over from the hydration process in the solidified kiln ash is negligible when the proper mixing ratio is used. The ratio will be determined in the CSI lab on a per material basis.

If a worst case event occur where there is water present, calculations indicate that it is necessary to use a material (gravel) above the liner having a hydraulic conductivity greater than or equal to 1 cm/sec on a one percent grade to ensure complete drainage. Otherwise, as much as 50 percent of the water could flow through the liner. A drain pipe diameter of 5 inches is required on a one percent grade.

Logar What can o

Should flow through the solid waste materials occur and assuming no drainage, approximately 330 days of continuous seepage to the liner would be required for complete penetration of the liner. With extremely low Darcy velocities, that is less than 4.0E-7 cm/sec, 100 percent of the flow is into the liner rather than into the drain pipe. With higher Darcy velocities, that is greater than 4.0E-7 cm/sec, a maximum of 0.4 percent of the total volume of water would flow into the liner, assuming a liner hydraulic conductivity of 1.0E-7 cm/sec. If a hydraulic conductivity of 1.0E-6 cm/sec is assumed, a maximum of 3.0 percent would flow into the liner.

Since no flow is expected from precipitation, free water from the solidification process is insignificant, and the transit time is substantial, it is our opinion that the gravel drain system, liner, and capillary sand barrier are adequate to protect soil and groundwater from contamination.

REFERENCES

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- U.S. Environmental Protection Agency, 1986, Design Construction and Evaluation of Clay Liners for Waste Management Facilities: EPA/530-SW-86-007.
- U.S. Environmental Protection Agency and U.S. Department of Agricultural Soil Conservation, 1977, Preliminary Guidance for Estimation Erosion on Areas Disturbed by Surface Mining Activities in the Interior Western United States: Denver, Colorado Interagency Agreement EPA-IAG-D6-F154.
- U.S. Environmental Protection Agency, 1975, Use of the Water Balance Method for Predicting Leachate Generation from Solid Waste Disposal Sites: Report SW-168 written for the Office of Solid Waste Management Programs.

APPENDIX N - COMPATIBILITY INFORMATION

- Under optimum compaction, a 91.4-cm (3-foot) liner of the clay in a tank 7.01 m (23 feet) deep containing benzene can begin to leak within 36 days.
- 4.5.22.1.3 <u>Discussion</u>—Although the soil sample used in the Trinity Engineering experiments is not adequately characterized, the results of the test clearly indicate potential for large permeability increases resulting from exposure to concentrated nonpolar hydrocarbons. The apparatus and test procedures used differ substantially from those used in permeability tests conducted by other investigators.

The data also illustrate the importance of moisture content during compaction. The performance of clay-soil liner in contact with chemicals such as benzene could be drastically influenced by the uniformity of the moisture content when the liner material was installed and compacted.

4.5.22.2 Test Data Submitted to Pennsylvania Department of Environmental Resources--

The Pennsylvania Department of Environmental Resources has received data pertinent to clay liner/chemical compatibility. The data pertain to specific sites and specific wastes and were submitted to the State by consulting engineers. Testing procedures vary and information needed to evaluate the data is not always provided in the reports. All test results show the clay liner permeabilities to be "within the range required" after exposure to the wastes tested.

Report A deals with tests to evaluate effects on a liner material in contact with a waste comprised of one part oil contaminated soil and four parts water. For the permeability tests, samples were air dried and then recompacted, at ±2 percent of optimum moisture content, to 95 percent of maximum dry density. Stainless steel molds 10.2 cm (4 inches) in diameter and 11.7 cm (4.6 inches) in height were used. After trimming, the molded sample was transferred to a constant head permeability device. A backpressure was applied to two of the four samples during saturation with 0.01 N calcium sulfate solution. Permeability measurements with the calcium sulfate showed no significant differences between the results for samples with or without backpressure. Thus, it was concluded that the effect of entrapped air was minimal.

After permeability tests with the standard calcium sulfate, the liner samples were placed in contact with the test fluid, sealed, and placed on a rocking table for 30 days. Three of the samples were tested with the waste fluid and one with calcium sulfate solution. After 30 days, the samples were drained and the permeability tests repeated. Reported results are shown in Table 4-15.

No discernible color change was observed after a 30-day exposure to the waste fluid. No shrinkage along the sides of the mold was observed, although a change in height of approximately 0.64 cm (0.25 inch) was reported.

TABLE 4-15. PERMEABILITY TEST RESULTS^a
(Pennsylvania Case A)

	Coefficien	t of permeability (cm/s)
Sample	With 0.01 N $CaSO_4$	After exposure to test fluid ^b
A ^C	1.1 × 10 ⁻⁷	1.2 × 10 ^{-7d}
ВС	5.9×10^{-8}	3.9×10^{-8}
С	9.4×10^{-8}	1.1×10^{-7}
D	1.7×10^{-7}	Not tested

^aTest data reported to Pennsylvania Department of Environmental Resources.

^bTest fluid was one part oil-contaminated soil to four parts water.

^cSamples under backpressure during initial permeability tests.

 $^{^{}m d}$ Control sample--tested after 30-day exposure to 0.01 N CaSO₄.

The waste tested--one part oil contaminated soil and four parts water--was not characterized further. Neither the extent of the oil contamination nor the characteristics of the contaminated oil were reported. It may be assumed that either tap water or deionized water was used to prepare the waste fluid. The extent to which the oil or its contaminants would be extracted by the water is unknown but probably very small. Although it is not discussed in the report, the waste material in contact with the clay liner sample may have involved three phases, with oil being the lightest phase.

No indication of the length of the permeability testing procedure or pore volumes displaced is given. The hydraulic heads used in the tests also are not reported. These details may be stated in a letter that is referenced in the report. Soil characterization data were not included.

Report B describes the results of a similar investigation in which liner samples from a disposal site were tested with four different wastes. The permeabilities are reported in Table 4-16. All samples showed slight (twofold) increases in permeability after a 30-day exposure to the wastes. It is notable that "control sets" exposed for 30 days to 0.01 N CaSO $_4$ showed similar increases.

Soil properties are listed below:

Cation exchange capacity: 11.1 meq/100 g soil

Predominant exchangeable cation: Calcium

Major mineral fraction: Alpha quartz

Other minerals (trace to minor): Microcline, Adularia, Muscovite,

Kaolinite

Percent clay size (0.002 mm): 13

Percent silt size (0.002 mm -0.05 mm): 30

Percent sand size (0.05 mm -2.0 mm): 22

Percent larger than 2.0 mm: 35

Liquid limit (percent water): 31

Plastic limit (percent water): 22

Wastes used in the tests were not characterized beyond the description given in the table. Presumably, they were diluted with water as was done in Case A, but this is not stated in the report. The duration of the tests, pore volumes replaced, and hydraulic head are not provided in the report.

Report C provides permeability data on a soil sample tested with chrome ore leachate. The leachate (pH = 13.0) contained 1,400 mg/L total chromium and 1,200 mg/L hexavalent chromium. A constant head test, conducted at a pressure of 20 psi, gave a permeability of 1.2×10^{-8} cm/s. With the falling head method, a permeability of 2.2×10^{-8} cm/s was measured.

Soil characterization and details of the test method were not provided in the brief report.

TABLE 4-16. PERMEABILITY TEST RESULTS^a
(Pennsylvania Case B)

	Coefficient of permeability (cm/s)					
Sample	Before exposure to waste	After exposure to waste	Waste			
Α	1.4 × 10 ⁻⁸	2.0 × 10 ⁻⁸	Electric furnace baghouse dust			
В	1.4 × 10 ⁻⁸	2.1 × 10 ⁻⁸	Tar decanter sludge (high in organics)			
С	1.8 × 10 ⁻⁸	3.0×10^{-8}	Neutralized pickle liquor rinse water sludge			
D	1.8 × 10 ⁻⁸	3.0 × 10 ⁻⁸	Hot strip mill recycle system sludge (high in oil)			

^aTest data reported to Pennsylvania Department of Environmental Resources.

pore volumes of test permeant fluid were passed through the samples. Elevated hydraulic gradients 110 and 450 were used for the tests, and cell pressures were 2.5 and 4.5 kg/cm 2 . Test times ranged from 100 to 350 days. In three of the samples, more than 12 pore volumes were exchanged.

Project C--A waste fluid characterized by high salt concentration and high total organic carbon was tested in four soil samples. The samples contained varying proportions of two soils--a fine-to-coarse sand, and a sandy clay with 1 to 3 percent commercial bentonite.

Permeability tests were run at a hydraulic gradient of 20. After initial permeability increases in some samples, the permeabilities decreased. Samples were tested for 692 days. For each sample, between 2 and 13 pore volumes were exchanged. The permeability decrease noted was approximately 1 order of magnitude for a cement bentonite sample. For the other three soil mixtures, permeability decreased by a factor of 2 to 4.

The constituents present in the permeant are not known precisely.

Available chemical analysis data indicate that the chemical concentrations in the waste material vary considerably from year to year.

<u>Project D</u>--Slight decreases in permeability were reported for clay-soil samples (with bentonite) exposed to contaminated groundwater samples taken from piezometers. Soil samples were 25 percent flyash, 25 percent uncontaminated clay, 25 percent contaminated clay, and 25 percent silt. Bentonite (1 percent) was added.

Two permeant fluids were tested; pH values were 10.57 (Sample A) and 8.8 (Sample B). Specific conductance was reported at 35,300 and 13,200 $\mu mho/cm$ at 25 °C.

Samples were tested at a gradient of 190 with cell pressure at 1.5 kg/cm 2 . Test duration was 90 days with more than three pore volumes exchanged. Final permeabilities were determined to be below 1 \times 10 8 cm/s.

Project E--Permeability tests were run on a silty clay soil from a proposed facility with four waste leachates as the permeant fluid. Soil samples were compacted in the laboratory (95 percent of standard Proctor density). Chemical characteristics of the waste fluids tested are shown in Table 4-17.

Permeability tests were performed at a hydraulic gradient of 47 and cell pressure of 0.75 kg/cm 2 . Total test time was 40 to 50 days. The results of the permeability tests are given in Table 4-18. All samples were saturated with 0.01 N calcium sulfate solution prior to introduction of the waste fluid.

Project F--Permeability data on undisturbed Shelby tube samples tested with tap water showed permeability values ranging from 6.1×10^{-8} to 2.0×10^{-9} cm/s with an average of 1.0×10^{-8} cm/s. Fifty-seven samples were tested with total test time varying from 6 to 10 days. Hydraulic gradient was 25 to 50, and cell pressure was 3.5 to 5.5 kg/cm².

TABLE 4-18. RESULTS OF PERMEABILITY TESTS, PROJECT Ea

Sample fluid	Permeant	Pore volumes exchanged	Permeability (cm/s)
P-1	0.01 N CaSO ₄	-	1.6 × 10 ⁻⁷
	No. 3 leachate	0 - 3.2	1.7×10^{-7}
		3.2 - 12.7	1.4×10^{-7}
P-2	0.01 N CaSO ₄	-	1.1×10^{-7}
	No. 4 leachate	0 - 8.8	1.3×10^{-7}
P-3	0.01 N CaSO ₄	- .	1.4×10^{-7}
	Sludge leachate	0 - 4.7	1.5×10^{-7}
		4.7 - 13.5	2.4×10^{-7}
		13.5 - 17.2	4.2×10^{-7}
P-4	0.01 N CaSO ₄	-	1.1×10^{-7}
	Composite leachate	0 - 0.8	1.2×10^{-7}
	•	0.85.6	8.1 × 10 ⁻⁷

^aData from D'Appolonia Consulting Engineers, Inc., 1983.

APPENDIX O — WATER BALANCE CALCULATIONS ***PRODUCTOR************************************	
	APPENDIX O - WATER BALANCE CALCULATIONS
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APPENDIX P - EROSION AND SOIL LOSS CALCULATIONS